

## Appendix 6: Geotechnical Factual Report



# Dunedin City Council

## Waste Futures - Smooth Hill Landfill

### Geotechnical Factual Report



August 2020



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# 1. Introduction

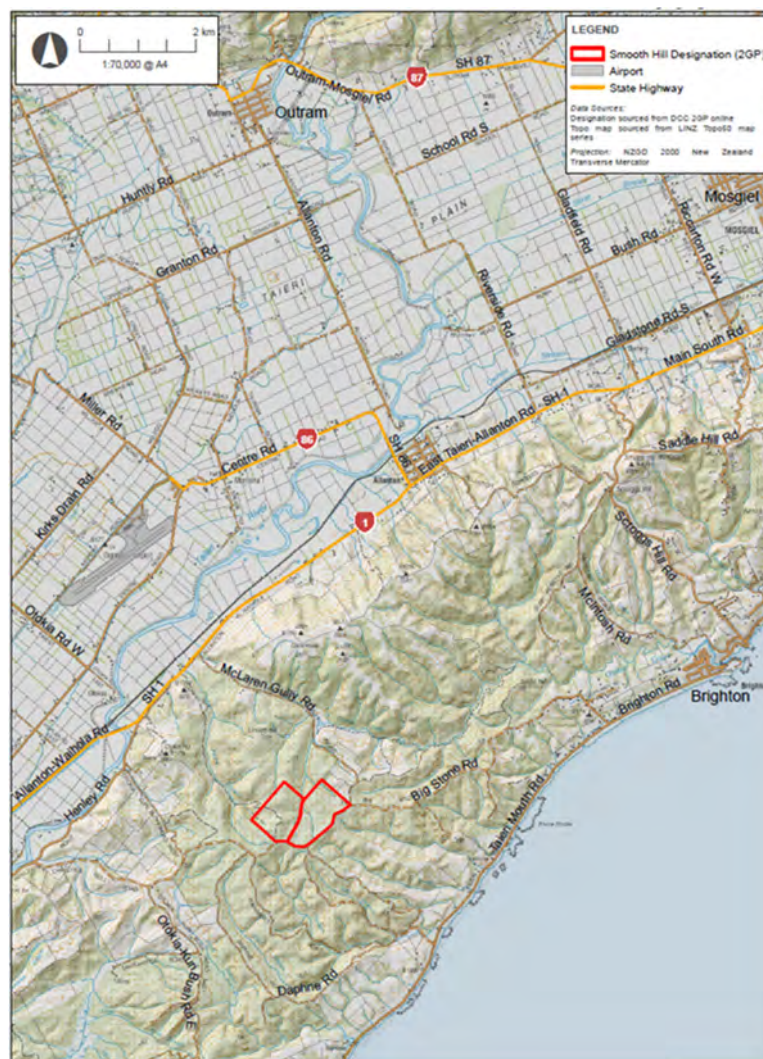
## 1.1 Project background

The Dunedin City Council (Council) collects residential waste and manages the disposal of both residential and the majority of commercial waste for the Dunedin City area and environs.

The Council has embarked on the Waste Futures Project to develop an improved comprehensive waste management and diverted material system for Dunedin, including future kerbside collection and waste disposal options. As part of the project, the Council has confirmed the need to develop a new landfill to replace the Council's current Green Island Landfill, which is likely to come to the end of its functional life sometime between 2023 and 2028.

The Council commenced a search for a new landfill location in the late 1980s and early 1990s and selected the Smooth Hill site in south-west Dunedin, shown in Figure 1 below, as the preferred option. At that time, the site was designated in the Dunedin District Plan, signalling and enabling its future use as a landfill site. The Council also secured an agreement with the current landowner, Fulton Hogan Ltd, to purchase the land. Over the following period, the Council extended the life of Green Island Landfill and further development of the Smooth Hill site has been on hold.

Figure 1 Site location



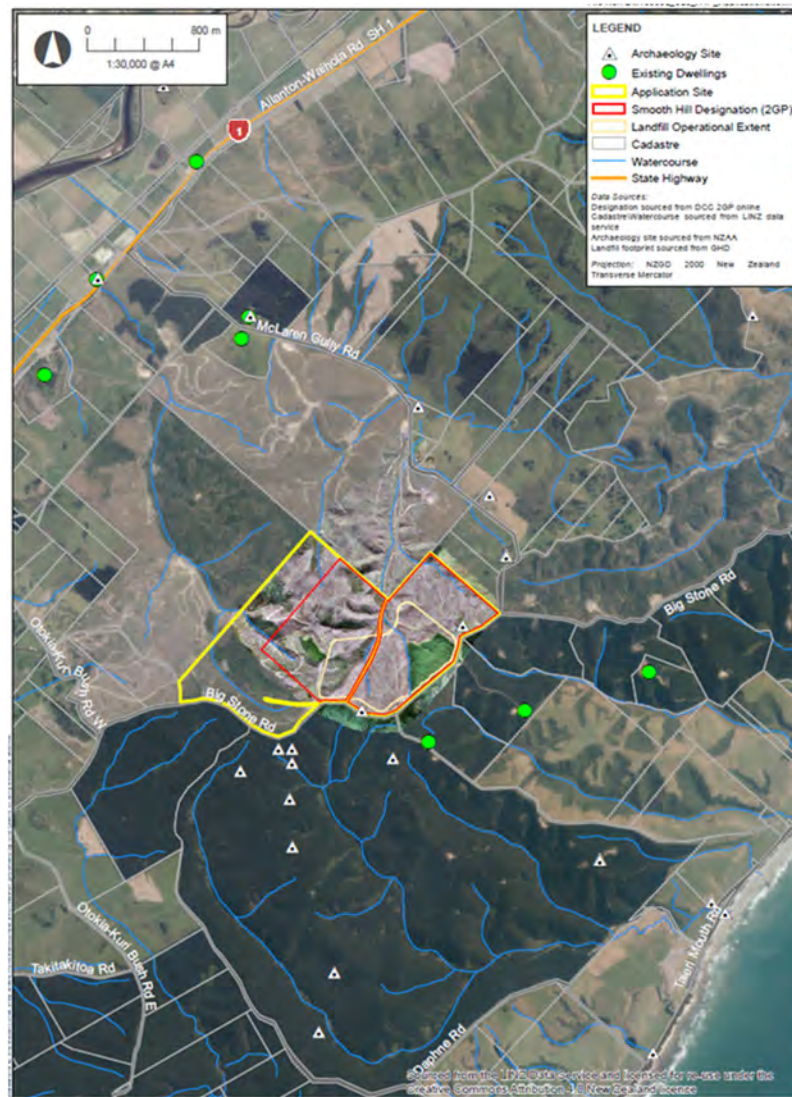
As part of the Waste Future's Project, the Council has reconfirmed the technical suitability of Smooth Hill for the disposal of waste. The Council has proceeded to develop a concept design for the landfill and associated road upgrades. The concept design (the subject of this report) for the landfill has been developed by GHD Ltd (GHD) with technical input from Boffa Miskell, and represents contemporary good practice landfill design that meets adopted New Zealand landfill design standards.

The proposal includes the following key components:

- The staged construction, operation, and aftercare of a Class 1 landfill within the existing designated site to accept municipal solid waste. The landfill will have a capacity of approximately 6 million cubic metres (equivalent to 5 million tonnes), and expected life (at current Dunedin disposal rates) of approximately 55 years. The landfill will receive waste only from commercial waste companies or bulk loads.
- Infrastructure to safely collect, manage, and dispose of landfill leachate, gas, groundwater, and stormwater to avoid consequential adverse effects on the receiving environment.
- Facilities supporting the operation of the landfill, including staff and maintenance facilities.
- Environmental monitoring systems.
- Landscape and ecological mitigation, including planting.
- Upgrades to McLaren Gully Road including its intersection with State Highway 1, and Big Stone Road, to facilitate vehicle access to the site.

The proposed Smooth Hill landfill site is located approximately 23 km southwest of Dunedin City. The boundary of the proposed site is shown in Figure 2. The waste facility itself will operate within these boundaries.

Figure 2 - Site Environs



## 1.2 Scope of geotechnical investigation

This report presents the factual results of the geotechnical investigation along with published and Client supplied geotechnical data related to the proposed waste site development. The purpose of the investigation was to assess the subsurface geotechnical and hydrogeological conditions at the proposed Smooth Hill landfill site. The hydrogeology is reported separately.

This information in this report has been used to inform and support the landfill design and the Assessment of Environmental Effects (AEE) and resource consent applications.



## 2. Site Setting

### 2.1 Site description

The proposed site is bordered by Big Stone Road along its southern boundary. Access from State Highway 1 (SH1) is currently via McLaren Gully Road. The proposed site is bounded to the north and west by forestry land, and to the northeast by farmland. Figure 3 provides a closer view of the proposed site.

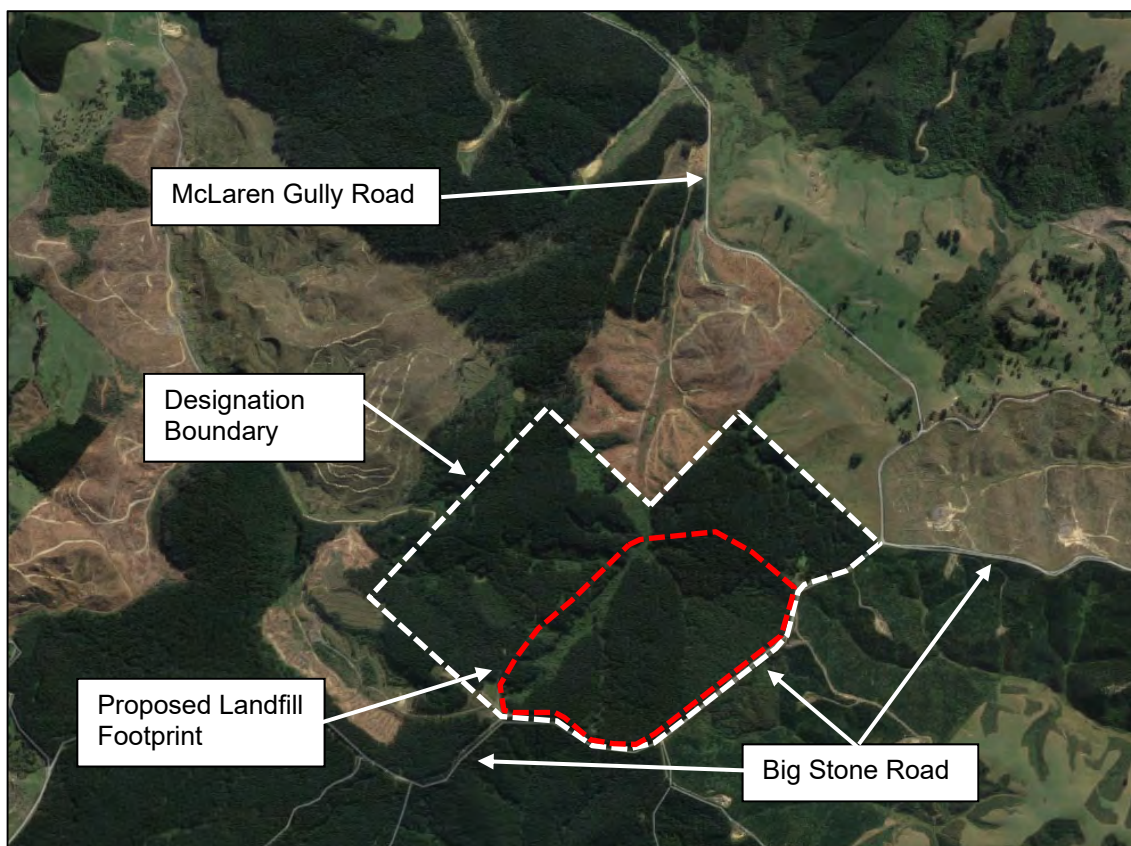


Figure 3 Proposed landfill site (base image sourced from Google Maps)

The proposed site is located in a south to north trending gully, which is fed by smaller gullies from the east, west and south. The flow direction for water exiting the gully is from the south to the north. The slopes around the southern half of the site form a natural “amphitheatre” shape, which is bisected by a larger central ridge, and a smaller ridge in the south-western corner – both trending south to north.

The site was, until recently, covered by a Radiata Pine plantation, the site cover is now a mixture of scrub, bare earth, forestry waste and replanted pine. A number of existing forestry tracks provide access around the site.

The ground is typically wet and boggy in the base of the gullies where there is standing or seeping water.

## 2.2 Local geology

### 2.2.1 Published geology

A review of the available geological maps (Bishop [1994], and Bishop and Turnbull [1996]) covering the site shows that the main lithology expected to be encountered is the Henley Breccia unit. Although not shown on the geological map, it is expected that the Henley Breccia unit is overlain by several metres of loess deposits, and locally by alluvium and colluvium.

Figure 4 presents an excerpt from the Bishop (1994) geological map.

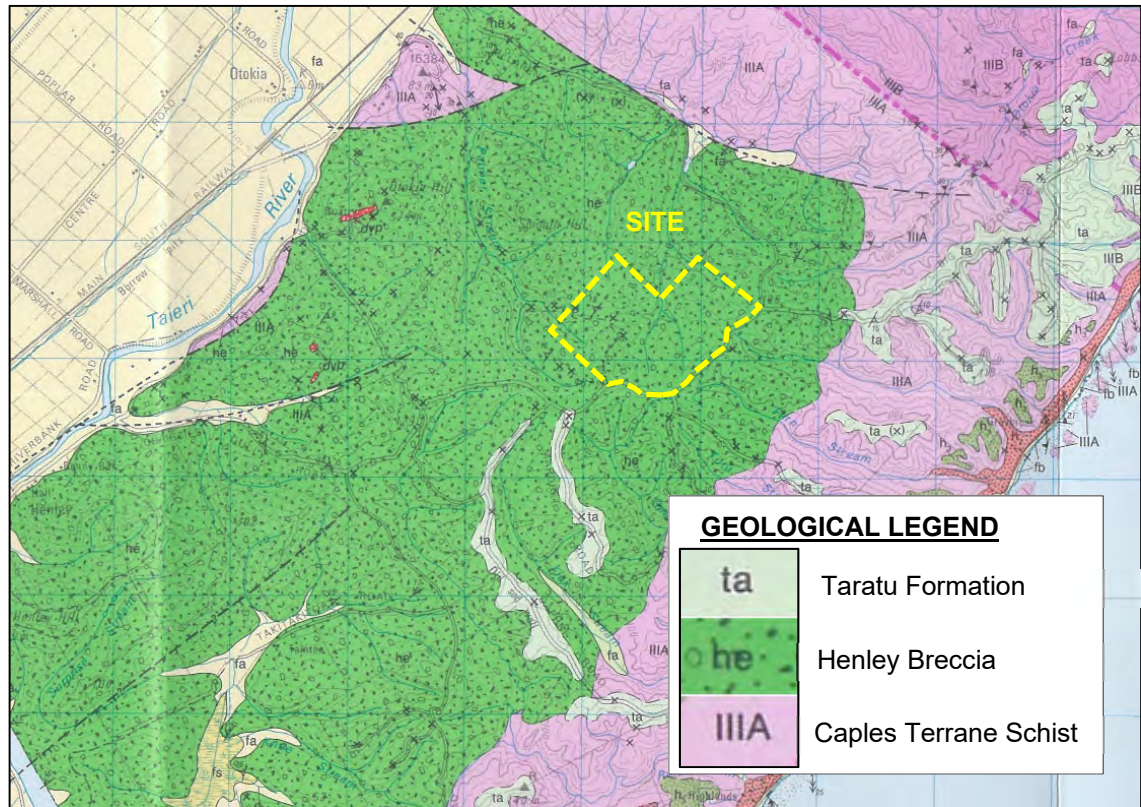


Figure 4 Excerpt from 1:50,000 Geology of the Milton Area (Bishop, 1994)

### 2.2.2 Expected lithologies

The basement rock in the proposed site area is expected to be Caples Terrane schist, textural zone IIIA (map symbol *IIIA*), which comprises well foliated quartzofeldspathic schist with prominent quartz veins. The schist was metamorphosed in the Jurassic period, and the metamorphic grade (textural zone) increases northward.

The schist basement is overlain unconformably by the Upper Cretaceous Henley Breccia (map symbol *he*) – a terrestrial sequence of piedmont breccias and conglomerates up to 1000 m thick. The breccia was derived from a high-standing schist block immediately west of the present-day Titri Fault. Henley Breccia was tilted before the formation of the Otago peneplain, which was cut across both it and the schist basement prior to the deposition of a relatively thin set of transgressive Upper Cretaceous to Tertiary terrestrial and shallow marine sediments (Bishop & Turnbull, 1996).

Taratu Formation (map symbol *ta*) is mapped as outcropping along the tops of several ridgelines to the south and east of the site. The Taratu Formation unconformably overlies the Henley Breccia and comprises yellow quartz sand and pebble conglomerate, with minor clay, carbonaceous siltstone and lignite, and limonite and silica cemented quartz conglomerate.



Bishop (1994), and Bishop & Turnbull (1996) have not mapped surficial materials such as loess, weathered bedrock or organic soils. However, the following description of loess soils in Otago is provided in Bishop & Turnbull (1996): *“In the Dunedin map area, such unmapped surficial materials are dominated by loess which, where remobilised, grades into loess colluvium... Loess forms a widespread blanket across most of eastern Otago, particularly near the coast... Loess typically forms a yellow-brown, massive layer or series of layers, mixed at the base with weathered bedrock and overlain by darker organic-rich soil. Columnar jointing and shrinkage cracks are common. Where loess mantles slopes, down-slope creep and alluvial processes have incorporated clasts of weathered underlying material, upslope material, and organic matter to form ‘loess colluvium’.”*

### 2.2.3 Nearby faults

There are a number of mapped faults in the Otago region. The known faults within close proximity to the landfill site are listed in Table 1. Fault data has been gathered from the GNS Active Faults Database website, and from Stirling, McVerry, et al (2012).

Active faults are defined by GNS and NZS 1170:2004 as faults with recurrence periods of less than 2000 years. On the basis of this definition the closest known active fault to the site is the Alpine Fault at a distance of approximately 240 km to the north-west, which is also classified as ‘Major Fault’ by NZS 1170:2004.

Table 1 Summary of known faults

Fault Name	Approximate Distance from Site	Maximum Likely Magnitude, $M_w$	Average Recurrence Interval (years)
Titri Fault	3 km NW	unknown	unknown
Akatore Fault	6 km SE	7.4	3,480
Maungatua Fault	10 km NW	unknown	unknown
North Taieri Fault	13 km N	unknown	unknown
Hyde Fault	47 km NNW	7.2	12,810
Billy’s Ridge Fault	47 km NNE	7.1	9,470
Taieri Ridge Fault	50 km NNE	7.1	9,750
Fault #8894 (GNS)	50 km SW	unknown	unknown
Tuapeka Fault	56 km NW	unknown	unknown
Clifton Fault	56 km SW	unknown	5,000 – 10,000
Logan Burn Fault	60 km NW	unknown	3,500 – 5,000
Blue Mountain Fault	70 km W	7.3	12,690
Long Valley Fault	75 km NW	6.8	2,810
Gimmerburn Fault Zone	76 km N	7.2	5,850
Old Man Fault	85 km NW	7.4	362,150

Spylaw Fault	89 km W	7.3	12,440
Alpine Fault	240 km NW	8.1	340

## 2.3 Historic mining

Anecdotal evidence provided by a local resident indicates historic mining may have occurred in this area of Otago. A review of publically available data with regard to historic mining has been carried out. The following sources were consulted:

- Regional geological maps (Benson [1968], McKellar [1990], Bishop [1994], and Bishop & Turnbull [1996]);
- Historic aerial photographs, retrieved from the Retrolens historic imagery resource;
- Mindat.org: [www.mindat.org](http://www.mindat.org)
- NZ Mine Plans website: <https://mineplans.nzpam.govt.nz>
- Appendices to the Journals of the House of Representatives, 1890 Session I, Section C: <https://atojs.natlib.govt.nz/cgi-bin/atojs?a=d&d=AJHR1890-I&e=10--1-0>

The sources consulted suggest that the geological unit containing a potential valuable commodity in this part of Otago is the Taratu Formation (also known as Taratu Coal Measure on older geological maps). The main commodity mined in the region appears to be coal/lignite. In the vicinity of the proposed landfill, the Taratu Formation only occurs as a relatively thin layer at the top of the higher ridges on the eastern edge of the designation area and away from the proposed landfill footprint or appurtenant structures. Outcrops and boreholes associated with the Taratu Formation at the site do not show any lignite layers within these Taratu materials. It is considered highly unlikely that mining would have occurred within the designation area.

The only known abstraction on the site is a small borrow pit associated with the Taratu Formation deposits on the eastern edge of the designation area. Fulton Hogan have used this as a borrow site for gravel used to form logging tracks in the site vicinity.

## 2.4 Previous investigations

GHD is not aware of any previous investigations at the proposed landfill site, though an existing piezometer was found adjacent to the north-eastern site entrance.

## 3. Summary of Investigations

### 3.1 General

GHD carried out two phases of geotechnical investigations between 27 May to 17 June 2019 (Phase I), and between 24 October to 7 November 2019 (Phase II). McNeill Drilling was the drilling subcontractor used for the first phase, and Speight Drilling Ltd was the drilling subcontractor used for the second phase. The investigations comprised machine boreholes and test pits. All investigation works were carried out under the supervision of a GHD Engineering Geologist.

The second phase of investigations was designed to address gaps in the ground model data that were identified following the end of the first phase. Due to restrictions in place for the second phase (surveys of protected native lizards, and nesting native falcons), there were areas that could not be accessed for investigation and as a consequence a number of planned borehole and test pits were either re-located or not completed.

Materials recovered from the investigation were logged following the methods and procedures in the New Zealand Geotechnical Society's (NZGS) "*Guideline for the Field Description of Soil and Rock for Engineering Purposes*" (2005).

Shear vane testing was undertaken in accordance with NZGS's "*Guideline for Hand Held Shear Vane Test*" (2001). The peak and remoulded shear strength values shown on the attached logs (Appendix B) represent dial readings off the vane, adjusted using the BS 1377 calibration.

An investigation location plan is provided in Appendix A.

### 3.2 Machine boreholes

McNeill Drilling drilled ten machine boreholes (BH01 to BH10) between 27 May and 16 June 2019, using a truck mounted UDR600 rig. Speight Drilling Ltd drilled five machine boreholes (BH201 to BH203, BH209 and BH211) between 24 October and 7 November 2019, using a tracked, Maruka-mounted rig.

All boreholes were drilled from ground surface, with no hand or hydro-excavation carried out.

Core samples were retrieved by rotary drilling methods using PQ (96 mm diameter) triple tube drilling. BH201 and BH202 were cored to approximately 10.0 m bgl, and then wash drilled (no core recovered) to their termination depth.

Where practical, vane shear strengths were measured in the end of the core barrel with a hand held shear vane, using the techniques described in the NZGS guideline.

Table 2 summarises the details of the investigation machine boreholes. Borehole logs are provided in Appendix B.

Table 2 Summary of machine boreholes

Test ID	Site Location	Commenced	Completed	Total Depth (m bgl)	Termination Reason	Piezometer
BH01	Attenuation Basin Foundation	6/06/2019	6/06/2019	15.0	Target Depth	Yes, x 2
BH02	Toe Bund Foundation	27/05/2019	28/05/2019	15.0	Target Depth	Yes, x 2
BH03	Toe Bund Foundation	28/05/2019	29/05/2019	20.0	Target Depth	Yes, x 2
BH04	Toe Bund Foundation	6/06/2019	7/06/2019	15.0	Target Depth	Yes, x 2
BH05	Central Ridge	29/05/2019	30/05/2019	30.0	Target Depth	Yes, x 2
BH06	Southwest Ridge	13/06/2019	14/06/2019	30.0	Target Depth	No
BH07	Central Ridge	30/05/2019	4/06/2019	20.0	Target Depth	Yes, x 2
BH08	Southeast Perimeter	11/06/2019	11/06/2019	20.0	Target Depth	No
BH09	Western Perimeter	12/06/2019	12/06/2019	16.5	Target Depth	Yes, x 1
BH10	Northeast Ridge	04/06/2019	05/06/2019	20.0	Target Depth	Yes, x 2
BH201	Southern Perimeter	28/10/2019	01/11/2019	61.0	Target Depth	Yes, x 1
BH202	Southern Perimeter	2/11/2019	4/11/2019	60.6	Target Depth	Yes, x 1
BH203	Southwest Perimeter	7/11/2019	7/11/2019	19.7	Target Depth	No
BH204	Western Ridge	Not completed				
BH209	Western Perimeter	24/10/2019	24/10/2019	10.0	Target Depth	No
BH210	Central gully base	Not completed				
BH211	Eastern Gully Base	4/11/2019	6/11/2019	25.2	Target Depth	Yes, x2

### 3.3 Piezometers

Piezometers were installed in selected boreholes to allow for permeability testing and follow-up groundwater measurements.

Groundwater was not encountered in BH06, BH08, BH203 or BH209 - therefore, no piezometers were installed.

Piezometers were typically nested, with two 32 mm PVC pipes installed in each borehole (except BH09, BH201 and BH202). The pipe was slotted over the targeted screened zone and surrounded by a coarse sand pack. Bentonite seals were placed above and below each screened zone.

The piezometer details were provided by GHD hydrogeologists to suit the conditions encountered in each borehole. Table 3 summarises the piezometer details.

Table 3 Summary of piezometer details

Borehole ID	Piezometer ID	Screened From (m bgl)	Screened To (m bgl)
BH01	BH01a	2.0	4.0
	BH01b	8.0	9.0
BH02	BH02a	3.0	5.0
	BH02b	7.0	9.0
BH03	BH03a	8.3	10.3
	BH03b	13.0	15.0
BH04	BH04a	4.5	6.5
	BH04b	12.0	16.0
BH05	BH05a	14.0	16.0
	BH05b	19.0	22.0
BH07	BH07a	11.5	14.5
	BH07b	16.8	19.8
BH09	BH09a	14.5	16.5
BH10	BH10a	13.5	15.5
	BH10b	18.0	20.0
BH201	BH201	54.0	60.0
BH202	BH202	54.0	60.0
BH211	BH211a	8.5	11.5
	BH211b	22.0	25.0

### 3.4 Test pits

Under the supervision of GHD, Fulton Hogan excavated eleven test pits between 27 May 2019 and 12 June 2019, using a 22 tonne excavator.

Where practical and safe, vane shear strengths were measured in the base and sides of the excavation with a hand held shear vane, using the techniques described in the NZGS guideline.

Table 4 summarises the details of the test pits. Test pit logs are provided in Appendix B.

Table 4 Test pit summary

Test Pit ID	Site Location	Excavation Date	Termination Depth (m bgl)	Termination Reason	Materials Encountered
TP01	Manuka gully (stockpile area)	12/06/2019	2.5	Target Depth	Alluvium, HW rock, siltstone
TP02	Manuka gully (stockpile area)	12/06/2019	2.6	Target Depth	Colluvium, alluvium, buried topsoil, siltstone
TP03	Manuka gully (stockpile area)	12/06/2019	2.0	Target Depth	Alluvium, siltstone
TP05	Southwest gully base	13/06/2019	3.3	Target Depth	Colluvium, HW rock, siltstone
TP06	Gully east of central ridge	13/06/2019	2.5	Target Depth	Alluvium, siltstone
TP07	Southwest gully base	28/05/2019	2.5	Target Depth	Loess, siltstone, breccia
TP08	Gully between southern ridges	28/05/2019	4.5	End of reach	Fill, buried topsoil, loess
TP09	Southeast gully outflow	13/06/2019	3.0	Target Depth	Slip debris, buried topsoil, alluvium, sandstone
TP10	Future laydown area	10/06/2019	3.6	Target Depth	Loess, HW siltstone
TP11	Future laydown area	10/06/2019	3.8	Target Depth	Loess, HW siltstone
TP12	Future laydown area	10/06/2019	4.4	Target Depth	Fill, buried topsoil, loess, HW siltstone

\*Note: TP04 was deleted from the field programme

### 3.5 Bulk samples

Bulk samples of loess and completely weathered (CW) rock were collected from shallow test pits on 7 and 13 November 2019. The shallow test pits were excavated by Fulton Hogan, with a 20 tonne excavator. The bulk sample details are summarised in Table 5.



Table 5 Bulk sample summary

Bulk Sample ID	Sample Date	Sample Depth	Sampled Material
BS01	7/11/2019	0.5 m bgl	Loess
BS02	7/11/2019	1.0 m bgl	Loess / CW rock
BS03	7/11/2019	0.7 m bgl	Loess
BS04	7/11/2019	1.5 m bgl	Loess / CW rock
BS05	13/11/2019	0.6 m bgl	Loess
BS06	13/11/2019	1.0 m bgl	Loess / CW rock
BS07	13/11/2019	0.5 m bgl	Loess
BS08	13/11/2019	0.6 m bgl	Loess
BS09	13/11/2019	1.2 m bgl	Loess / CW rock
BS10	13/11/2019	0.7 m bgl	Loess
BS11	13/11/2019	1.3 m bgl	Loess
BS12	13/11/2019	0.4 m bgl	Loess
BS13	13/11/2019	1.2 m bgl	Loess
BS14	13/11/2019	0.5 m bgl	Loess
BS15	13/11/2019	1.1 m bgl	Loess / CW rock

### 3.6 Groundwater

To monitor whether the groundwater had returned to a static level after drilling, manual groundwater measurements were taken on several occasions during the field investigation programme. This was because, groundwater levels noted during or immediately after drilling are typically in an elevated state due to the use of water during the drilling process, and therefore may not represent a static groundwater level. Groundwater levels may also fluctuate seasonally.

At the completion of drilling BH01, prior to piezometer installation, the drillers observed artesian groundwater, in that groundwater was flowing out of the top of the borehole; the subsequent level measured in BH01a was also above ground level. However, since installation, the integrity of the shallow piezometer (BH01a) has been compromised and it is no longer possible to record a groundwater level, but groundwater can be observed leaking from around the edge of the installation indicating that artesian groundwater is present.

BH201 and BH202 were wash drilled to approximately 60 m to ensure interception of the groundwater table along the southern boundary.

An existing piezometer (comprising a single 50 mm PVC pipe, in a 100 mm diameter borehole) was discovered adjacent to the northeast site access. No information about this piezometer (drill date, target, etc.) is available. The base of this piezometer was measured at 42.50 m bgl.

The groundwater level (GWL) at the completion of drilling/excavation, and the most recent measurements are presented in Table 6.

Table 6 Summary of measured groundwater levels

Piezometer ID	GWL at End of Drilling (Date)	Latest GWL Measurement (25/11/19)
BH01a	NR*	Piezometer compromised
BH01b		0.02 m bgl
BH02a	0.75 m bgl (28/05/19) (measured pre-install)	-0.06 m bgl (above ground level)
BH02b		0.745 m bgl
BH03a	3.95 m bgl (29/05/19) (measured pre-install)	4.19 m bgl
BH03b		4.24 m bgl
BH04a	2.0 m bgl (07/06/19) (measured pre-install)	1.79 m bgl
BH04b		4.61 m bgl
BH05a	NR	Dry
BH05b		Dry
BH07a	NR	Dry
BH07b		Dry
BH09a	13.9 m bgl (12/06/19)	14.58 m bgl
BH10a	10.2 m bgl (07/06/19) (measured pre-install)	Dry
BH10b		Dry
NE Gate (existing)	NA**	26.07 m bgl
TP02	0.4 m bgl (12/06/19)	NFM***
TP03	1.2 m bgl (12/06/19)	NFM
TP05	1.9 m bgl (13/06/19)	NFM
TP07	1.4 m bgl (28/05/19)	NFM
BH201	NR	46.455 m bgl
BH202	NR	47.55 m bgl
BH211a	NR	2.81 m bgl
BH211b	NR	13.0 m bgl

\*NR = Not recorded

\*\*NA = Not available

\*\*\*NFM = No further measurements recorded

### 3.7 Investigation coordinates

Positions for machine boreholes and test pits were recorded by Woods Surveying. Coordinates are presented in the North Taieri Circuit (2000) projection, and elevation RLs are presented in terms of New Zealand Vertical Datum (2016).

BH06, BH08 and all test pits were picked up with a cluster of points around the pads. The coordinates for the most central pickup point has been selected to represent the test location. These points are marked with an asterisk (\*) in the table below.

TP05, TP07, TP08, and all of the Phase II investigations have not yet been surveyed. Coordinates for these test locations have been estimated from Google Earth, and are marked with a double asterisk (\*\*) in the table below. Elevations for these points have been estimated from the Stantec contour map presented in Appendix A.

Table 7 summarises the position coordinates for all test locations.

Table 7 Summary of test location positions

Test Location ID	Easting	Northing	Elevation (m RL)
BH01	396465.49	788214.52	96.01
BH02	396358.59	788022.89	97.41
BH03	396428.38	787998.34	107.48
BH04	396563.60	788063.75	108.15
BH05	396459.76	787862.12	129.50
BH06	396168.25*	787593.98*	149.75*
BH07	396493.65	787671.87	139.73
BH08	396809.71*	787700.67*	143.89*
BH09	395951.84	788050.36	132.80
BH10	396788.26	788118.50	139.07
BH201	396596**	787540**	144**
BH202	396181**	787498**	144**
BH203	395779**	787672**	182**
BH209	395775**	788148**	132**
BH211	396598**	787965**	107**
TP01	395988.85	788077.23	121.20
TP02	396103.50	788056.91	110.40
TP03	396262.16	788048.16	102.61
TP05	396281**	787868**	105**
TP06	396585.70	787800.45	108.24
TP07	396182**	787790**	120**
TP08	396303**	787682**	115**

Test Location ID	Easting	Northing	Elevation (m RL)
TP09	396577.97	787947.86	101.04
TP10	396820.11	788079.25	140.74
TP11	396907.03	788032.98	141.24
TP12	396956.93	787986.46	142.28
BS01 / BS02	396149**	787571**	150**
BS03 / BS04	396202**	787994**	135**
BS05 / BS06	396537**	787504**	152**
BS07	396500**	787616**	141**
BS08 / BS09	396490**	787771**	130**
BS10 / BS11	396441**	787922**	119**
BS12 / BS13	396382**	787582**	132**
BS14 / BS15	396366**	787738**	120**

### 3.8 Laboratory testing

#### 3.8.1 Phase I test schedule

Selected samples obtained from Phase I of the geotechnical investigation were tested at the IANZ accredited Central Testing Services laboratory in Alexandra. Table 8 summarises the laboratory testing programme undertaken.

Table 8 Summary of geotechnical laboratory testing

Sample Source	Depth From (m bgl)	Depth To (m bgl)	Atterberg Limits - NZS 4402:1986, Test 2.2, 2.3 & 2.4	Particle Size Distribution - NZS 4402:1986, Test 2.8.1 & 2.8.4.	NZ Standard Compaction - NZS 4402:1986, Test 4.1.1	Pinhole Dispersion and Crumb Test - ASTM D4647 & ASTM D6572	Triaxial Permeability - ASTM D5084
TP10	2.2	3.6	x	x	x	x	x
BH05	0.0	1.2					
BH07	0.0	1.4	x	x	x	x	x
(combined)							

#### 3.8.2 Phase II test schedule

On completion of Phase I of the geotechnical investigation a further suite of samples were tested by Central Testing Services in Alexandra. Two suites of lab testing were undertaken with the following purposes:

- To determine the suitability of the Loess soils for either lime or bentonite stabilisation as a method of reducing erodability / dispersivity. Eight (8) bulk samples were combined, and divided into four sub-samples. The four sub-samples were then tested as outlined in Table 9.
- To determine suitability of CW-HW Henley Breccia Soils for use as engineered fill beneath the landfill liner. Two samples were tested as outlined in Table 10.

Table 9 Summary of geotechnical laboratory testing for stabilised soils.

Sample Source	Sub-sample Number	Atterberg Limits - NZS 4402:1986, Test 2.2, 2.3 & 2.4 (Natural Soil)	Lime demand test (NSW Transport; Roads & Maritime Services Test Method T144 (Not IANZ Accredited))	Atterberg Limits - NZS 4402:1986, Test 2.2, 2.3 & 2.4 (Stabilised Soil)	NZ Standard Compaction - NZS 4402:1986, Test 4.1.1	Shear Strength – Shear Vane – NZGS 2001	Pinhole Dispersion and Crumb Test - ASTM D4647 & ASTM D6572
BS01 (0.5m)  BS03 (0.7m)	Sub-sample #1	X	X	X (Lime Stabilised – 1 day curing)	X	X	X
BS07 (0.5m)  BS08 (0.6m)	Sub-sample #2	X	X	X (Lime Stabilised – 7 day curing)	X	X	X
BS10 (0.7m)  BS11 (1.3m)	Sub-sample #3	X		X (Bentonite Stabilised – 1 day curing)	X	X	X
BS12 (0.4m)  BS13 (1.2m)	Sub-sample #4	X		X (Bentonite Stabilised- 7 day curing)	X	X	X

Table 10 Summary of geotechnical laboratory testing for engineered fill

Sample Source	Depth From (m bgl)	Depth To (m bgl)	Atterberg Limits - NZS 4402:1986, Test 2.2, 2.3 & 2.4	NZ Standard Compaction - NZS 4402:1986, Test 4.1.1	Unconfined Compressive Strength of Soil, NZS 4402:1986: Test 6.3.1
BH05	2.7	7.2	X	X	X
BH10	2.4	7.0	X	X	X

### 3.8.3 Phase I test results

Table 11 to Table 16 summarise the results of the laboratory testing outlined in Section 3.8.1. Detailed laboratory test results are presented in Appendix C.

Table 11 Summary of particle size distribution test results (NZS 4402:1986, Test 2.8.1 and 2.8.4)

Sample Source	Geological Unit	Percent Passing (%)			
		Gravel (2 to 60 mm)	Sand (0.06 to 2 mm)	Silt (0.002 to 0.06 mm)	Clay (<0.002 mm)
TP10	Loess	6	13	72	9
BH05/BH07	Loess	1	10	60	29

Table 12 Summary of Atterberg limit test results (NZS 4402:1986, Test 2.1, 2.2, 2.3 and 2.4)

Sample Source	Geological Unit	Water Content (%)	Liquid Limit	Plastic Limit	Plasticity Index
TP10	Loess	15.5	39	28	11
BH05/BH07	Loess	23.6	42	23	19

Table 13 Summary of NZ standard compaction test results (NZS 4402:1986, Test 2.1 and 4.1.1)

Sample Source	Geological Unit	Water Content – As Received (%)	Maximum Dry Density (t/m <sup>3</sup> )	Optimum Water Content (%)
TP10	Loess	15.5	1.71	16.0
BH05/BH07	Loess	23.6	1.70	17.5



Table 14 Summary of pinhole dispersion test results (ASTM D4647-13e1)

Sample Source	Geological Unit	Elapsed Time (min)	Flow Rate (ml/s)	Outflow Colour	Hole Diameter Pre-test	Hole Diameter Post-test	Classification
TP10	Loess	1	0.25	Slightly dark	1.0 mm	~2.0 mm (4 mm at exit)	Dispersive (D)
		5	0.27	Moderately dark			
		10	0.31	Dark			
BH05 / BH07	Loess	1	0.25	Barely visible	1.0 mm	~2.0 mm	Dispersive (D)
		5	0.27	Moderately dark			
		10	0.49	Very dark			

Table 15 Summary of crumb test results (ASTM D6572-13e2 (Method B))

Sample Source	Geological Unit	Elapsed Time	Estimated Slaking	Observations	Crumb Test Classification
TP10	Loess	2 min	~50%	No colloidal cloud	Grade 4 (Highly Dispersive)
		1 hr	~100%	Dense colloidal cloud over	
		6 hr	~100%	Moderate colloidal	
BH05/BH07	Loess	2 min	~20%	No colloidal cloud	Grade 4 (Highly Dispersive)
		1 hr	~100%	Dense colloidal cloud over	
		6 hr	~100%	Dense colloidal cloud over	

Table 16 Summary of triaxial permeability test results (ASTM D5084-16a)

Sample Source	Geological Unit	Cell Pressure (kPa)	Initial Permeability (m/s)	Final Permeability (m/s)
TP10	Loess	610	$2.9 \times 10^{-8}$	$3.2 \times 10^{-8}$
TP10	Loess	727	$2.7 \times 10^{-8}$	$2.8 \times 10^{-8}$
BH05/BH07	Loess	460	$1.7 \times 10^{-9}$	$2.1 \times 10^{-9}$
BH05/BH07	Loess	527	$5.6 \times 10^{-10}$	$5.3 \times 10^{-10}$

### 3.8.4 Phase II test results

Table 17 to Table 21 summarise the results of the laboratory testing outlined in Section 3.8.2. Detailed laboratory test results are presented in Appendix C.

Table 17 Summary of Atterberg limit test results (Natural Soils)

Sample	Geological Unit	Water Content (%)	Liquid Limit	Plastic Limit	Plasticity Index
Sub-sample #1	Loess (untreated)	25	41	25	16
Sub-sample #2	Loess (untreated)	25	41	25	16
Sub-sample #3	Loess (untreated)	25	41	25	16
Sub-sample #4	Loess (untreated)	25	41	25	16

Table 18 Summary of lime demand test results

Sample	Geological Unit	pH 0 % added Lime	pH 1 % added Lime	pH 2 % added Lime	pH 3 % added Lime	pH 4 % added Lime	pH 5 % added Lime	pH 6 % added Lime	pH 7 % added Lime
Sub-sample #1	Loess (untreated)	5.12	10.15	12.12	12.42	12.46	12.49	12.48	12.42
Sub-sample #1	Loess (untreated)	5.16	10.31	12.08	12.5	12.55	12.56	12.55	12.55

Table 19 Summary of Atterberg limit test results (Henley Breccia Formation)

Sample	Geological Unit	Liquid Limit	Plastic Limit	Plasticity Index
BH05 2.7 – 7.2 m	Henley Breccia – CW Siltstone	41	25	16
BH10 2.4 – 7.0 m	Henley Breccia – CW Siltstone / Sandstone	37	23	14

Table 20 Summary of NZ standard compaction test results (Henley Breccia Formation)

Sample Source	Geological Unit	Maximum Dry Density (t/m <sup>3</sup> )	Optimum Water Content (%)
BH05 2.7 – 7.2 m	Henley Breccia – CW Siltstone	1.76	16.0
BH10 2.4 – 7.0 m	Henley Breccia – CW Siltstone / Sandstone	1.85	14.0

Table 21 Summary of unconfined compressive strength of re-compacted samples (Henley Breccia Formation)

Sample Source	Geological Unit	Unconfined Compressive Strength (kPa)
BH05 2.7 – 7.2 m	Henley Breccia – CW Siltstone	100
BH10 2.4 – 7.0 m	Henley Breccia – CW Siltstone / Sandstone	93

## 4. References

The following documents have been consulted in preparation of the guideline:

- Bishop, D.G. 1994: *Geology of the Milton area*. Scale 1:50,000. Institute of Geological & Nuclear Sciences geological map 9. 1 sheet + 32 p. Institute of Geological & Nuclear Sciences Ltd, Lower Hutt, New Zealand.
- Bishop, D.G., Turnbull, I.M. (compilers) 1996. *Geology of the Dunedin Area*. Institute of Geological and Nuclear Sciences 1:250,000 geological map 21. 1 sheet + 52 p. Lower Hutt, New Zealand: Institute of Geological and Nuclear Sciences Limited.
- GNS Active Faults Database, <http://maps.gns.cri.nz/website/af/viewer.htm>
- New Zealand Geotechnical Society (2001): *Guideline for Hand Held Shear Vane Test*.
- New Zealand Geotechnical Society (2005): *Guideline for the Field Classification and Description of Soil and Rock for Engineering Purposes*.
- Stantec (not dated): *Smooth Hill Site – Plan*. Reference 80510415-01-001-S10, Revision A
- Stirling, McVerry, et al (2010): *National Seismic Hazard Model for New Zealand: 2010 Update*. Bulletin of the Seismological Society of America, Vol. 102, No. 4, pp. 1514-1542, August 2012.

## 5. Limitations

This report has been prepared by GHD for Dunedin City Council) and may only be used and relied on by the Client for the purpose agreed between GHD and the Client as set out in Section 1 of this report.

GHD otherwise disclaims responsibility to any person other than the Client and Council officers, consultants, the hearings panel and submitters associated with the resource consent and notice of requirement process for the Smooth Hill Landfill Project arising in connection with this report. GHD also excludes implied warranties and conditions, to the extent legally permissible.

The services undertaken by GHD in connection with preparing this report were limited to those specifically detailed in the report and are subject to the scope limitations set out in the report.

The opinions, conclusions and any recommendations in this report are based on conditions encountered and information reviewed at the date of preparation of the report. GHD has no responsibility or obligation to update this report to account for events or changes occurring subsequent to the date that the report was prepared.

The opinions, conclusions and any recommendations in this report are based on assumptions made by GHD described in this report. GHD disclaims liability arising from any of the assumptions being incorrect.

GHD has prepared this report on the basis of information provided by Client and others who provided information to GHD (including Government authorities)], which GHD has not independently verified or checked beyond the agreed scope of work. GHD does not accept liability in connection with such unverified information, including errors and omissions in the report which were caused by errors or omissions in that information.

The opinions, conclusions and any recommendations in this report are based on information obtained from, and testing undertaken at or in connection with, specific sample points. Site conditions at other parts of the site may be different from the site conditions found at the specific sample points.

Investigations undertaken in respect of this report are constrained by the particular site conditions, such as the location of vegetation and topography. As a result, not all relevant site features and conditions may have been identified in this report.

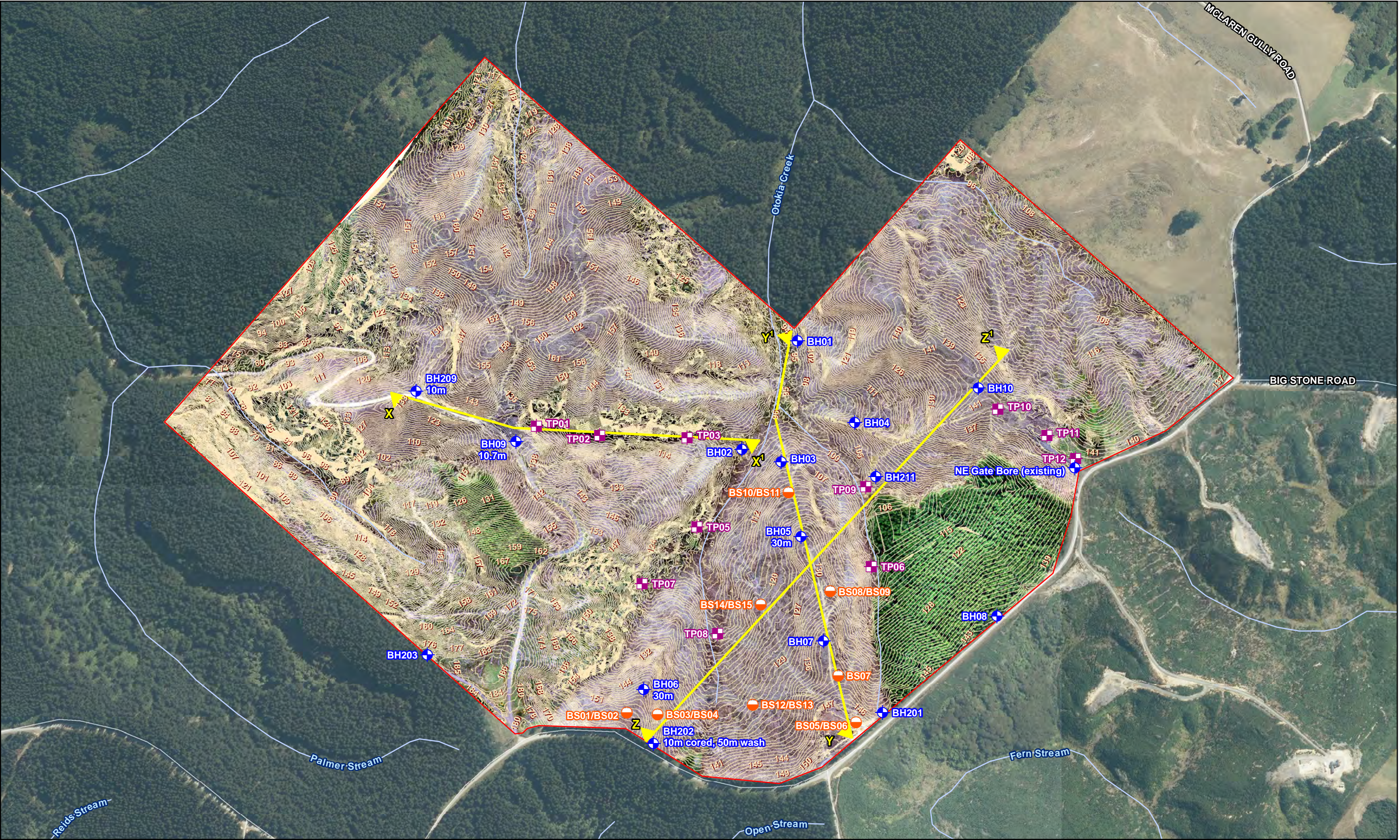
Site conditions (including the presence of hazardous substances and/or site contamination) may change after the date of this Report. GHD does not accept responsibility arising from, or in connection with, any change to the site conditions. GHD is also not responsible for updating this report if the site conditions change.

# Appendices




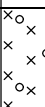











# Appendix A – Plans










## Appendix B – Borehole and Test Pit Logs and Photographs

		Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Dam Foundation Job Number: 12506381 Commenced: 6/06/2019						Hole No. : BH01a Sheet : 1 of 2 Hole Length : 15.00m Scale @ A4 : 1:50 Logged : MF Processed : HB Checked : JHS										
Easting: 396465.49		Northing: 788214.52		System: TAIETM2000														
RL: 96.01		Datum: NZVD2016																
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQR (%)	Defect Spacing (mm)	Instrumentation Installation	Water level	
							Number / Type	Result										
95	0		Gravelly SILT; trace fine to coarse sand, trace clay; light yellow-brown and orange-brown. Stiff, moist, low plasticity; gravel, fine to medium, sub-angular to sub-rounded or quartz and schist (COLLUVIUM)	COLLUVIUM	M	St					PQTT			77			0	
94	1		1.20 - 2.70 m: CORELOSS Possible soft material comprising slip base & stream alluvium lost.								PQTT				20			1
93	2		Silty CLAY, trace fine sand; grey and orange-brown. Soft to firm, moist, high plasticity (ALLUVIUM)	ALLUVIUM	M	S-F					PQTT			0			2	
92	3		Silty fine to coarse SAND, trace organics; grey. Poorly graded / 3.00 - 3.90 m: CORELOSS (inferred depth)								PQTT							3
91	4		Slightly weathered, light grey fine to coarse SANDSTONE; moderately strong to strong, no defects (HENLEY BRECCIA)	HENLEY BRECCIA							PQTT			100 35 22			4	
90	4.4		Slightly weathered, grey SILTSTONE; very weak to weak no defects								PQTT				98 98 80			5
89	4.8		Slightly weathered, light grey fine to coarse SANDSTONE; very weak to weak, no defects								PQTT				100 100 100			6
88	5		5.25 - 5.28 m: black organic-rich layer 5.28 - 5.38 m: moderately strong to strong 5.38 - 6.00 m: very weak to weak								PQTT							7
87	6		6.00 m: moderately strong to strong, closey spaced black organic-rich laminates								PQTT							8
	6.8		Slightly weathered, light yellow-brown and red-brown SILTSTONE; very weak to weak, no defects 6.90 - 7.05 m: light grey and red-brown 7.05 - 7.30 m: light grey with purple-brown layersvery closely spaced purple interlaminaes 7.30 - 7.40 m: gravelly fine to medium SANDSTONE; gravel is fine								PQTT				53 53 53			9
	7.8		Slightly weathered, light grey with purple-brown laminates, fine to medium SANDSTONE; very weak to weak, no defects								PQTT							10
	8.3		8.30 - 9.00 m: CORELOSS (possible sandy gravel)								PQTT							
	9		Slightly weathered, grey and brown SILTSTONE; very weak to weak, no defects								PQTT				100 100 100			
	9.5		Slightly weathered, light grey with purple-brown laminates fine to medium SANDSTONE; very weak to weak, no defects								PQTT							
Notes and Comments: End of Hole @ 15.00m, Target Depth.  Ground stripped by ~0.6 m, including all topsoil to construct drill pad. Piezo installed 10/06/2019.  Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical		Orientation:		Ground Water Level										
				Contractor: McNeills		Equipment: UDR600 (truck mounted)		Shear Vane Id:		Date	Time	Reading (mbgl)	Hole depth (mbgl)					

		Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Dam Foundation Job Number: 12506381 Commenced: 6/06/2019 Completed: 6/06/2019						Hole No. : BH01a Sheet : 2 of 2 Hole Length : 15.00m Scale @ A4 : 1:50 Logged : MF Processed : HB Checked : JHS									
Easting: 396465.49 RL: 96.01		Northing: 788214.52 Datum: NZVD2016		System: TAIETM2000													
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect Spacing (mm)	Instrumentation Installation	Water level
							Number / Type	Result									
85	11		Slightly weathered, light grey with purple-brown laminates fine to medium SANDSTONE; very weak to weak, no defects (continued from layer starting at 9.5m ) 10.50 - 11.30 m: very thinly bedded	HENLEY BRECCIA						PQTT		SW		100 100 100			
84	12		Unweathered, moderately thickly bedded, grey BRECCIA; moderately strong to strong, no defects. Clasts; fine to medium, angular to sub-rounded; quartz and schist; matrix supported, coarse sand matrix 11.70 - 12.20 slightly gravelly fine to coarse gravel SANDSTONE 12.20 m: medium to coarse gravel size clasts, clast supported							PQTT				97 97 97			
83	13		12.80 m: fewer clasts, fine to medium gravel size, matrix supported 13.10 m: weak to moderately strong; fine to coarse gravel clasts, clast supported; moderately well indurated							PQTT		UW		100 100 100			
82	14		13.50 - 15.00 m: CORELOSS Driller unable to recover core - slipping out of barrel.							PQTT				0 0 0			
81	15		End of Hole @ 15.00m, Target Depth.														
80	16																
79	17																
78	18																
77	19																
Notes and Comments: End of Hole @ 15.00m, Target Depth.  Ground stripped by ~0.6 m, including all topsoil to construct drill pad. Piezo installed 10/06/2019.  Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical		Orientation:		Ground Water Level				Date	Time	Reading (mbgl)	Hole depth (mbgl)		
				Contractor: McNeills		Equipment: UDR600 (truck mounted)											
				Shear Vane Id:													

			Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Dam Foundation Job Number: 12506381 Commenced: 6/06/2019						Hole No. : BH01b Sheet : 1 of 2 Hole Length : 15.00m Scale @ A4 : 1:50 Logged : MF Processed : HB Checked : JHS								
Easting: 396465.49			Northing: 788214.52			System: TAIETM2000											
RL: 96.01			Datum: NZVD2016														
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SQR RQR (%)	Defect Spacing (mm)	Instrumentation Installation	Water level
			Gravelly SILT; trace fine to coarse sand, trace clay; light yellow-brown and orange-brown. Stiff, moist, low plasticity; gravel, fine to medium, sub-angular to sub-rounded or quartz and schist (COLLUVIUM)	COLLUVIUM	M	St				PQTT				77			
			1.20 - 2.70 m: CORELOSS Possible soft material comprising slip base & stream alluvium lost.							PQTT				20			
			Silty CLAY, trace fine sand; grey and orange-brown. Soft to firm, moist, high plasticity (ALLUVIUM)	ALLUVIUM	M	S-F				PQTT				0			
			Silty fine to coarse SAND, trace organics; grey. Poorly graded / 3.00 - 3.90 m: CORELOSS (inferred depth)							PQTT				100 35 22			
			Slightly weathered, light grey fine to coarse SANDSTONE; moderately strong to strong, no defects (HENLEY BRECCIA)	HENLEY BRECCIA						PQTT				98 98 80			
			Slightly weathered, grey SILTSTONE; very weak to weak no defects							PQTT				100 100 100			
			Slightly weathered, light grey fine to coarse SANDSTONE; very weak to weak, no defects 5.25 - 5.28 m: black organic-rich layer 5.28 - 5.38 m: moderately strong to strong 5.38 - 6.00 m: very weak to weak							PQTT				53 53 53			
			6.00 m: moderately strong to strong, closey spaced black organic-rich laminates							PQTT				100 100 100			
			Slightly weathered, light yellow-brown and red-brown SILTSTONE; very weak to weak, no defects 6.90 - 7.05 m: light grey and red-brown 7.05 - 7.30 m: light grey with purple-brown layersvery closely spaced purple interlaminaes 7.30 - 7.40 m: gravelly fine to medium SANDSTONE; gravel is fine							PQTT				100 100 100			
			Slightly weathered, light grey with purple-brown laminates, fine to medium SANDSTONE; very weak to weak, no defects 8.30 - 9.00 m: CORELOSS (possible sandy gravel)							PQTT				100 100 100			
			Slightly weathered, grey and brown SILTSTONE; very weak to weak, no defects							PQTT				100 100 100			
			Slightly weathered, light grey with purple-brown laminates fine to medium SANDSTONE; very weak to weak, no defects							PQTT				100 100 100			
Notes and Comments:				Inclination: Vertical				Orientation:				Ground Water Level					
End of Hole @ 15.00m, Target Depth.				Contractor: McNeills				Date				Time					
Looks like drill pad on slip debris pile. Scarp above (east) of pad. Ground stripped ~0.6 m, including all topsoil. Piezo installed 10/06/2019.				Equipment: UDR600 (truck mounted)				Reading (mbgl)				Hole depth (mbgl)					
Refer to explanation sheets for abbreviation and symbols				Shear Vane Id:													

<div></div> <div>Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Dam Foundation Job Number: 12506381 Commenced: 6/06/2019 Completed: 6/06/2019</div>										Hole No. : BH01b Sheet : 2 of 2 Hole Length : 15.00m Scale @ A4 : 1:50 Logged : MF Processed : HB Checked : JHS																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
Easting: 396465.49 RL: 96.01					Northing: 788214.52 Datum: NZVD2016					System: TAIETM2000																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
<div><div>RL (m)</div><div>Depth (m)</div><div>Graphic</div></div>										<div>Material Description</div>										<div>Geological Unit</div>										<div>Moisture condition</div>										<div>Consistency / Relative density</div>										<div>Sample</div> <div>Number / Type</div> <div>Result</div>										<div>Casing</div>										<div>Method</div>										<div>Flush Return (%)</div>										<div>Weathering</div>										<div>Estimated Strength (MPa)</div>										<div>TCR SCR RQD (%)</div>										<div>Defect Spacing (mm)</div>										<div>Instrumentation Installation</div>										<div>Water level</div>																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																					
<div><div>85</div><div>11</div><div>11.3</div></div>										<div>Slightly weathered, light grey with purple-brown laminates fine to medium SANDSTONE; very weak to weak, no defects (continued from layer starting at 9.5m ) 10.50 - 11.30 m: very thinly bedded</div>										HENLEY BRECCIA																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																													



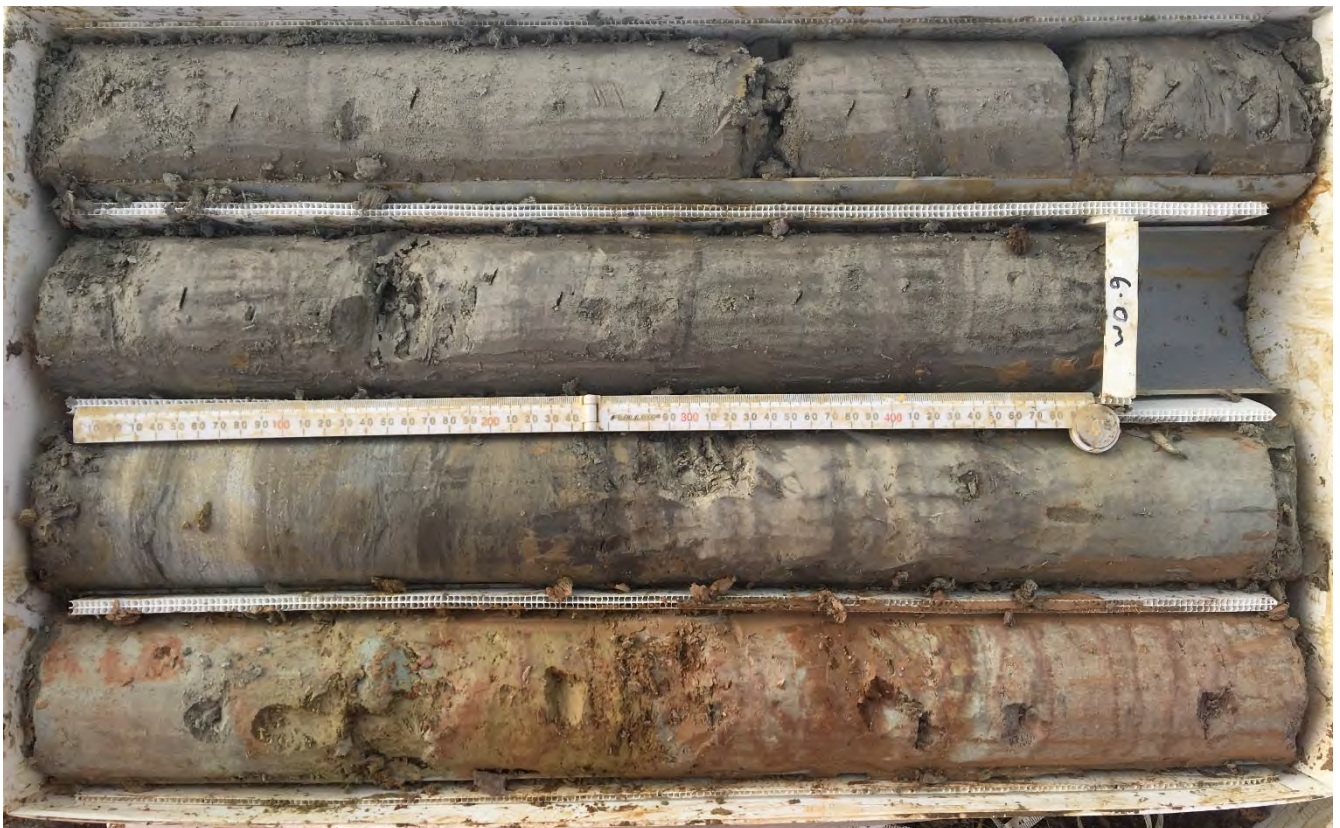


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Project	Smooth Hill Landfill Consenting	
Client	Dunedin City Council	
Job number	12506381	Page 1 of 3
Borehole ID	BH01	



Box 1 of 5: 0.00 m to 4.80 m



Box 2 of 5: 4.80 m to 7.20 m



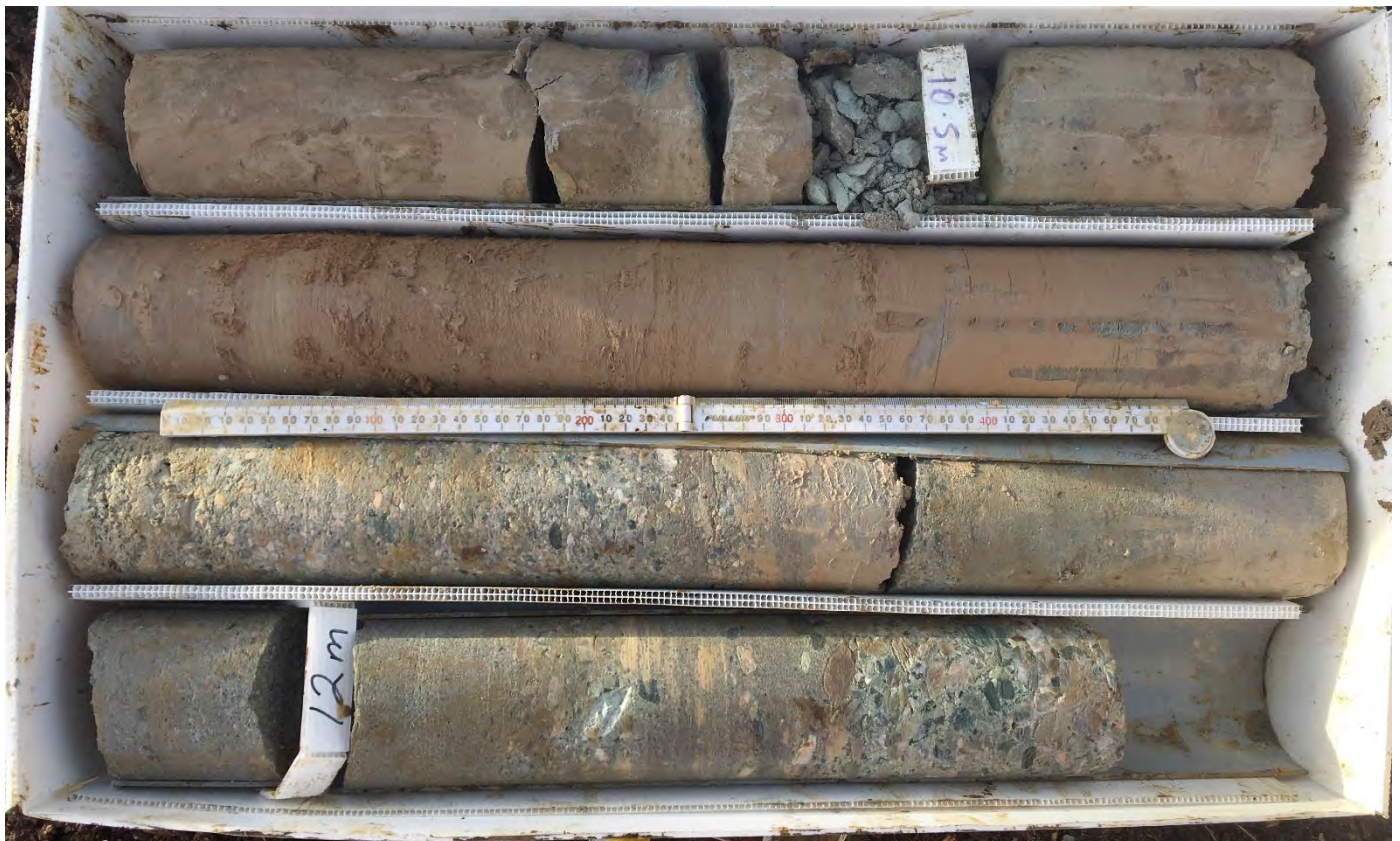


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Project	Smooth Hill Landfill Consenting	
Client	Dunedin City Council	
Job number	12506381	Page 2 of 3
Borehole ID	BH01	



Box 3 of 5: 8.20 m to 10.20 m



Box 4 of 5: 10.20 m to 12.40 m







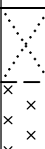
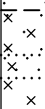










Project	Smooth Hill Landfill Consenting	
Client	Dunedin City Council	
Job number	12506381	Page 3 of 3
Borehole ID	BH01	



Box 5 of 5: 12.40 m to 15.00 m (EOH)



			Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Toe Bund Foundation Job Number: 12506381 Commenced: 27/05/2019 Completed: 28/05/2019						Hole No. : BH02a Sheet : 2 of 2 Hole Length : 15.00m Scale @ A4 : 1:50 Logged : MF Processed : HB Checked : JHS								
Easting: 396358.59 RL: 97.41			Northing: 788022.89 Datum: NZVD2016			System: TAIETM2000											
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect Spacing (mm)	Instrumentation Installation	Water level
187		△	Unweathered, bedded, grey BRECCIA; weak to moderately strong, no defects; clasts: fine to medium gravel, quartz and schist, angular to subrounded; coarse sand matrix ( <i>continued from layer starting at 6.6m</i> )	HENLEY BRECCIA					122mm	PQTT							
11		△	11.00 - 11.80 m: weak to moderately strong							PQTT				100 100 100			
12		△	11.80 - 12.40 m: clast size fine to medium gravel, more matrix dominated, moderately strong to strong							PQTT				100 100 100			
13		△	12.40 - 13.50 m: matrix silt to coarse coarse sand, minor to some clay; weak to moderately strong clasts fine to corase gravel size							PQTT		UW		100 100 100			
14		△	13.50 - 15.00 m: matrix silt to coarse sand; moderately strong to strong							PQTT				100 100 100			
15			End of Hole @ 15.00m, Target Depth.														
16																	
17																	
18																	
19																	
Notes and Comments: End of Hole @ 15.00m, Target Depth. Groundwater SWL at 0.23 mbgl (31/05/2019). Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical Orientation: Contractor: McNeills Equipment: UDR600 (truck mounted) Shear Vane Id: GEO2288				Ground Water Level Date: 28/05/19 Time: 00:00 Reading (mbgl): 0.7 Hole depth (mbgl): 15									

			Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Toe Bund Foundation Job Number: 12506381 Commenced: 27/05/2019 Completed: 28/05/2019						Hole No. : BH02b Sheet : 1 of 2 Hole Length : 15.00m Scale @ A4 : 1:50 Logged : MF Processed : HB Checked : JHS								
Easting: 396358.59 RL: 97.41			Northing: 788022.89 Datum: NZVD2016			System: TAIETM2000											
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect Spacing (mm)	Instrumentation Installation	Water level
							Number / Type	Result									
97	0		0.00 - 0.50 m: CORELOSS (inferred depth)	LOESS													
96	0.5		SILT, minor clay, trace to minor fine sand; light grey brown. Stiff to very stiff, moist, non-plastic. Contains iron weathered spots and small (<50mm) lenses of iron staining (LOESS)		M	St-Vst					PQTT				67		
95	1.5		1.50 - 2.15 m: CORELOSS (inferred depth)	ALLUVIUM				SV@1.5m 42/5									
94	2.15		Fine sandy SILT, trace clay; light grey. Firm, moist, non-plastic		M	F					PQTT				50		
93	2.62		Silty fine SAND; grey. 'Very loose to loose', saturated, poorly graded (HISTORIC [PRE GLACIAL] ALLUVIUM)		S	F											
92	2.9		Fine sandy SILT; grey. Firm, moist, non-plastic		M	F			SV@2.8m								
91	3.35		2.80 m: black fibrous organic material (roots)	HENLEY BRECCIA													
90	3.7		2.90 - 3.35 m: CORELOSS (inferred depth) (possible gravel)		S						PQTT				70 70 70		
89	3.95		Fine GRAVEL; light grey. Gravel: quartz and schist, angular to sub-angular. Fine matrix washed away (HENLEY BRECCIA - MODERATELY WEATHERED)		D												
88	4.3		Fine to coarse SAND, minor to some fine gravel; grey. 'Very dense', dry, non-plastic; gravel: angular to sub-rounded quartz and schist					SV@4.3m UTP									
87	4.60		Unweathered, moderately thinly to moderately thickly interbedded, grey SILTSTONE and fine grained SANDSTONE; very weak; uniform grainsize within layers														
86	5.30		4.60 VN, 30°, QZ														
85	5.70		5.30 - 5.70 Unweathered, thinly interbedded grey BRECCIA (60%) and fine to medium grained grey SANDSTONE 40%; strong. Clasts: fine to medium gravel, angular of quartz and schist. Coarse sand matrix.														
84	6.11		Silty CLAY; dark grey. Firm to stiff, moist, high plasticity														
83	6.56		Unweathered, moderately thickly interbedded, grey fine SANDSTONE and SILTSTONE; very weak, poorly graded														
82	6.80		6.11 m: 10 mm lignite														
81	7.00		Unweathered, grey silty fine SANDSTONE; weak to moderately strong														
80	7.20		Unweathered, bedded, grey BRECCIA; weak to moderately strong, no defects; clasts: fine to medium gravel, quartz and schist, angular to subrounded; coarse sand matrix														
79	7.80																
78	8.20																
77	8.80																
76	9.20		9.10 - 9.20 m: SILT, grey; hard, dry														
75	9.80		9.20 - 11.00 m: moderately strong to strong														
74	10.00		9.80 m: clast size increased to fine to coarse gravel														
Notes and Comments: End of Hole @ 15.00m, Target Depth. Groundwater SWL at 0.21 mbgl (31/05/2019). Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical		Orientation:		Ground Water Level									
				Contractor: McNeills		Equipment: UDR600 (truck mounted)		Shear Vane Id: GEO1826		Date		Time		Reading (mbgl)		Hole depth (mbgl)	







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Project	Smooth Hill Landfill Consenting	
Client	Dunedin City Council	
Job number	12506381	Page 1 of 3
Borehole ID	BH02	



Box 1 of 6: 0.0 m to 3.7 m



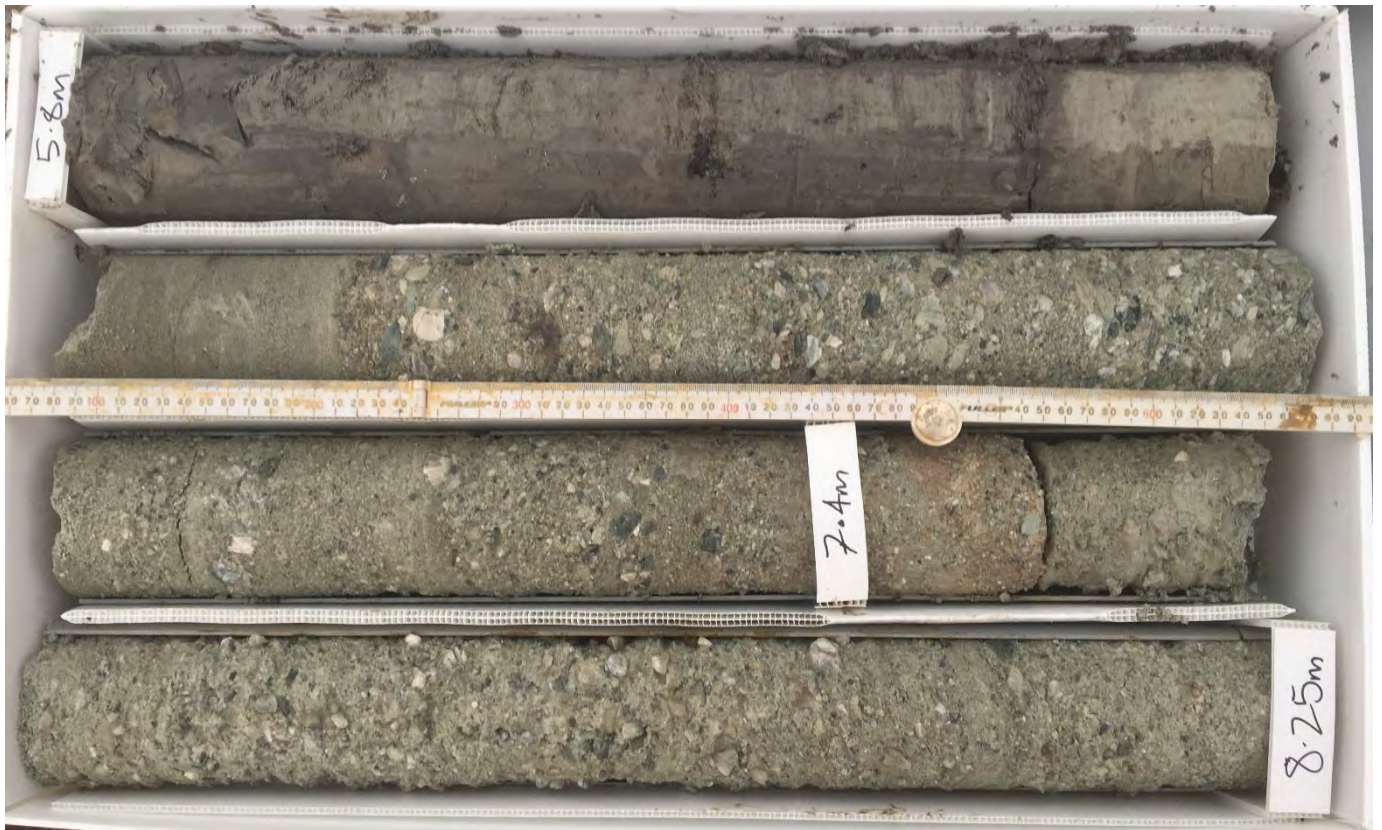
Box 2 of 6: 3.7 m to 5.8 m



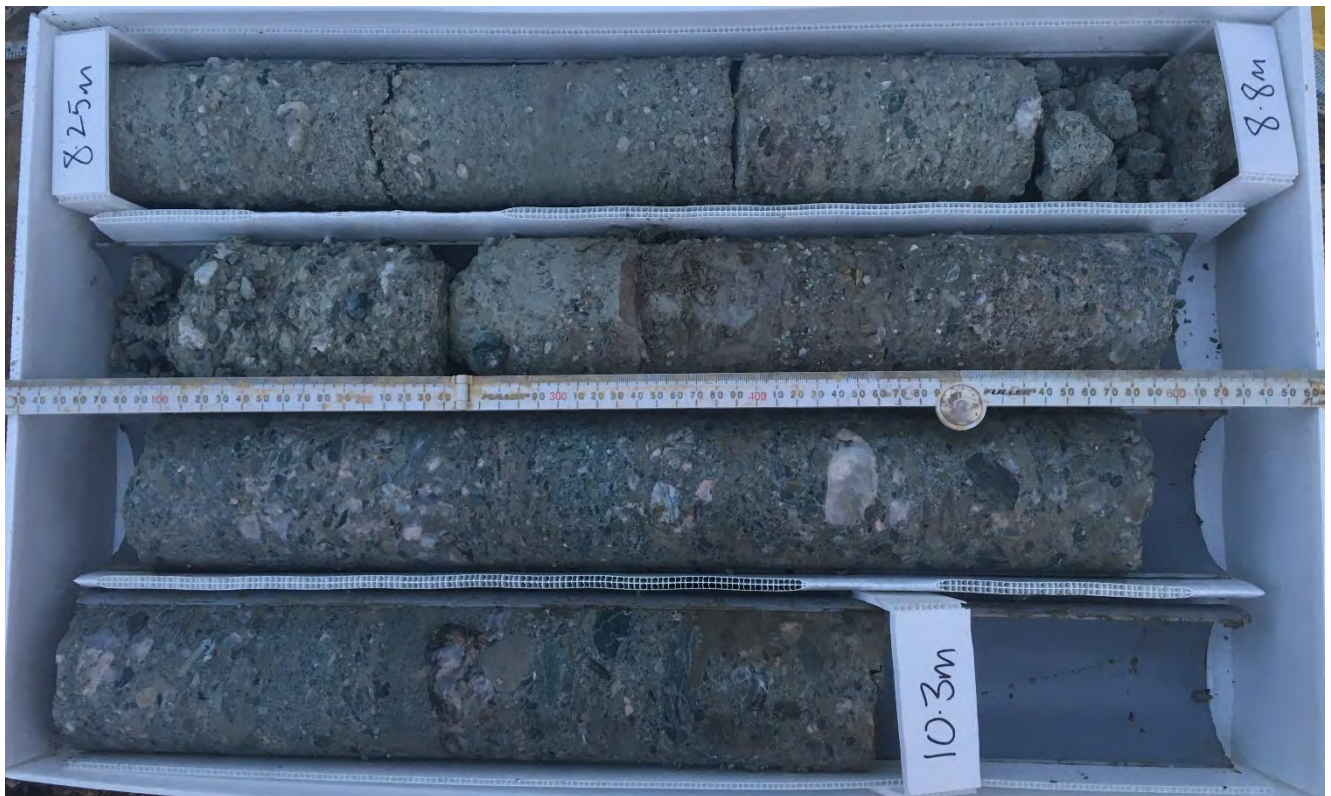


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Project	Smooth Hill Landfill Consenting	
Client	Dunedin City Council	
Job number	12506381	Page 2 of 3
Borehole ID	BH02	



Box 3 of 6: 5.8 m to 8.25 m



Box 4 of 6: 8.25 m to 10.3 m



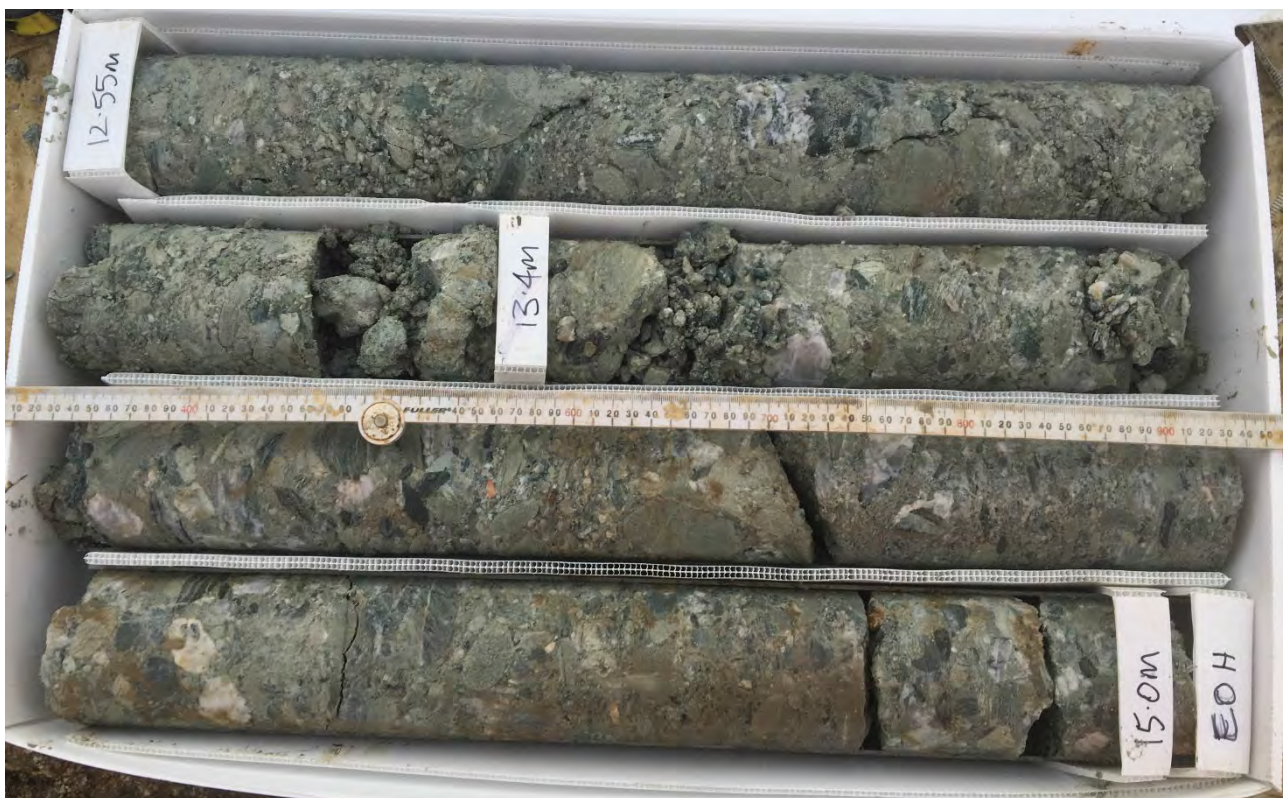


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Project	Smooth Hill Landfill Consenting	
Client	Dunedin City Council	
Job number	12506381	Page 3 of 3
Borehole ID	BH02	




Box 5 of 6: 10.3 m to 12.55 m




Box 6 of 6: 12.55 m to 15.0 m (EOH)






<div></div> <div>Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Toe Bund/Central Ridge Job Number: 12506381 Commenced: 28/05/2019 Completed: 29/05/2019</div>										Hole No. : BH03a Sheet : 2 of 2 Hole Length : 20.00m Scale @ A4 : 1:50 Logged : MF Processed : HB Checked : JHS									
Easting: 396428.38 RL: 107.48					Northing: 787998.34 Datum: NZVD2016					System: TAIETM2000									
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SQR RQD (%)	Defect Spacing (mm)	Instrumentation Installation	Water level		
							Number / Type	Result											
107.48	0.0	X	Unweathered, grey fine to medium SANDSTONE; very weak to weak, no defects	HENLEY BRECCIA	D-M	VS-t-H			112mm	PQTT		UW		100 100 100					
11.000	0.5	X	SILT, minor clay; brown. Very stiff to hard, dry to moist, non-plastic (Possible relic Topsoil)																
10.7	1.0	X	Unweathered, light grey SILTSTONE; very weak to weak, no defects																
10.6	1.5	△	Unweathered, light grey and white BRECCIA; moderately strong to strong; clasts: quartz and schist, fine to medium gravel size, angular to subangular. Matrix supported, coarse sand matrix																
10.6	2.0	△																	
10.6	2.5	△																	
10.6	3.0	△																	
10.6	3.5	△																	
10.6	4.0	△																	
10.6	4.5	△																	
10.6	5.0	△																	
10.6	5.5	△	Unweathered, light grey fine SANDSTONE; moderately strong to strong; no defects 13.79 m: 30 mm SILT layer; stiff																
10.6	6.0	△	Unweathered, moderately thinly to moderately thickly bedded, light grey and white BRECCIA; moderately strong to strong; clasts; quartz & schist, fine to medium gravel size, angular to sub angular. Matrix supported; coarse sand matrix. Distinct beds of fine and coarse clasts	D-M	'D'			PQTT		94 94 94		100 100 100							
10.6	6.5	△																	
10.6	7.0	△																	
10.6	7.5	△																	
10.6	8.0	△																	
10.6	8.5	△																	
10.6	9.0	△																	
10.6	9.5	△																	
10.6	10.0	△																	
10.6	10.5	△																	
10.6	11.0	△	Fine to medium SAND; grey. 'Dense', dry to moist, poorly graded, non lithified (SANDSTONE: Extremely weak to very weak)																
10.6	11.5	△	Unweathered, light grey, fine SANDSTONE, moderately strong to strong, no defects																
10.6	12.0	△	Unweathered, grey BRECCIA; moderately strong to strong; clasts: quartz and schist, fine to medium grained sized; angular to sub-angular; clast supported 18.70 - 19.25 m: clasts fine to coarse gravel sized	PQTT		100 100 100													
10.6	12.5	△																	
10.6	13.0	△	Unweathered, light grey fine SANDSTONE; moderately strong to strong  19.40 JT, 15°, pl, r, VN, VE, Fe-stain	PQTT		100 100 100													
10.6	13.5	△																	

Notes and Comments: End of Hole @ 20.00m, Target Depth. Groundwater SWL at 3.9 mbgl during piezo install. Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical Orientation: Contractor: McNeills Equipment: UDR600 (truck mounted) Shear Vane Id: GEO2288				Ground Water Level Date: 29/05/19 Time: 00:00 Reading (mbgl): 3.9 Hole depth (mbgl): 20			
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		Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Toe bund/Central Ridge Job Number: 12506381 Commenced: 28/05/2019 Completed: 29/05/2019						Hole No. : BH03b Sheet : 1 of 2 Hole Length : 20.00m Scale @ A4 : 1:50 Logged : MF Processed : HB Checked : JHS										
Easting: 396428.38 RL: 107.48		Northing: 787998.34 Datum: NZVD2016		System: TAIETM2000														
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Number / Type	Result	Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQR (%)	Defect Spacing (mm)	Instrumentation Installation	Water level	
107.0	0.2		TOPSOIL: silt, minor clay; light brown and grey. Very stiff, moist, low plasticity; organics mixed in soil	LOESS	M	VSt	SV@1.5m 194 kPa			PQTT				73				
106.5	1.9		Clayey SILT, trace fine sand, trace fine gravel; yellow-brown and light grey. Very stiff to hard, moist, high plasticity (LOESS)			VSt-H												
105.5	2.3		SILT, minor clay, minor fine sand; light grey and orange-brown. Very stiff to hard, dry to moist, low plasticity		D-M	VSt-H												
105.0	2.6		SILT, minor fine sand, trace clay; light grey and orange-brown. Hard, dry, non-plastic. Variable iron staining (HENLEY BRECCIA)	HENLEY BRECCIA	D	H				PQTT								
104.5	3.2		2.60 - 3.20 m: CORELOSS (inferred depth)							PQTT		CW		60				
104.0	3.65		Silty sandy coarse GRAVEL; brown. Well graded; clasts: angular to sub-angular quartz and schist. Completely weathered breccia							PQTT								
103.5	3.85		Fine to coarse GRAVEL, some silt, minor fine to coarse sand; purple. Dry, well graded; clasts: subangular to angular, quartz and schist (completely weathered breccia)							PQTT								
103.0	5.7									PQTT								
102.5	6.26		Highly weathered, red-grey BRECCIA; moderately strong; clasts, fine to coarse gravel, quartz and schist, matrix supported		M					PQTT		HW						
101.5	6.6		Fine to coarse GRAVEL, some silt, minor fine to coarse sand; purple. Dry, well graded; clasts: sub angular to angular, quartz and schist; highly weathered breccia								PQTT				100			
101.0	7.5		SILT, minor fine to medium sand, trace to minor fine gravel; grey and yellow-brown. Very stiff, moist, non-plastic								PQTT							
100.5	7.9		Fine to medium GRAVEL; white and grey. Well graded; clasts: angular to subangular, quartz and schist (matrix lost during drilling)		D	H				PQTT								
100.0	8.45		SILT, trace fine gravel; brown-grey-light purple. Hard, dry, non plastic.								PQTT							
99.5	8.9		Unweathered, dark-grey SILTSTONE; weak to moderately strong, no defects								PQTT					100 100 100		
99.0	9.2		Unweathered, grey fine SANDSTONE; weak to moderately strong; no defects							PQTT								
98.5	9.2		8.90 - 9.20 m: CORELOSS (inferred depth) Likely 'loose' sand layer washed away							PQTT					73 73 73			
98.0			Unweathered, moderately thickly interbedded, dark grey and grey SILTSTONE and SANDSTONE; weak to moderately strong, no defects							PQTT								
Notes and Comments: End of Hole @ 20.00m, Target Depth. Groundwater SWL at 12.6 mbgl during piezo install. Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical		Orientation:		Ground Water Level										
				Contractor: McNeills		Equipment: UDR600 (truck mounted)		Shear Vane Id: GEO2288		Date		Time		Reading (mbgl)		Hole depth (mbgl)		

Groundwater level at the end of drilling

<div></div> <div>Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Toe bund/Central Ridge Job Number: 12506381 Commenced: 28/05/2019 Completed: 29/05/2019</div>										Hole No. : BH03b Sheet : 2 of 2 Hole Length : 20.00m Scale @ A4 : 1:50 Logged : MF Processed : HB Checked : JHS									
Easting: 396428.38 RL: 107.48					Northing: 787998.34 Datum: NZVD2016					System: TAIETM2000									
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQR (%)	Defect Spacing (mm)	Instrumentation Installation	Water level		
							Number / Type	Result											
107.48	0.0	X	Unweathered, grey fine to medium SANDSTONE; very weak to weak, no defects	HENLEY BRECCIA	D-M	VSt-H			112mm	PQTT		UW		100					
11.00	0.9	X	SILT, minor clay; brown. Very stiff to hard, dry to moist, non-plastic (Possible relic Topsoil)													100			
11.00	1.0	X	Unweathered, light grey SILTSTONE; very weak to weak, no defects													100			
11.00	1.1	Δ	Unweathered, light grey and white BRECCIA; moderately strong to strong; clasts: quartz and schist, fine to medium gravel size, angular to subangular. Matrix supported, coarse sand matrix													100			
11.00	1.2	Δ														100			
11.00	1.3	Δ														100			
11.00	1.4	Δ														100			
11.00	1.5	Δ														100			
11.00	1.6	Δ														100			
11.00	1.7	Δ														100			
13.65	13.65	X	Unweathered, light grey fine SANDSTONE; moderately strong to strong; no defects 13.79 m: 30 mm SILT layer; stiff	HENLEY BRECCIA	D-M	'D'			112mm	PQTT		UW		94					
14.2	14.2	Δ	Unweathered, moderately thinly to moderately thickly bedded, light grey and white BRECCIA; moderately strong to strong; clasts: quartz & schist, fine to medium gravel size, angular to sub angular. Matrix supported; coarse sand matrix. Distinct beds of fi													94			
14.2	14.3	Δ														94			
14.2	14.4	Δ														94			
14.2	14.5	Δ														94			
14.2	14.6	Δ														94			
14.2	14.7	Δ														94			
14.2	14.8	Δ														94			
14.2	14.9	Δ														94			
14.2	15.0	Δ														94			
17.2	17.2	X	Fine to medium SAND; grey. 'Dense', dry to moist, poorly graded, non lithified (SANDSTONE: Extremely weak to very weak)	HENLEY BRECCIA	D-M	'D'			112mm	PQTT		UW		100					
17.6	17.6	X	Unweathered, light grey, fine SANDSTONE, moderately strong to strong, no defects													100			
17.6	17.7	Δ														100			
17.6	17.8	Δ														100			
17.6	17.9	Δ														100			
17.6	18.0	Δ														100			
17.6	18.1	Δ														100			
17.6	18.2	Δ														100			
17.6	18.3	Δ														100			
17.6	18.4	Δ														100			
18.25	18.25	Δ	Unweathered, grey BRECCIA; moderately strong to strong; clasts: quartz and schist, fine to medium grained sized; angular to sub-angular; clast supported 18.70 - 19.25 m: clasts fine to coarse gravel sized	HENLEY BRECCIA	D-M	'D'			112mm	PQTT		UW		100					
18.25	18.3	Δ														100			
18.25	18.4	Δ														100			
18.25	18.5	Δ														100			
18.25	18.6	Δ														100			
18.25	18.7	Δ														100			
18.25	18.8	Δ														100			
18.25	18.9	Δ														100			
18.25	19.0	Δ														100			
18.25	19.1	Δ														100			
19.25	19.25	Δ	Unweathered, light grey fine SANDSTONE; moderately strong to strong  19.40 JT, 15°, pl, r, VN, VE, Fe-stain	HENLEY BRECCIA	D-M	'D'			112mm	PQTT		UW		100					
19.25	19.3	Δ														100			
19.25	19.4	Δ														100			
19.25	19.5	Δ														100			
19.25	19.6	Δ														100			
19.25	19.7	Δ														100			
19.25	19.8	Δ														100			
19.25	19.9	Δ														100			
19.25	20.0	Δ														100			
19.25	20.1	Δ														100			
Notes and Comments: End of Hole @ 20.00m, Target Depth. Groundwater SWL at 12.6 mbgl during piezo install. Refer to explanation sheets for abbreviation and symbols										Inclination: Vertical Orientation: Contractor: McNeills Equipment: UDR600 (truck mounted) Shear Vane Id: GEO2288				Ground Water Level Date: 29/05/19 Time: 00:00 Reading (mbgl): 3.9 Hole depth (mbgl): 20					





Project	Smooth Hill Landfill Consenting	
Client	Dunedin City Council	
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Box 1 of 8: 0.0 m to 2.6 m



Box 2 of 8: 2.6 m to 6.25 m





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Project	Smooth Hill Landfill Consenting	
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Box 3 of 8: 6.25 m to 9.25 m



Box 4 of 8: 9.25 m to 11.05 m





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Project	Smooth Hill Landfill Consenting	
Client	Dunedin City Council	
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Box 5 of 8: 11.05 m to 13.3 m



Box 6 of 8: 13.3 m to 15.6 m



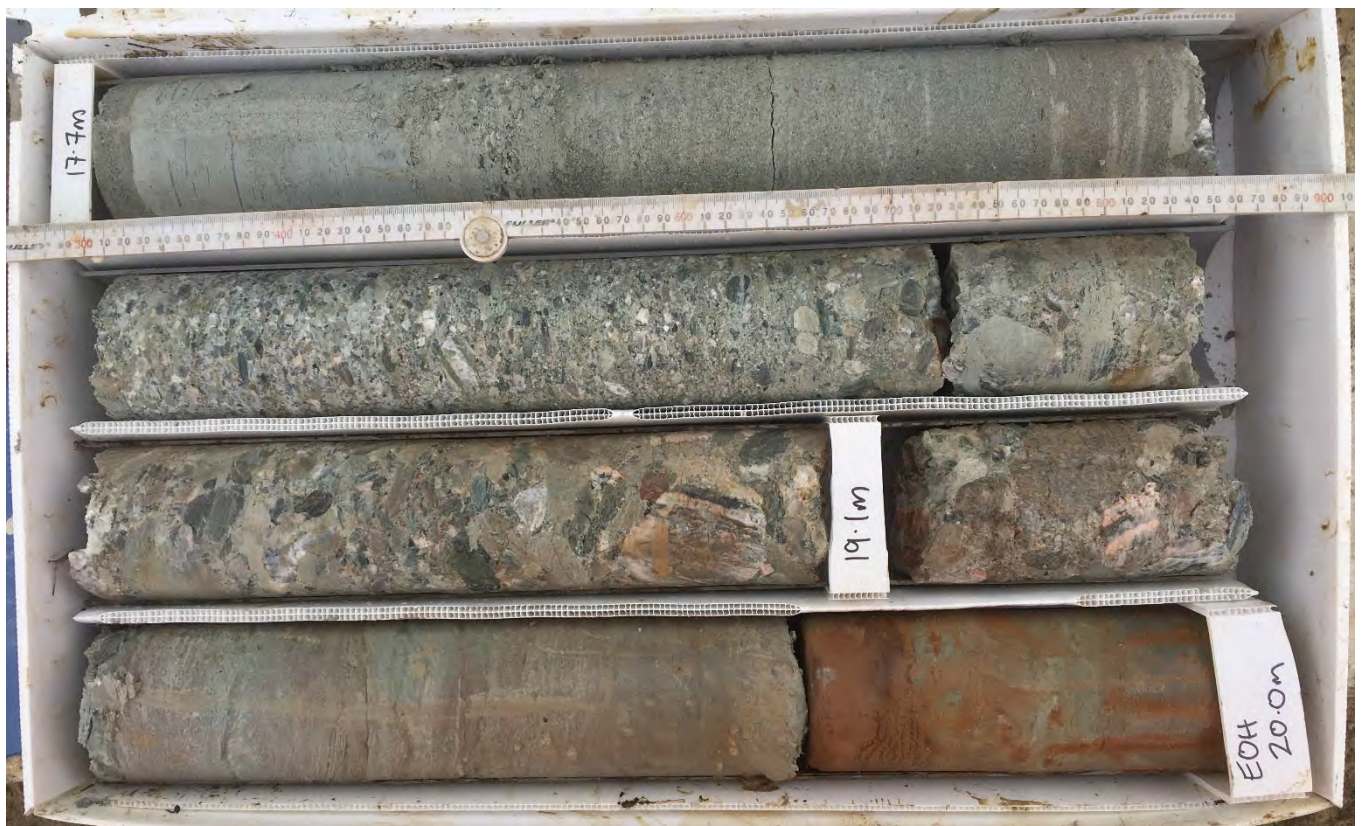


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Project	Smooth Hill Landfill Consenting	
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Box 7 of 8: 15.6 m to 17.7 m



Box 8 of 8: 17.7 m to 20.0 m (EOH)





		Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Toe Bund Foundation Job Number: 12506381 Commenced: 6/06/2019 Completed: 7/06/2019						Hole No. : BH04a Sheet : 2 of 2 Hole Length : 15.00m Scale @ A4 : 1:50 Logged : MF Processed : HB Checked : JHS										
Easting: 396563.6 RL: 108.15		Northing: 788063.75 Datum: NZVD2016		System: TAIETM2000														
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQR (%)	Defect Spacing (mm)	Instrumentation Installation	Water level	
							Number / Type	Result										
108.15	10.1	△	Slightly weathered, grey BRECCIA; moderately strong to strong; very widely spaced defects; well indurated; clasts: fine to medium gravel, quartz and schist, sub-angular to sub-rounded; fine to coarse sand matrix, matrix supported	HENLEY BRECCIA						PQTT				83 50 50				
107.8	10.6	△	Slightly weathered, light grey fine to coarse SANDSTONE; very weak to weak; no defects							PQTT				97 97 97				
107.1	11.1	△	Slightly weathered, brown SILTSTONE; very weak to weak; no defects (Possible Relic Topsoil) 11.00 m: light grey															
106.6	11.6	△	Slightly weathered, light grey and pink-grey BRECCIA; weak to moderately strong; no defects; moderately well indurated; clasts: fine to coarse gravel, quartz and schist, sub-angular to angular, clast supported, clast size decreases with depth; fine to coarse sand matrix											88 64 56				
106.1	12.1	△	11.50 - 12.20 m: moderately strong to strong, fine to medium gravel clasts															
105.6	12.6	△	12.20 - 15.00 m: weak to moderately strong, fine to coarse gravel clasts															
105.1	13.1	△																
104.6	13.6	△																
104.1	14.1	△	13.90 m: clasts medium to coarse gravel								PQTT				100 73 73			
103.6	14.6	△																
103.1	15.1	△	End of Hole @ 15.00m, Target Depth.															
102.6	15.6																	
102.1	16.1																	
101.6	16.6																	
101.1	17.1																	
100.6	17.6																	
100.1	18.1																	
99.6	18.6																	
99.1	19.1																	
98.6	19.6																	
98.1	20.1																	
Notes and Comments: End of Hole @ 15.00m, Target Depth.  Groundwater at 2.52 mbtoc (TOC 0.53 m agl) - corrected groundwater at 1.99 mbgl 07/06/2019.  Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical		Orientation:		Ground Water Level				Date	Time	Reading (mbgl)	Hole depth (mbgl)			
				Contractor: McNeills		Equipment: UDR600 (truck mounted)						07/06/19	00:00	1.99	15			
				Shear Vane Id:														









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Project	Smooth Hill Landfill Consenting	
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Box 1 of 6: 0.0 m to 2.7 m

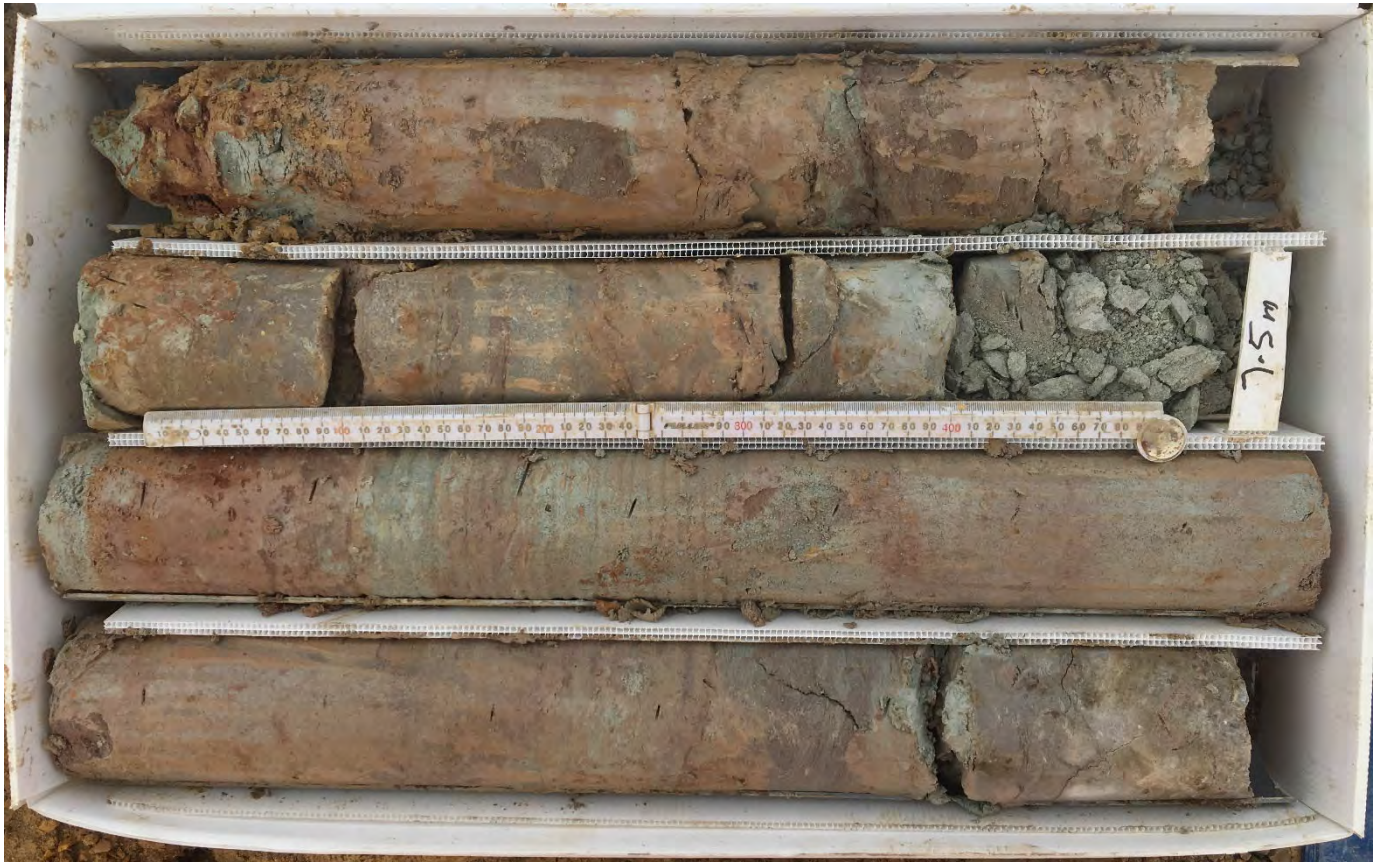


Box 2 of 6: 2.7 m to 5.9 m





Project	Smooth Hill Landfill Consenting	
Client	Dunedin City Council	
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Borehole ID	BH04	



Box 3 of 6: 5.9 m to 8.7 m



Box 4 of 6: 8.7 m to 11.1 m





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Project	Smooth Hill Landfill Consenting	
Client	Dunedin City Council	
Job number	12506381	Page 3 of 3
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Box 5 of 6: 11.1 m to 13.5 m




Box 6 of 6: 13.5 m to 15.0 m (EOH)





				Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Central Ridge Job Number: 12506381 Commenced: 29/05/2019 Completed: 30/05/2019				Hole No. : BH05a Sheet : 1 of 3 Hole Length : 30.00m Scale @ A4 : 1:50 Logged : MF Processed : HB Checked : JHS										
Easting: 396459.76 RL: 129.5		Northing: 787862.12 Datum: NZVD2016		System: TAIETM2000														
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect Spacing (mm)	Instrumentation Installation	Water level	
							Number / Type	Result										
129.0	0.0		Clayey SILT; grey and dark orange-brown. Very stiff, moist, high plasticity (LOESS)	LOESS	M	VSt	0.00			PQTT				100				
128.5	0.5		SILT, minor clay, trace fine sand; orange-brown mottled grey. Very stiff, moist, low plasticity		M	VSt												
128.0	1.0		Gravelly SILT, trace clay; orange-brown and white. Very stiff, moist, low plasticity; gravel is fine to medium, angular to sub-angular schist		M	VSt												
127.5	1.5		1.50 - 2.70 m: CORELOSS				1.20			PQTT				0				
127.0	2.0									PQTT								
126.5	2.5		Gravelly SILT, trace clay; orange-brown and white. Very stiff, moist, low plasticity. Gravel is fine to medium, angular to sub-angular schist. Completely to highly weathered rock (HENLEY BRECCIA)		M	VSt				PQTT				53				
126.0	3.0		2.80 - 3.50 m: CORELOSS							PQTT								
125.5	3.5		Fine to medium GRAVEL; cream/white. Well graded, gravel: quartz and schist, angular to sub-angular		M	VSt												
125.0	4.0		Gravelly SILT, trace clay; orange-brown and white. Very stiff, moist, low plasticity; iron staining on upper contact; gravel is fine to medium, angular to sub-angular, of quartz schist		M	VSt												
124.5	4.5		SILT, trace organics; light grey. Hard, dry to moist, non-plastic. Highly weathered rock		D-M	H												
124.0	5.0		Gravelly SILT, trace clay, trace organics; orange-brown and light grey. Very stiff, moist, non-plastic; gravel is fine to medium, angular to sub-angular, of quartz schist		D-M	H				PQTT				100				
123.5	5.5		SILT, trace clay, trace organics; light grey with orange-brown streaks. Hard, dry to moist, non-plastic															
123.0	6.0		5.10 m: orange-brown mottled light grey 5.35 m: 2-3 mm iron "gravel" beds 5.60 m: thinly laminated	HENLEY BRECCIA	M	St-VSt												
122.5	6.5		Silty CLAY, trace fine sand, trace organics; grey-brown. Stiff to very stiff, moist, high plasticity		M	H					PQTT				100			
122.0	7.0		SILT, trace clay, trace organics; light grey with black flecks. Hard, moist, non-plastic. Iron staining on lower contact		M	H												
121.5	7.5		SILT, trace medium gravel; grey-brown. Hard, moist, non-plastic; dark orange-brown iron stained contact at 6.80 m															
121.0	8.0		SILT, trace organics; light grey and orange-brown, with black flecks. Hard, dry, non-plastic, thin (<1mm) 'rusty' laminations		D	H				PQTT				100 93 87				
120.5	8.5		Moderately weathered, thinly bedded, dark grey with black streaks SILTSTONE; very weak to weak; moderately wide to widely spaced defects; contains organic-rich layers															
120.0	9.0		7.75 m: 50 mm fine grained sandstone interbed 7.90 - 8.50 m: with closely spaced lignite interlamination/ very thin interbeds															
119.5	9.5		8.50 m: slightly weathered 8.70 m: weak to moderately strong															
119.0	10.0		Slightly weathered, grey, SANDSTONE; weak to moderately strong; wide spaced defects; fine sand to coarse gravel size grains 9.60 m: 170 mm siltstone interbed							PQTT				93 93 93				
<b>Notes and Comments:</b> End of Hole @ 30.00m, Target Depth. ~ 0.5 m topsoil stripped to make drill pad Groundwater SWL at 16.4 mbgl during piezo install.  Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical		Orientation:		Ground Water Level				Date	Time	Reading (mbgl)	Hole depth (mbgl)			
				Contractor: McNeills		Equipment: UDR600 (truck mounted)												
				Shear Vane Id: GEO2288														

<div></div> <div>Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Central Ridge Job Number: 12506381 Commenced: 29/05/2019 Completed: 30/05/2019</div>										Hole No. : BH05a Sheet : 2 of 3 Hole Length : 30.00m Scale @ A4 : 1:50 Logged : MF Processed : HB Checked : JHS									
Easting: 396459.76 RL: 129.5					Northing: 787862.12 Datum: NZVD2016					System: TAIETM2000									
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect Spacing (mm)	Instrumentation Installation	Water level		
							Number / Type	Result											
119	11		Slightly weathered, grey, SANDSTONE; weak to moderately strong; wide spaced defects; fine sand to coarse gravel size grains (continued from layer starting at 9.1m ) 10.20 - 10.90 m: fine grained 10.25 m: 20 mm lignite interbed  10.90 - 11.40 m: fine to coarse sand, trace fine gravel of quartz schist  11.40 - 14.10 m: fine grained size, occasional organic laminates, widely spaced defects, black staining on faces  12.69 m: 50mm thick organic-rich layer, 2 mm lignite at each end  13.20 m: moderately strong to strong	HENLEY BRECCIA						PQTT				97 97 97					
118	12									PQTT				100 100 100					
117	13									PQTT				100 100 100					
116	14									PQTT				100 100 100					
115	15		Slightly weathered, massive, grey and white BRECCIA; moderately strong to strong; very widely spaced defects; matrix supported; clasts: fine to coarse gravel size, angular to sub-angular, quartz and schist, some clasts up to cobble size; matrix: coarse sand 14.60 - 15.60 m: weak to moderately strong							PQTT		SW		100 93 93					
114	16		15.60 - 16.20 m: moderately strong to strong							PQTT				100 93 93					
113	17		16.20 - 16.60 m: weak to moderately strong  16.60 - 17.10 m: moderately strong to strong							PQTT				87 80 67					
112	18		17.10 - 17.70 m: weak to moderately strong							PQTT				100 93 93					
111	19		17.70 - 18.10 m: CORELOSS							PQTT				100 93 93					
110			Slightly weathered, massive, grey and white BRECCIA; very weak to weak; very widely spaced defects; clast supported. Clasts: fine to coarse gravel size, angular to sub-angular, quartz and schist, some clasts up to cobble size 18.30 m: coarse gravel dominated 18.50 - 19.20 m: moderately strong to strong, harder matrix							PQTT				100 94 94					
			Slightly weathered, grey, fine SANDSTONE; very weak to weak; moderately widely spaced; orange iron stained layer. Breaks preferentially on iron stained layers						PQTT										
Notes and Comments: End of Hole @ 30.00m, Target Depth.  ~ 0.5 m topsoil stripped to make drill pad Groundwater SWL at 16.4 mbgl during piezo install.  Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical		Orientation:		Ground Water Level											
				Contractor: McNeills		Equipment: UDR600 (truck mounted)		Shear Vane Id: GEO2288		Date		Time		Reading (mbgl)		Hole depth (mbgl)			

<div></div> <div>Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Central Ridge Job Number: 12506381 Commenced: 29/05/2019 Completed: 30/05/2019</div>										Hole No. : BH05a Sheet : 3 of 3 Hole Length : 30.00m Scale @ A4 : 1:50 Logged : MF Processed : HB Checked : JHS									
Easting: 396459.76 RL: 129.5					Northing: 787862.12 Datum: NZVD2016					System: TAIETM2000									
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect Spacing (mm)	Instrumentation Installation	Water level		
							Number / Type	Result											
109	21		Slightly weathered, grey, fine SANDSTONE; very weak to weak; moderately widely spaced; orange iron stained layer. Breaks preferentially on iron stained layers ( <i>continued from layer starting at 19.2m</i> ) 20.00 m: spaced thin siltstone interbeds 20.55 m: moderately strong to strong 20.65 - 21.80 m: very weak to weak	HENLEY BRECCIA						PQTT				100 94 94					
108	22		21.80 - 22.00 m: weak to moderately strong 21.95 m: 20 mm SILTSTONE interbed 22.00 m: moderately strong to strong							PQTT				100 100 100					
107	23		Slightly weathered, grey, pink and white BRECCIA; moderately strong to strong; very widely spaced defects; clast supported; clasts: fine to coarse gravel size, angular to sub-angular, quartz and schist							PQTT				87 87 77					
106	24		23.80 - 24.20 m: clasts: fine gravel size with occasional coarse gravel 24.10 - 24.30 m: very weak to weak 24.30 - 24.65 m: moderately strong to strong							PQTT		SW		100 93 93					
105	25		24.65 - 24.75 m: very weak to weak 24.75 - 26.65 m: moderately strong to strong							PQTT									
104	26		25.30 - 26.65 m: clasts fine to coarse gravel sized							PQTT				87 87 73					
103	27		Reddish brown SILTSTONE; extremely weak to very weak 26.80 m: light grey, SILT/SAND mix; hard/very dense							PQTT				100 100 100					
102	28		Silty fine to medium silty SANDSTONE, very weak; sharp base contact with breccia, dips 20-30°							PQTT									
101	29		Slightly weathered, light brown, grey and white BRECCIA; weak to moderately strong; very widely spaced defects; clast supported; clasts: fine to coarse gravel size, quartz and schist, angular to sub-angular 28.40 - 30.00 0							PQTT		UW		94 91 91					
100									122mm										
Notes and Comments: End of Hole @ 30.00m, Target Depth. ~ 0.5 m topsoil stripped to make drill pad Groundwater SWL at 16.4 mbgl during piezo install. Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical		Orientation:		Ground Water Level											
				Contractor: McNeills		Equipment: UDR600 (truck mounted)		Shear Vane Id: GEO2288		Date: 30/05/19		Time: 00:00		Reading (mbgl)		Hole depth (mbgl): 30			



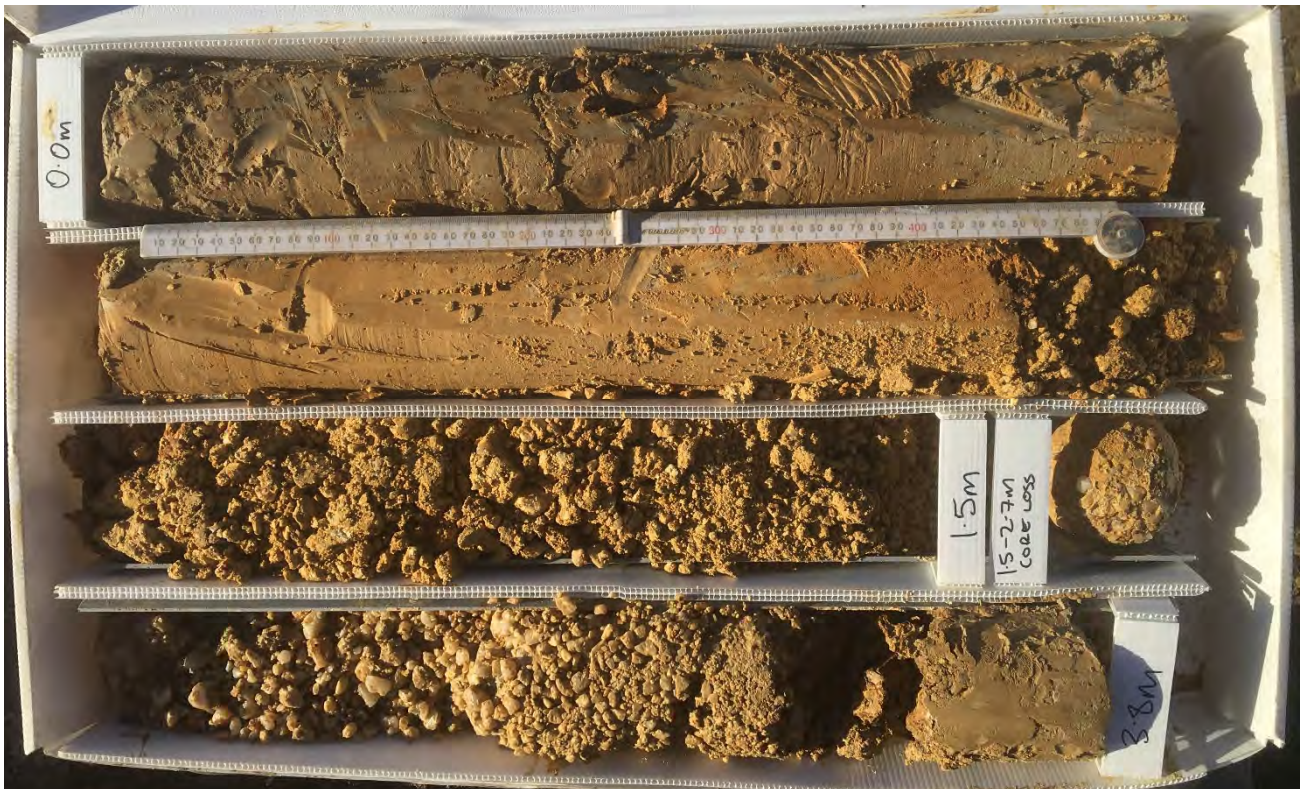
			Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Central Ridge Job Number: 12506381 Commenced: 29/05/2019 Completed: 30/05/2019						Hole No. : BH05b Sheet : 1 of 3 Hole Length : 30.00m Scale @ A4 : 1:50 Logged : MF Processed : HB Checked : JHS								
Easting: 396459.76 RL: 129.5			Northing: 787862.12 Datum: NZVD2016			System: TAIETM2000											
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect Spacing (mm)	Instrumentation Installation	Water level
			Clayey SILT; grey and dark orange-brown. Very stiff, moist, high plasticity (LOESS)	LOESS	M	VSt	SV@1.5m UTP			PQTT				100			
			SILT, minor clay, trace fine sand; orange-brown mottled grey. Very stiff, moist, low plasticity		M	VSt											
			Gravelly SILT, trace clay; orange-brown and white. Very stiff, moist, low plasticity; gravel is fine to medium, angular to sub-angular schist 1.50 - 2.70 m: CORELOSS		M	VSt											
			Gravelly SILT, trace clay; orange-brown and white. Very stiff, moist, low plasticity. Gravel is fine to medium, angular to sub-angular schist. Completely to highly weathered rock (HENLEY BRECCIA) 2.80 - 3.50 m: CORELOSS	HENLEY BRECCIA	M	VSt	SV@5.7m UTP			PQTT		HW-CW		53			
			Fine to medium GRAVEL; cream/white. Well graded, gravel: quartz and schist, angular to sub-angular		M	VSt											
			Gravelly SILT, trace clay; orange-brown and white. Very stiff, moist, low plasticity; iron staining on upper contact; gravel is fine to medium, angular to sub-angular, of quartz schist		M	VSt											
			SILT, trace organics; light grey. Hard, dry to moist, non-plastic. Highly weathered rock		D-M	H											
			Gravelly SILT, trace clay, trace organics; orange-brown and light grey. Very stiff, moist, non-plastic; gravel is fine to medium, angular to sub-angular, of quartz schist														
			SILT, trace clay, trace organics; light grey with orange-brown streaks. Hard, dry to moist, non-plastic														
			5.10 m: orange-brown mottled light grey 5.35 m: 2-3 mm iron "gravel" beds 5.60 m: thinly laminated		M	St-VSt											
			Silty CLAY, trace fine sand, trace organics; grey-brown. Stiff to very stiff, moist, high plasticity		M	H											
			SILT, trace clay, trace organics; light grey with black flecks. Hard, moist, non-plastic. Iron staining on lower contact		M	H											
			SILT, trace medium gravel; grey-brown. Hard, moist, non-plastic; dark orange-brown iron stained contact at 6.80 m		D	H											
			SILT, trace organics; light grey and orange-brown, with black flecks. Hard, dry, non-plastic, thin (<1mm) 'rusty' laminations Moderately weathered, thinly bedded, dark grey with black streaks SILTSTONE; very weak to weak; moderately wide to widely spaced defects; contains organic-rich layers 7.75 m: 50 mm fine grained sandstone interbed 7.90 - 8.50 m: with closely spaced lignite interlamination/ very thin interbeds 8.50 m: slightly weathered 8.70 m: weak to moderately strong														
			Slightly weathered, grey, SANDSTONE; weak to moderately strong; wide spaced defects; fine sand to coarse gravel size grains 9.60 m: 170 mm siltstone interbed						PQTT		SW		93 93 93				
Notes and Comments: End of Hole @ 30.00m, Target Depth. ~ 0.5 m topsoil stripped to make drill pad Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical		Orientation:		Ground Water Level									
				Contractor: McNeills		Equipment: UDR600 (truck mounted) Shear Vane Id: GEO2288		Date		Time		Reading (mbgl)		Hole depth (mbgl)			

		Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Central Ridge Job Number: 12506381 Commenced: 29/05/2019 Completed: 30/05/2019						Hole No. : BH05b Sheet : 2 of 3 Hole Length : 30.00m Scale @ A4 : 1:50									
Easting: 396459.76 RL: 129.5		Northing: 787862.12 Datum: NZVD2016		System: TAIETM2000		Logged : MF Processed : HB Checked : JHS											
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect Spacing (mm)	Instrumentation Installation	Water level
			Slightly weathered, grey, SANDSTONE; weak to moderately strong; wide spaced defects; fine sand to coarse gravel size grains (continued from layer starting at 9.1m ) 10.20 - 10.90 m: fine grained 10.25 m: 20 mm lignite interbed  10.90 - 11.40 m: fine to coarse sand, trace fine gravel of quartz schist  11.40 - 14.10 m: fine grained size, occasional organic laminates, widely spaced defects, black staining on faces  12.69 m: 50mm thick organic-rich layer, 2 mm lignite at each end  13.20 m: moderately strong to strong	HENLEY BRECCIA						PQTT				97 97 97			
										PQTT				100 100 100			
										PQTT				100 100 100			
										PQTT				100 93 93			
										PQTT				87 80 67			
										PQTT				100 93 93			
			Slightly weathered, massive, grey and white BRECCIA; moderately strong to strong; very widely spaced defects; matrix supported; clasts: fine to coarse gravel size, angular to sub-angular, quartz and schist, some clasts up to cobble size; matrix: coarse sa 14.60 - 15.60 m: weak to moderately strong  15.60 - 16.20 m: moderately strong to strong  16.20 - 16.60 m: weak to moderately strong  16.60 - 17.10 m: moderately strong to strong  17.10 - 17.70 m: weak to moderately strong														
			17.70 - 18.10 m: CORELOSS														
			Slightly weathered, massive, grey and white BRECCIA; very weak to weak; very widely spaced defects; clast supported. Clasts: fine to coarse gravel size, angular to sub-angular, quartz and schist, some clasts up to cobble size 18.30 m: coarse gravel dominated 18.50 - 19.20 m: moderately strong to strong, harder matrix							PQTT				100 93 93			
			Slightly weathered, grey, fine SANDSTONE; very weak to weak; moderately widely spaced; orange iron stained layer. Breaks preferentially on iron stained layers							PQTT				100 94 94			
Notes and Comments: End of Hole @ 30.00m, Target Depth. ~ 0.5 m topsoil stripped to make drill pad				Inclination: Vertical		Orientation:		Ground Water Level									
Refer to explanation sheets for abbreviation and symbols				Contractor: McNeills		Equipment: UDR600 (truck mounted)		Shear Vane Id: GEO2288		Date	Time	Reading (mbgl)	Hole depth (mbgl)				

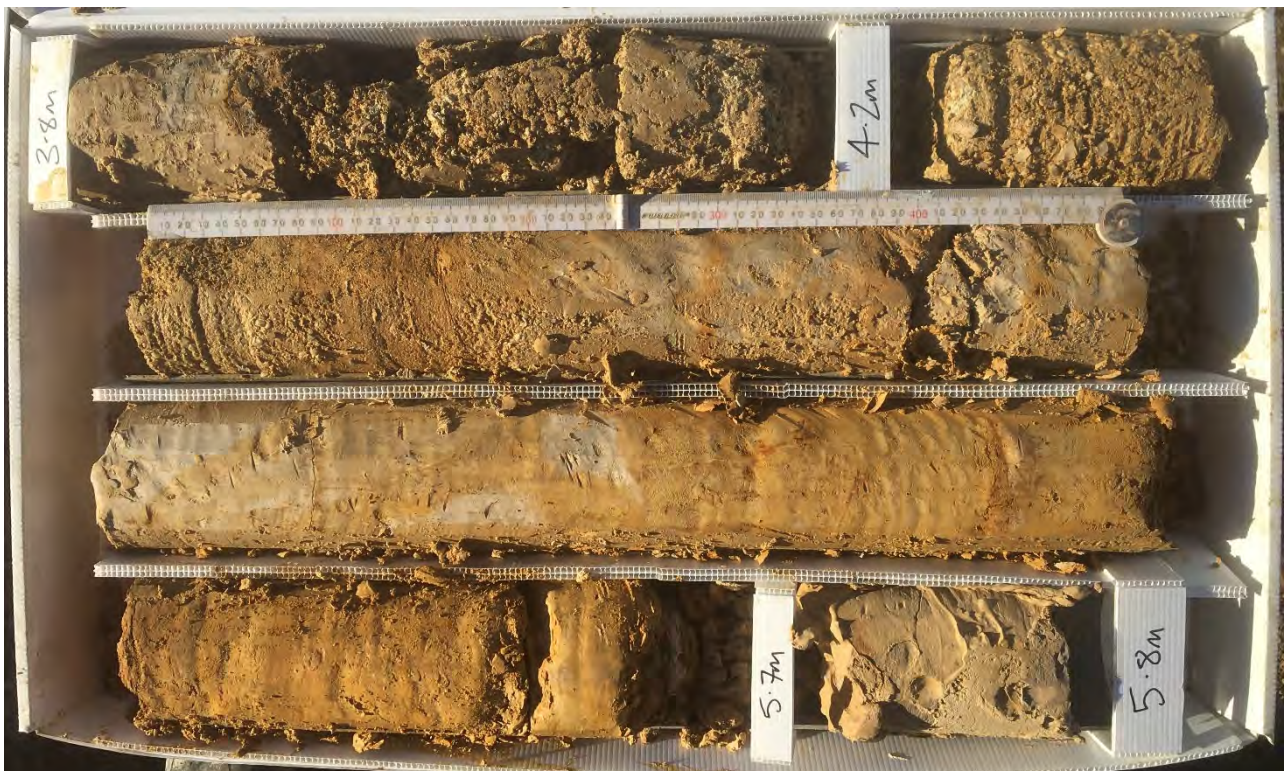




Project	Smooth Hill Landfill Consenting	
Client	Dunedin City Council	
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Box 1 of 14: 0.0 m to 3.8 m



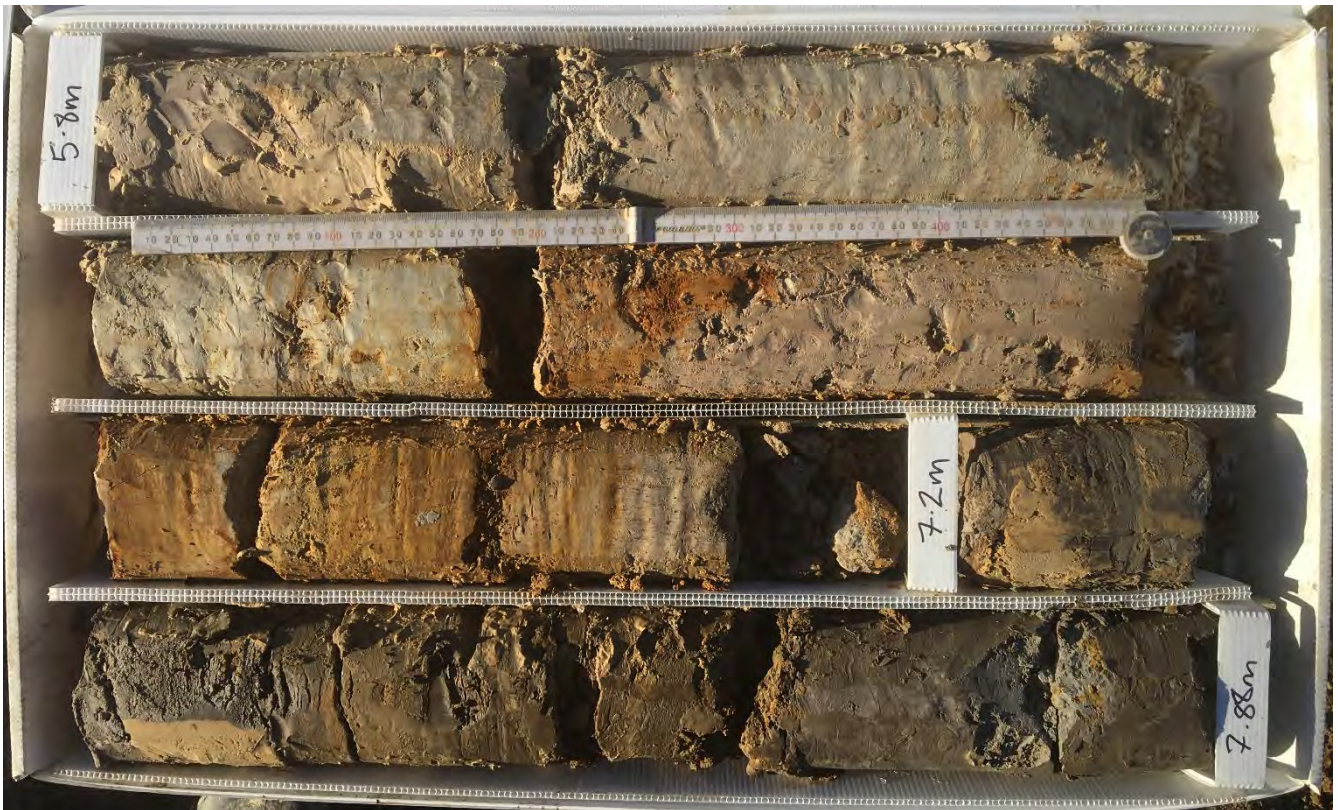
Box 2 of 14: 3.8 m to 5.8 m





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Box 3 of 14: 5.8 m to 7.88 m



Box 4 of 14: 7.88 m to 9.93 m





Project	Smooth Hill Landfill Consenting	
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Box 5 of 14: 9.93 m to 11.87 m



Box 6 of 14: 11.87 m to 13.9 m

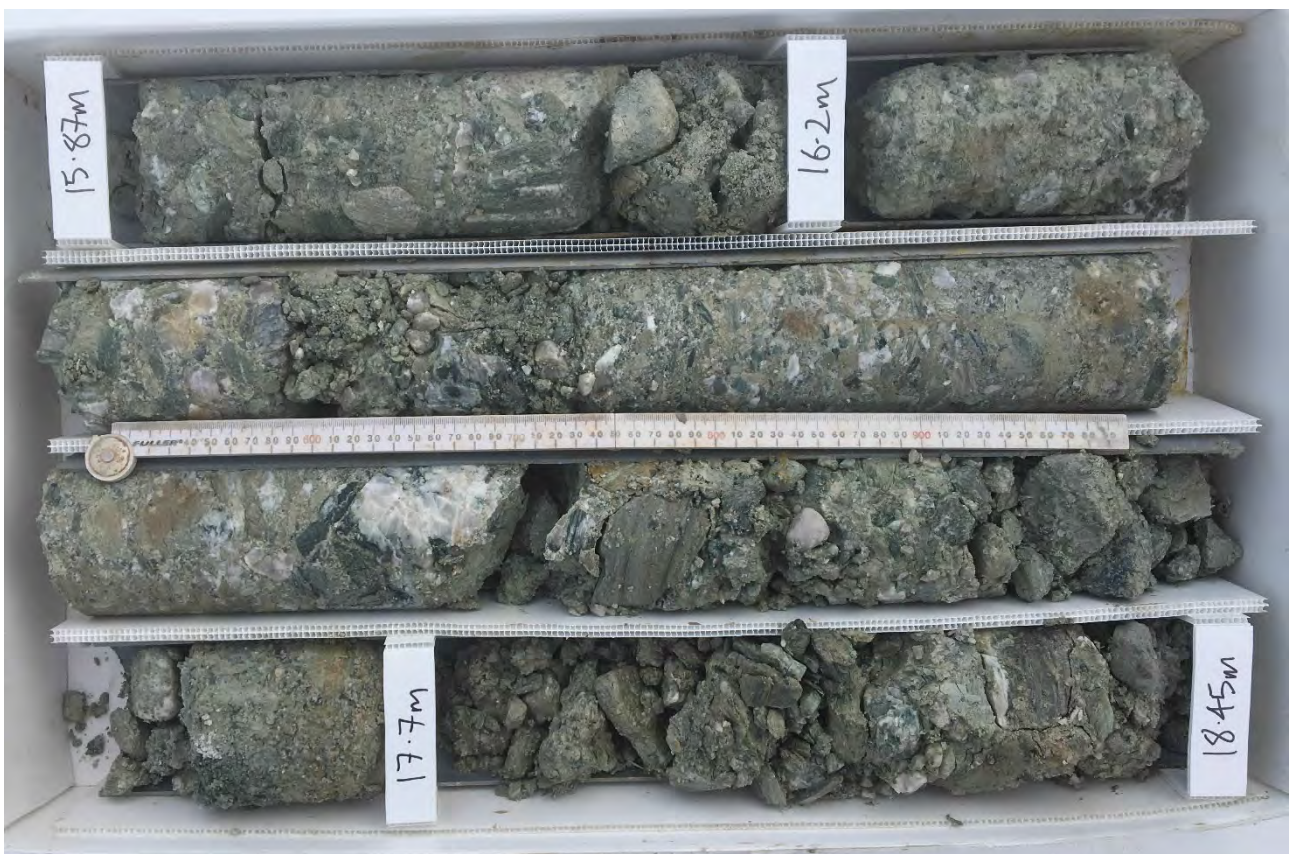




Project	Smooth Hill Landfill Consenting	
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Box 7 of 14: 13.9 m to 15.87 m



Box 8 of 14: 15.87 m to 18.45 m





Project	Smooth Hill Landfill Consenting	
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Box 9 of 14: 18.45 m to 20.1 m

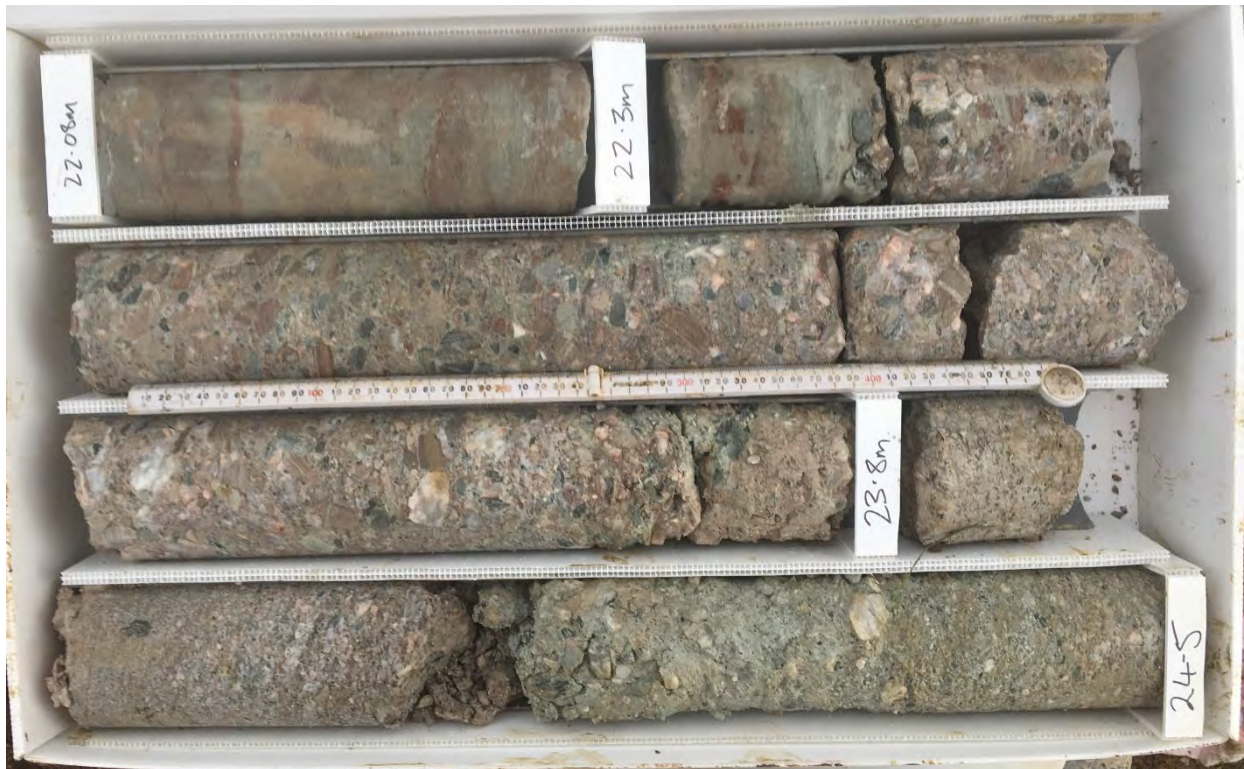


Box 10 of 14: 20.1 m to 22.08 m

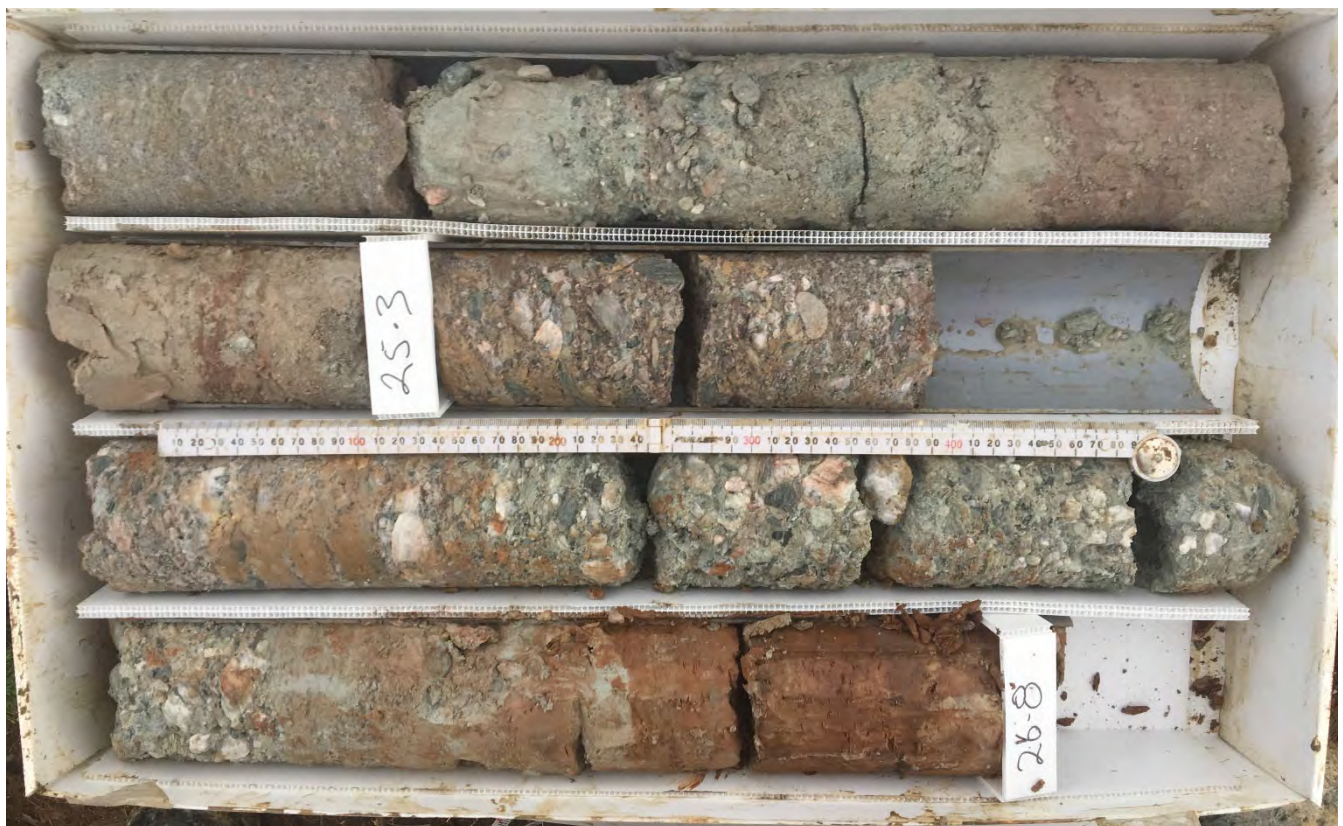




Project	Smooth Hill Landfill Consenting	
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Box 11 of 14: 22.08 m to 24.5 m



Box 12 of 14: 24.5 m to 26.8 m





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Project	Smooth Hill Landfill Consenting	
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Box 13 of 14: 26.8 m to 29.0 m


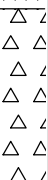


Box 14 of 14: 29.0 m to 30.0 m (EOH)

		Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Southwest Ridge Job Number: 12506381 Commenced: 13/06/2019 Completed: 14/06/2019						Hole No. : BH06 Sheet : 1 of 3 Hole Length : 30.00m Scale @ A4 : 1:50 Logged : MF Processed : HB Checked : JHS										
Easting: 396168.25 RL: 149.75		Northing: 787593.98 Datum: NZVD2016		System: TAIETM2000														
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect Spacing (mm)	Instrumentation Installation	Water level	
							Number / Type	Result										
	0		Top 250 mm dug out for drill pad (TOPSOIL)	TS														
149	0.25		SILT, trace to minor clay, trace fine to medium sand, trace fine gravel; grey and orange-brown. Very stiff, moist, low plasticity (LOESS)	LOESS	M	VSt				PQTT				83				
	1													17				
148	1.25		Highly weathered, yellow-brown SILTSTONE; extremely weak; no defects (HENLEY BRECCIA)	HENLEY BRECCIA						PQTT								
	2		Highly weathered, thinly bedded, yellow-brown silty fine SANDSTONE; extremely weak; no defects; iron-staining in layers and spots; trace organics throughout;							PQTT					100			
															89			
147	2.70		2.70 m: 170 mm layer gravelly SANDSTONE							PQTT								
	3																	
146	4.90		4.90 m: 200mm loose sand/pebbly layer; likely coreloss depth							PQTT					93			
															75			
															75			
145	5.50		5.50 - 5.70 m: Fine gravel (quartz and schist, angular to sub-angular) layer							PQTT					78			
															78			
															78			
144	6.70		6.70 - 7.20 m: moderately weathered, very weak to weak							PQTT					100			
															100			
															100			
143	7.20		Moderately weathered, yellow-brown SILTSTONE; very weak to weak; very widely spaced defects							PQTT								
			7.30 - 7.75 m: light grey with black flecks															
142	7.75		7.75 m: orange-brown with black streaks; break on bedding plane, dark iron-staining on face							PQTT					60			
			Moderately weathered, grey, fine to coarse SANDSTONE; extremely weak to very weak												60			
			8.10 - 8.70 m: CORELOSS												60			
141	8.70		Moderately weathered, fine to medium SANDSTONE; very weak to weak; widely spaced defects							PQTT					100			
															87			
															87			
140	9.25		9.25 m: tight break, iron-stained face, staining decreases for 50 mm above and below break							PQTT								
			9.72 m: 15-20 mm dark brown layer															
Notes and Comments: End of Hole @ 30.00m, Target Depth. Groundwater not encountered.  Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical		Orientation:		Ground Water Level										
				Contractor: McNeills		Equipment: UDR600 (truck mounted)		Shear Vane Id:		Date	Time	Reading (mbgl)	Hole depth (mbgl)					



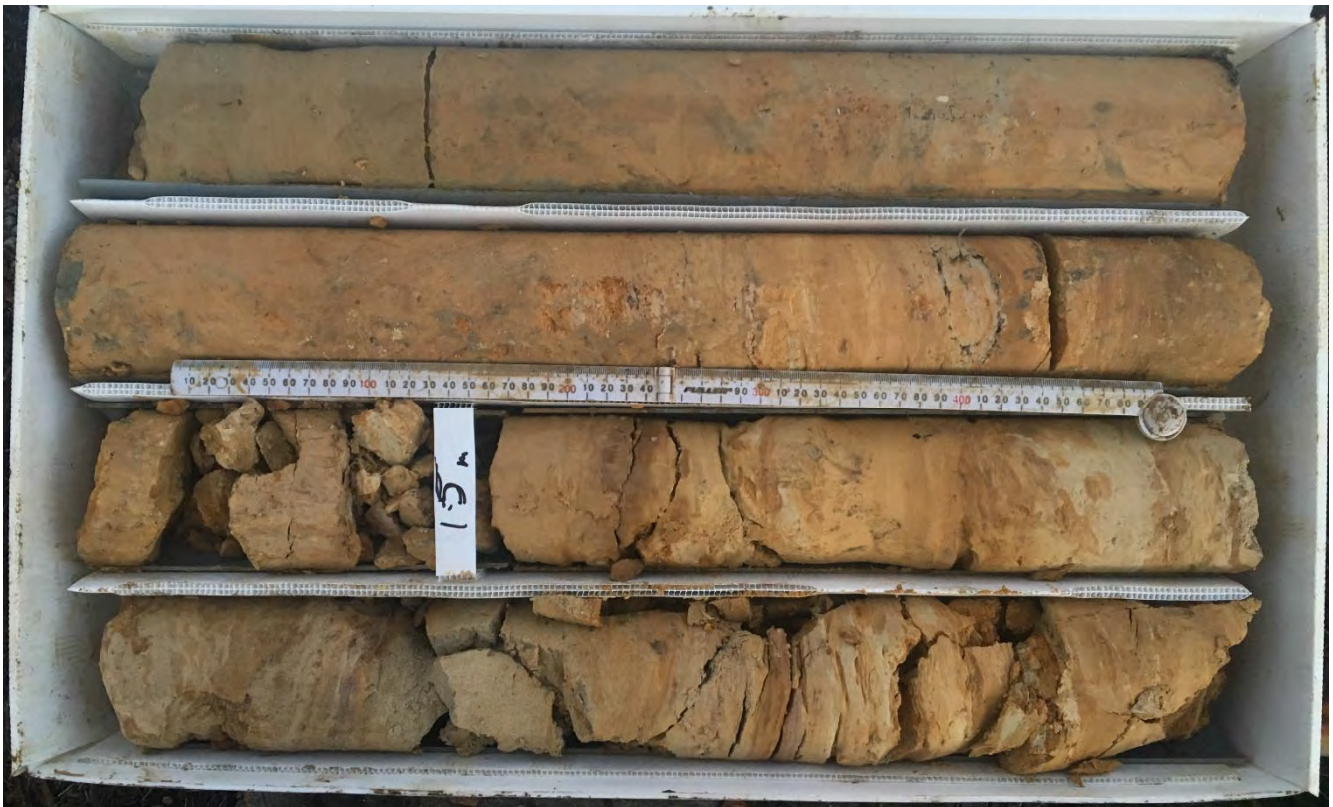
		Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Southwest Ridge Job Number: 12506381 Commenced: 13/06/2019 Completed: 14/06/2019						Hole No. : BH06 Sheet : 2 of 3 Hole Length : 30.00m Scale @ A4 : 1:50 Logged : MF Processed : HB Checked : JHS									
Easting: 396168.25 RL: 149.75		Northing: 787593.98 Datum: NZVD2016		System: TAIETM2000													
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Number / Type	Result	Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR RQD (%)	Defect Spacing (mm)	Instrumentation Installation	Water level
139	10.2		9.75 m: tight break, iron stained face; core stained below break, but not above 10.00 - 10.20 m: with some fine rounded gravel 10.20 - 10.45 m: CORELOSS Moderately weathered, laminated to moderately thickly bedded, light grey and orange-brown SANDSTONE; extremely to very weak; poorly indurated; very widely spaced defects 10.75 m: 100 mm SILTSTONE, black basal contact 10.85 m: 60 mm light grey with black flecks, minor organic inclusions 11.00 m: yellow-brown and orange-brown 11.10 m: 2-3 mm iron stained 'rusty' layer 11.55 m: 70 mm light grey interbed  12.00 JT, 45°, pl, r, CLAY, Iron stained clay infill. 12.25 m: 100 mm minor organics, orange-brown and black layer Moderately weathered grey and white CONGLOMERATE; very weak to weak; clasts, fine to medium, rounded to sub-rounded, quartz and schist 12.54 - 13.40 m: CORELOSS Moderately weathered grey and white CONGLOMERATE; very weak to weak; clasts, fine to medium, rounded to sub-rounded, quartz and schist Slightly weathered, grey with occasional black, fine to medium SANDSTONE; very weak to weak; poorly indurated, no defects. Closely to very closely spaced laminations of lignite	HENLEY BRECCIA						PQTT		MW		83 83 83			
138	12									PQTT				56 56 56			
137	13									PQTT				87 87 87			
136	14									PQTT				100 100 100			
135	15									PQTT				100 100 87			
134	16									PQTT				87 63 63			
133	17									PQTT				93 65 47			
132	18																
131	19																
130	20																
Notes and Comments: End of Hole @ 30.00m, Target Depth. Groundwater not encountered.  Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical		Orientation:		Ground Water Level									
				Contractor: McNeills		Equipment: UDR600 (truck mounted)				Date		Time		Reading (mbgl)		Hole depth (mbgl)	
				Shear Vane Id:													

			<b>Project : Smooth Hill Landfill Consenting</b> <b>Client : Dunedin City Council</b> <b>Site : Southwest Ridge</b> <b>Job Number: 12506381</b> Commenced: 13/06/2019      Completed: 14/06/2019						<b>Hole No. : BH06</b> Sheet : 3 of 3 Hole Length : 30.00m Scale @ A4 : 1:50 Logged : MF Processed : HB Checked : JHS														
Easting: 396168.25      Northing: 787593.98      System: TAIETM2000 RL: 149.75      Datum: NZVD2016																							
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Number / Type	Result	Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect	Spacing (mm)	Instrumentation Installation	Water level					
129	21		of lignite and widely spaced moderately thin siltstone beds Unweathered, light grey and black fine to medium SANDSTONE; very weak to weak; very widely spaced defects; with moderately widely spaced laminated very thin to thin beds of lignite and widely spaced moderately thin siltstone beds <i>(continued from layer starting at 19.4m)</i> 20.20 m: fine to coarse sand 20.70 m: fine to medium sand 21.06 m: 230 mm siltstone interbed	HENLEY BRECCIA						PQTT				93 65 47									
128	22		21.70 m: very thinly bedded (2-10 mm)							PQTT				100 100 100									
127	23		22.20 m: moderately thickly bedded (~ 300 mm) 22.40 m: 150 mm siltstone interbed							PQTT				87 87 87									
126	24		22.75 m: laminated (2-10 mm)							PQTT				100 100 100									
125	25									PQTT				88 88 88									
124	26									PQTT				100 32 32									
123	27		27.50 - 28.50 m: very closey spaced fractures, possibly drilling induced						PQTT														
122	28		28.40 - 28.50 m: dark grey-brown for 100 mm						PQTT														
121	29		28.70 - 28.80 m: dark brown layer - looks like lithified topsoil Unweathered, light grey BRECCIA; weak to moderately strong; no defects; moderately well indurated; clasts: quartz and schist, fine gravel size, sub-angular to sub-rounded; matrix supported; matrix: fine to coarse sand						PQTT				76 76 76										
120																							
<b>End of Hole @ 30.00m, Target Depth.</b> End of Hole @ 30.00m, Target Depth. Groundwater not encountered. Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical      Orientation: Contractor: McNeills Equipment: UDR600 (truck mounted) Shear Vane Id:						<b>Ground Water Level</b> <table border="1"> <tr> <th>Date</th> <th>Time</th> <th>Reading (mbgl)</th> <th>Hole depth (mbgl)</th> </tr> <tr> <td>14/06/19</td> <td>00:00</td> <td></td> <td>30</td> </tr> </table>						Date	Time	Reading (mbgl)	Hole depth (mbgl)	14/06/19	00:00		30
Date	Time	Reading (mbgl)	Hole depth (mbgl)																				
14/06/19	00:00		30																				





Project	Smooth Hill Landfill Consenting	
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Box 1 of 13: 0.0 m to 2.4 m



Box 2 of 13: 2.4 m to 4.6 m





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Box 3 of 13: 4.6 m to 7.2 m



Box 4 of 13: 7.2 m to 10.0 m





CLIENTS | PEOPLE | PERFORMANCE

Project	Smooth Hill Landfill Consenting	
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Box 5 of 13: 10.0 m to 13.2 m



Box 6 of 13: 13.2 m to 15.6 m





Project	Smooth Hill Landfill Consenting	
Client	Dunedin City Council	
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Box 7 of 13: 15.6 m to 17.7 m

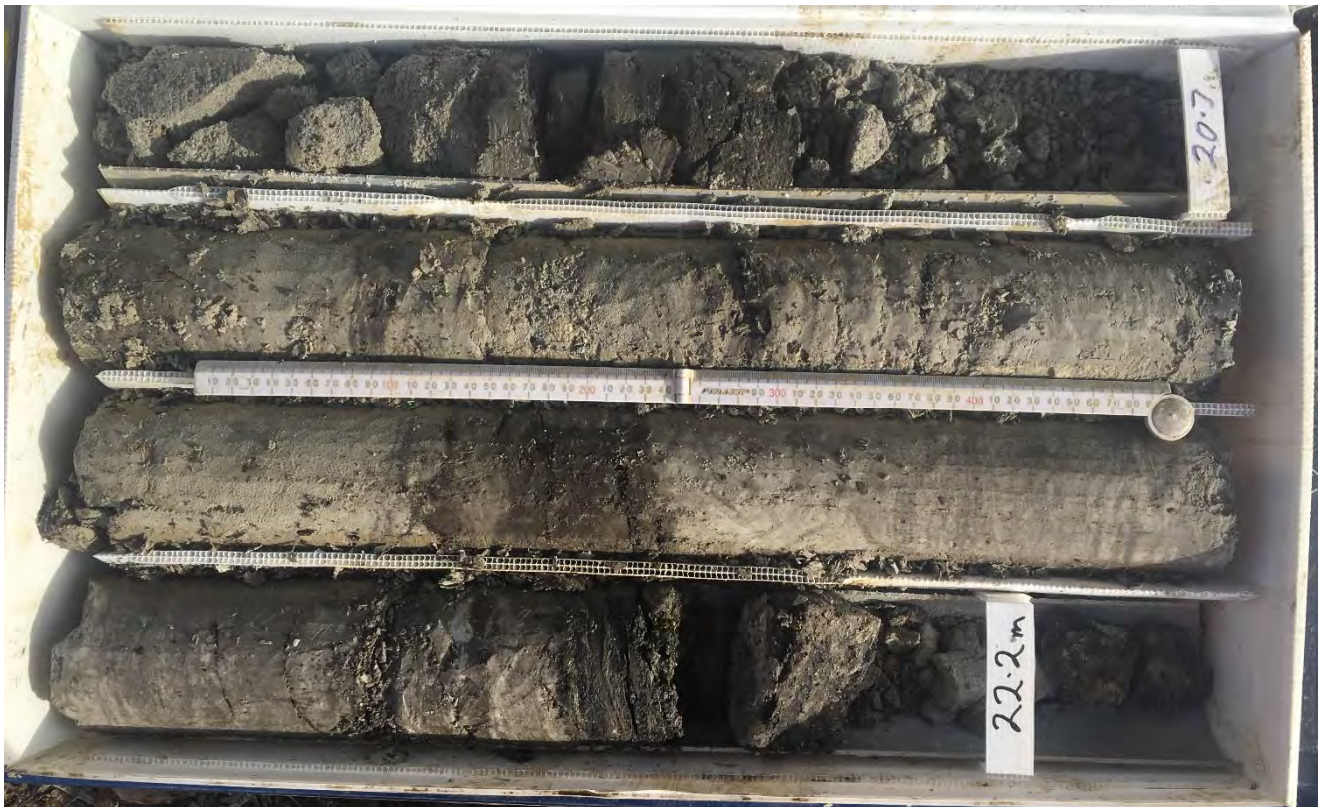


Box 8 of 13: 17.7 m to 20.2 m





Project	Smooth Hill Landfill Consenting	
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Box 9 of 13: 20.2 m to 22.2 m



Box 10 of 13: 22.2 m to 24.7 m

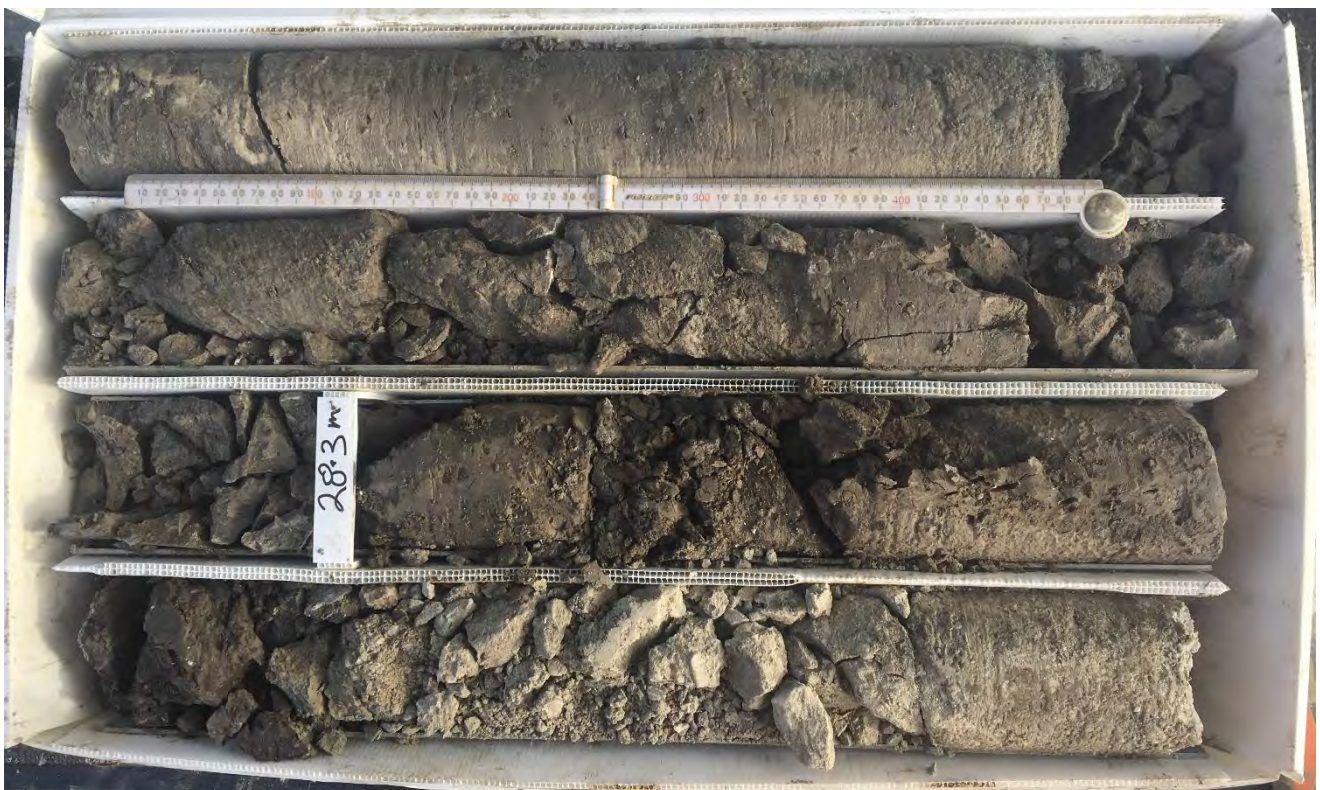




Project	Smooth Hill Landfill Consenting	
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Box 11 of 13: 24.7 m to 27.0 m

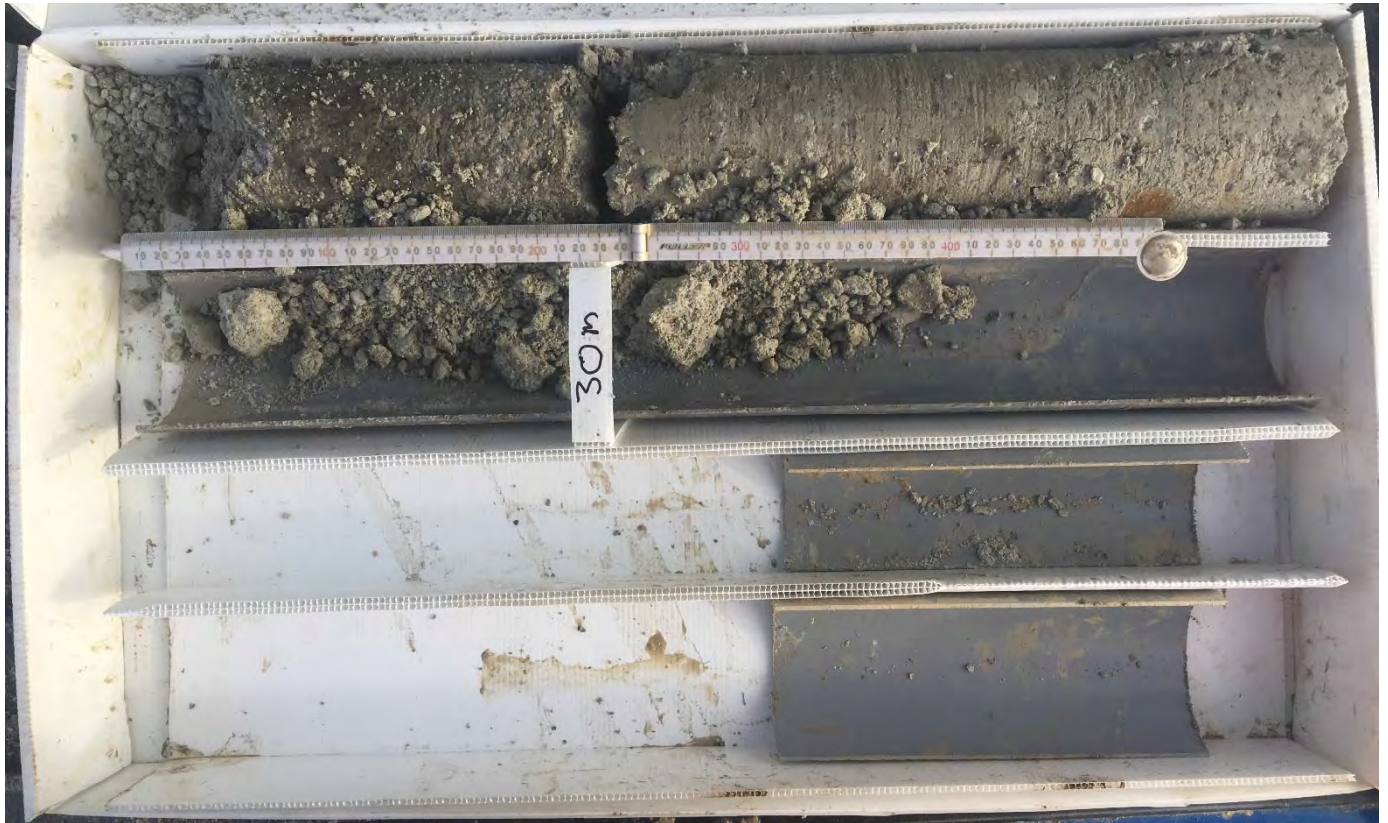


Box 12 of 13: 27.0 m to 29.3 m





Project	Smooth Hill Landfill Consenting	
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Box 13 of 13: 29.3 m to 30.0 m (EOH)















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Project	Smooth Hill Landfill Consenting	
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Box 1 of 8: 0.0 m to 2.7 m



Box 2 of 8: 2.7 m to 4.8 m





Project	Smooth Hill Landfill Consenting	
Client	Dunedin City Council	
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Box 3 of 8: 4.8 m to 7.0 m



Box 4 of 8: 7.0 m to 9.1 m



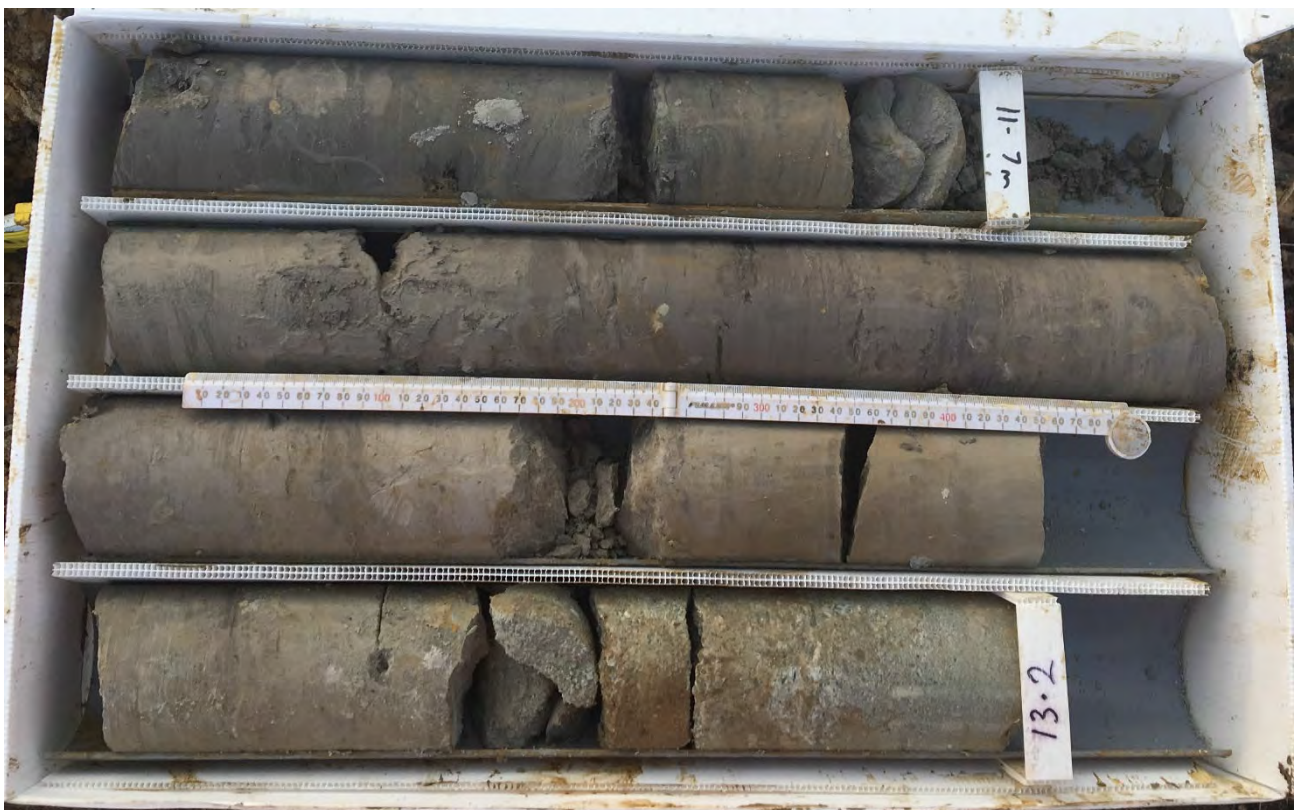


CLIENTS | PEOPLE | PERFORMANCE

Project	Smooth Hill Landfill Consenting	
Client	Dunedin City Council	
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Box 5 of 8: 9.1 m to 11.2 m

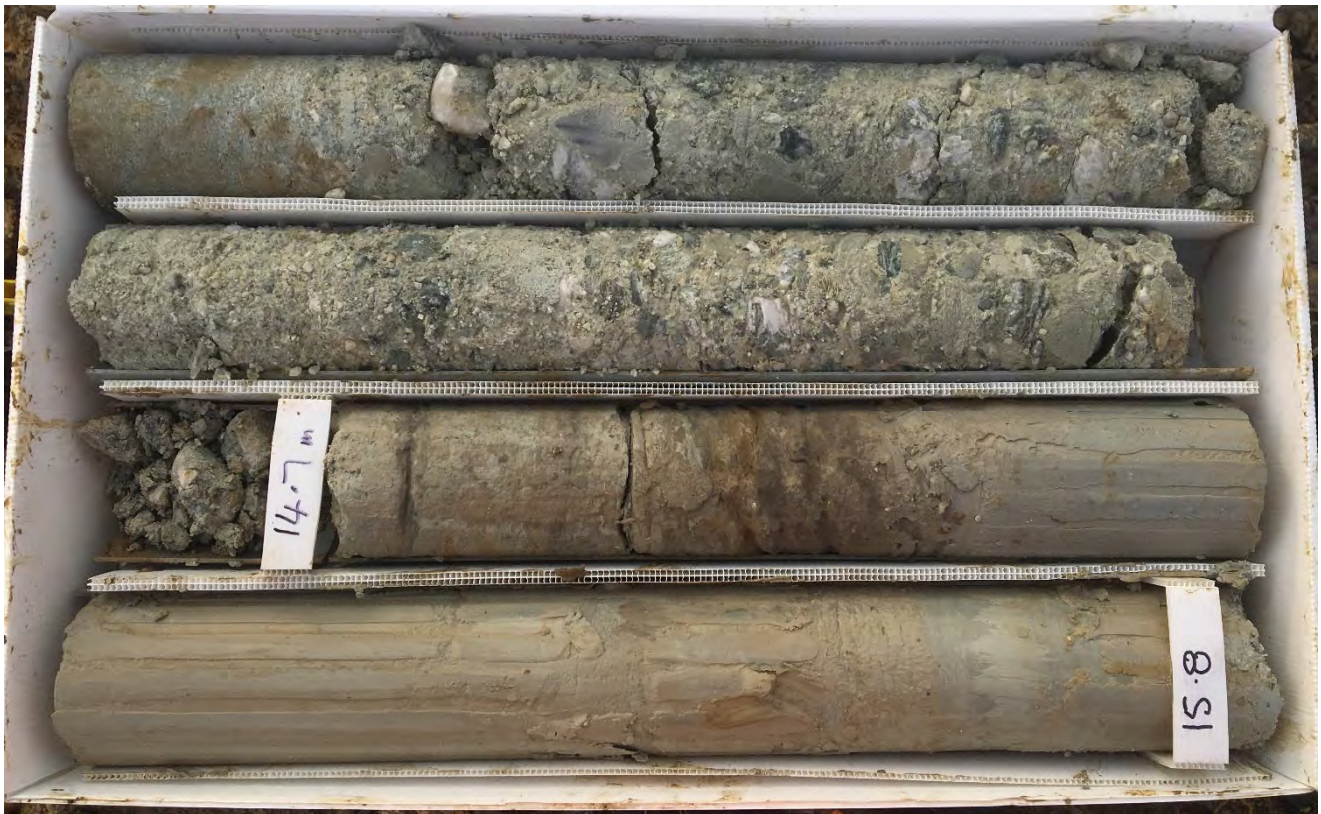


Box 6 of 8: 11.2 m to 13.2 m






Project	Smooth Hill Landfill Consenting	
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Box 7 of 8: 13.2 m to 15.8 m



Box 8 of 8: 15.8 m to 20.0 m (EOH)

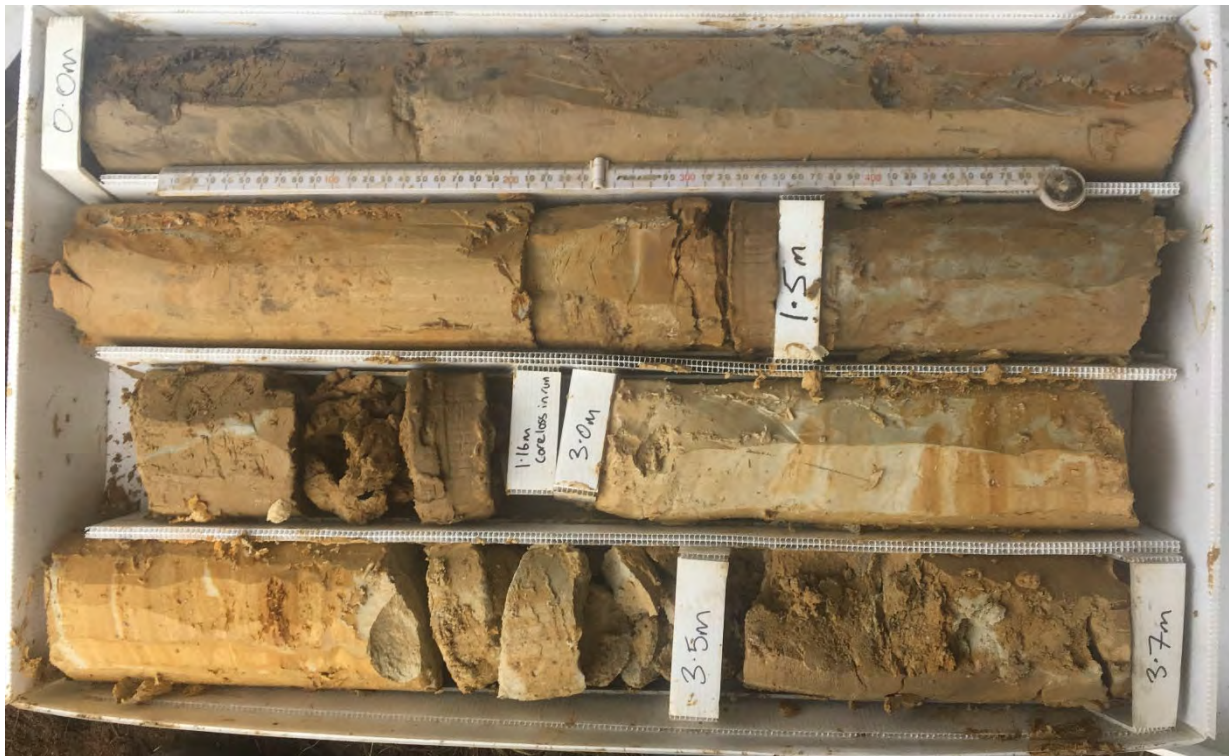
		Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Big Stone Road Job Number: 12506381 Commenced: 11/06/2019						Hole No. : BH08 Sheet : 1 of 2 Hole Length : 20.00m Scale @ A4 : 1:50 Logged : MF Processed : HB Checked : JHS									
Easting: 396809.71		Northing: 787700.67		System: TAIETM2000													
RL: 143.89		Datum: NZVD2016															
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect Spacing (mm)	Instrumentation Installation	Water level
							Number / Type	Result									
143.89	0.25		TOPSOIL; silt, trace to minor clay, trace fine sand; dark grey and yellow-brown. Very stiff, moist, low plasticity; trace roots	TS	M	VSt											
143.89	0.60		SILT, trace clay, trace fine sand; grey mottled orange-brown. Very stiff, moist, low plasticity (LOESS) 0.60 m: more orange-brown mottled grey		M	VSt				PQTT				68			
142.84	1.50		1.50 m: grey and brown mottled orange iron stained inclusions														
142.84	1.84		1.84 - 2.86 m: CORE LOSS	LOESS						PQTT				23			
141.286	2.86		Fine sandy SILT, trace clay; light grey-brown. Very stiff, dry, low plasticity 3.00 m: light grey and orange; iron stained laminations		D	VSt		SV@3m 194 kPa		PQTT				100			
140.375	3.75		SILT, trace to minor clay, trace coarse sand (rusty); light grey and orange-brown. Very stiff to hard, moist, low plasticity		M	VSt-H		SV@3.5m UTP		PQTT				90			
140.41	4.1		SILT, minor to some clay; brown with black flecks and streaks. Very stiff to hard, dry to moist, high plasticity; trace to minor organics (BURIED TOPSOIL)	BTS	D-M	VSt-H				PQTT							
139.52	5.2		Highly weathered, grey, orange-brown and yellow-brown BRECCIA; very weak to weak; no defects; clasts: quartz and schist, sub-angular to sub-rounded, fine gravel size; matrix: fine to coarse sand; matrix supported							PQTT				100			
139.52	5.2		Highly weathered, grey and orange-brown SILTSTONE; extremely weak to very weak; no defects											100			
139.52	5.2		Highly weathered, grey, orange-brown and yellow-brown BRECCIA; very weak to weak; no defects; clasts: quartz and schist, sub-angular to sub-rounded, fine to medium gravel, matrix: fine to coarse sand; matrix supported											100			
138.62	6.2		6.20 - 6.90 m: CORELOSS (inferred silty GRAVEL)							PQTT				53			
137.69	7.1		Fine to medium GRAVEL; orange-brown, white, yellow-brown and grey. Poorly graded; inferred silt matrix from minimal matrix recovery; gravel, quartz and schist, angular to sub-rounded.											40			
137.69	7.1													33			
136.765	7.4		Moderately weathered, grey, orange-brown and white BRECCIA; weak; gravel quartz and schist, angular to sub-rounded, fine to medium gravel; matrix: fine to coarse sand; matrix supported	HENLEY BRECCIA						PQTT							
136.81	8.1		Slightly weathered, light grey SILTSTONE; very weak to weak; no defects (grades into next unit)							PQTT				100			
136.81	8.1		Slightly weathered, light grey fine to coarse SANDSTONE; very weak to weak; no defects											100			
136.81	8.1													100			
135.93	9.0		Slightly weathered, light grey and grey BRECCIA; weak to moderately strong; no defects; no visible bedding; matrix: fine to coarse sand, matrix supported; clasts: quartz and schist, sub-rounded to angular, fine to medium gravel size							PQTT				93			
134.93	14.10		9.00 - 14.10 m: unweathered, fine to coarse gravel size clasts, clast supported											93			
Notes and Comments: End of Hole @ 20.00m, Target Depth. Groundwater not encountered. No piezos were installed. Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical		Orientation:		Ground Water Level		Date	Time	Reading (mbgl)	Hole depth (mbgl)				
				Contractor: McNeills		Equipment: UDR600 (truck mounted)											
				Shear Vane Id: GEO2288													







Project	Smooth Hill Landfill Consenting	
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Box 1 of 8: 0.0 m to 3.7 m

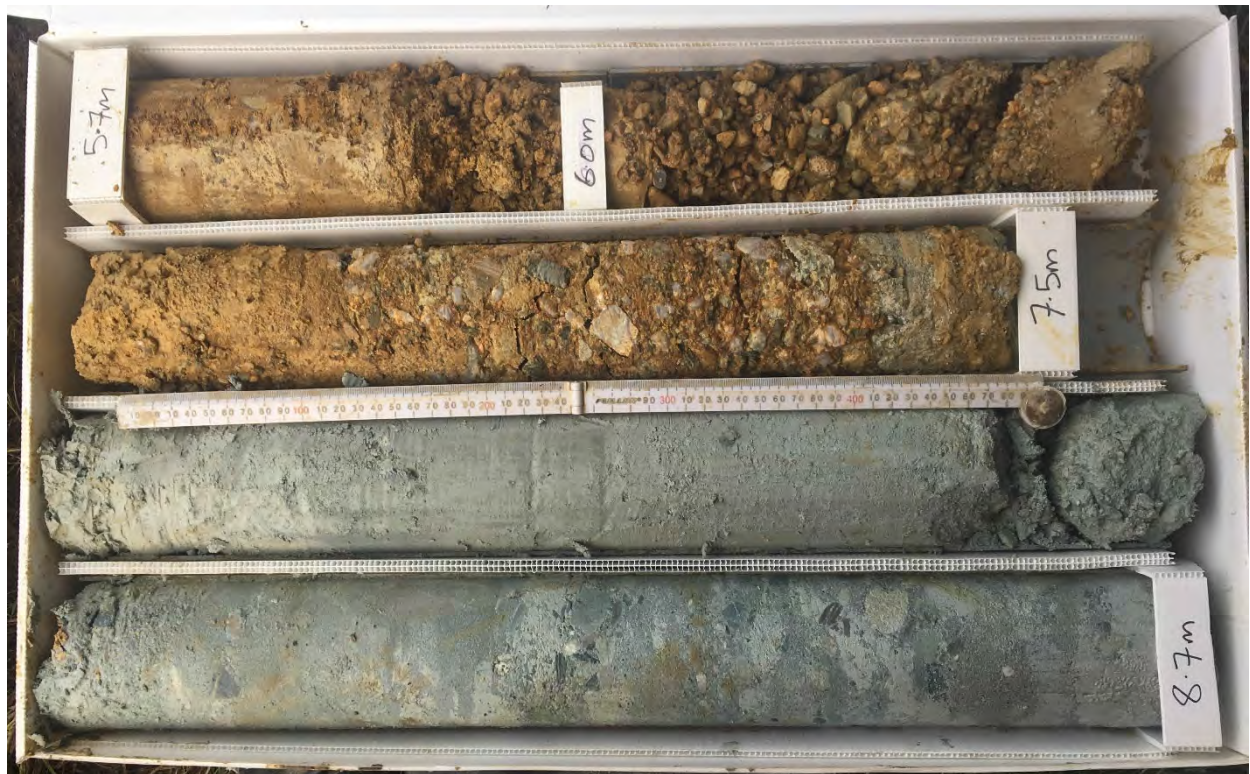


Box 2 of 8: 3.7 m to 5.7 m

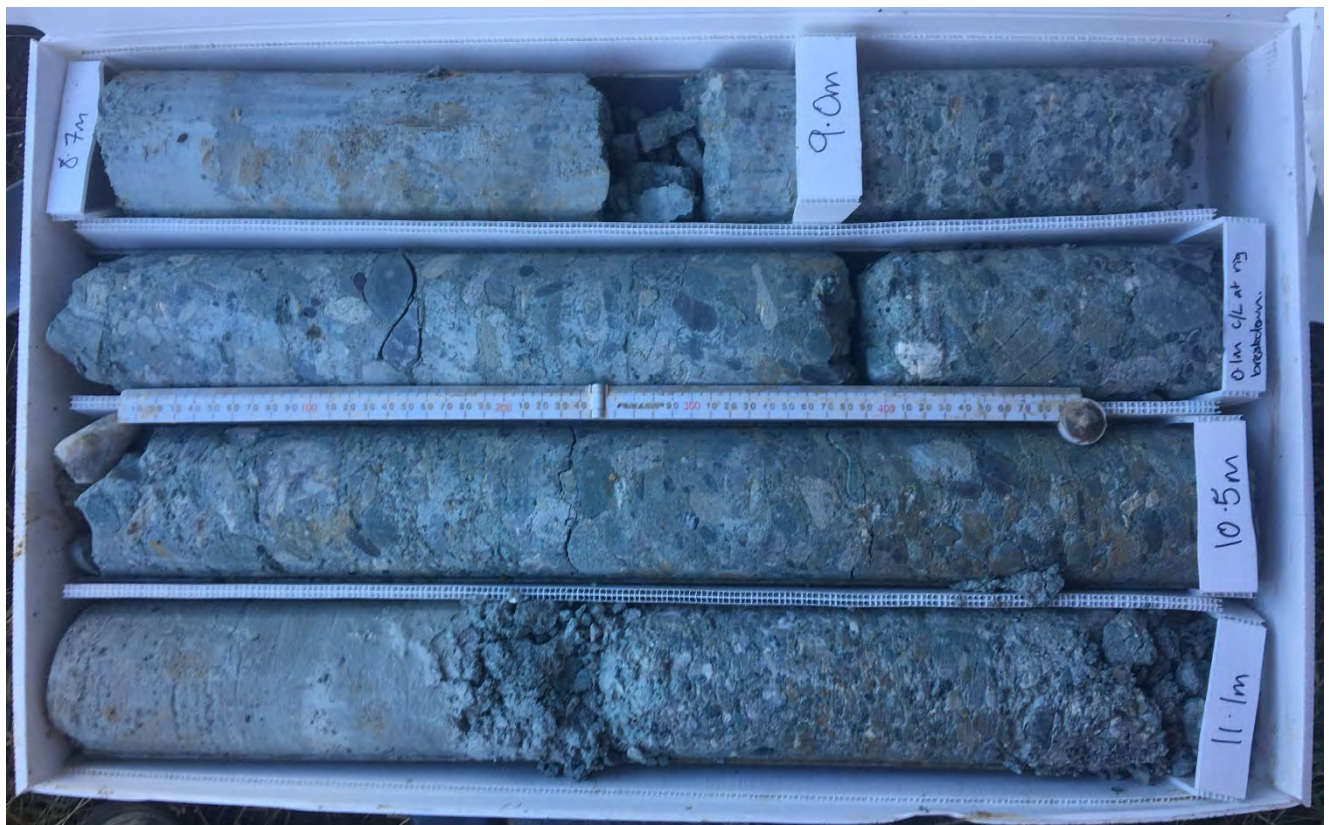




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Box 3 of 8: 5.7 m to 8.7 m



Box 4 of 8: 8.7 m to 11.1 m

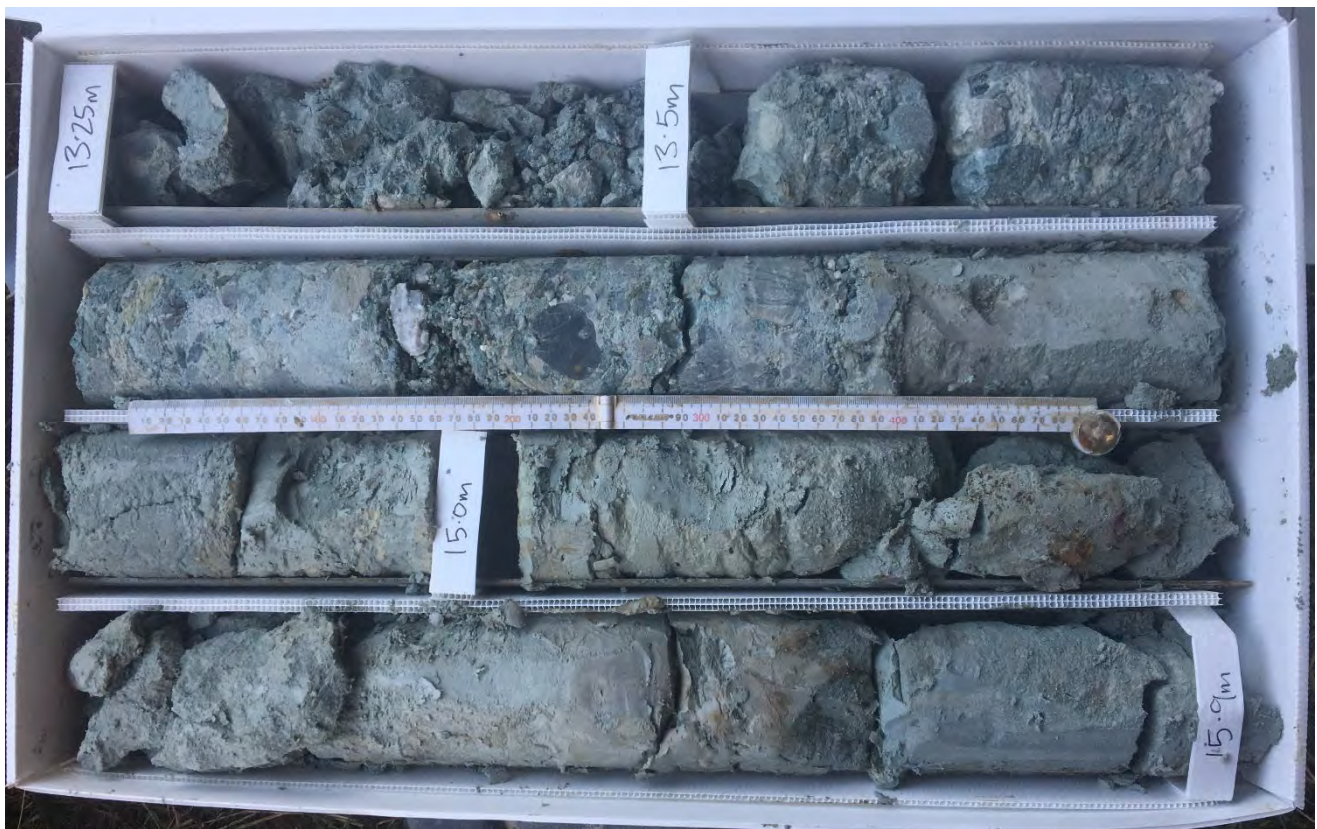




Project	Smooth Hill Landfill Consenting	
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Box 5 of 8: 11.1 m to 13.25 m



Box 6 of 8: 13.25 m to 15.9 m






Project	Smooth Hill Landfill Consenting	
Client	Dunedin City Council	
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Borehole ID	BH08	



Box 7 of 8: 15.9 m to 18.0 m



Box 8 of 8: 18.0 m to 20.0 m (EOH)

<div></div> <div>Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Western Boundary Job Number: 12506381 Commenced: 12/06/2019 Completed: 12/06/2019</div>										<div>Hole No. : BH09a</div> <div>Sheet : 1 of 2 Hole Length : 16.50m Scale @ A4 : 1:50</div> <div>Logged : MF Processed : HB Checked : JHS</div>																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																			
Easting: 395951.84 Northing: 788050.36 System: TAIETM2000 RL: 132.8 Datum: NZVD2016																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																													
<div>RL (m)</div> <div>Depth (m)</div> <div>Graphic</div>										<div>Material Description</div>										<div>Geological Unit</div>		<div>Moisture condition</div>		<div>Consistency / Relative density</div>		<div>Sample</div>		<div>Casing</div>		<div>Method</div>		<div>Flush Return (%)</div>		<div>Weathering</div>		<div>Estimated Strength (MPa)</div>		<div>TCR SCR RQR (%)</div>		<div>Defect Spacing (mm)</div>		<div>Instrumentation Installation</div>		<div>Water level</div>																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																	
0.00 - 0.50 m: CORELOSS (FILL: reworked Loess)										FILL																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																			



<div></div> <div>Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Western Boundary Job Number: 12506381 Commenced: 12/06/2019 Completed: 12/06/2019</div>										Hole No. : BH09a Sheet : 2 of 2 Hole Length : 16.50m Scale @ A4 : 1:50 Logged : MF Processed : HB Checked : JHS									
Easting: 395951.84 RL: 132.8					Northing: 788050.36 Datum: NZVD2016					System: TAIETM2000									
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect Spacing (mm)	Instrumentation Installation	Water level		
			Slightly weathered, thinly laminated, grey silty fine SANDSTONE; very weak; very wide spaced defects (continued from layer starting at 9.1m )  10.80 m: very weak to weak  11.90 m: extremely to very weak; trace to minor fine gravel: quartz and schist, angular to sub-angular  12.40 - 16.50 m: CORELOSS *minimal silty fine sand and angular to sub-rounded, quartz and schist gravel recovered (Inferred weathered breccia)	HENLEY BRECCIA						PQTT				100 93					
										PQTT		SW		100 100 100					
										PQTT				27 20 16					
										PQTT				15 0 0					
										PQTT				0 0 0					
			End of Hole @ 16.50m, Target Depth.																
											</								

Groundwater at 13.93 mbgl



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Project	Smooth Hill Landfill Consenting	
Client	Dunedin City Council	
Job number	12506381	Page 1 of 2
Borehole ID	BH09	



Box 1 of 3: 0.0 m to 9.6 m



Box 2 of 3: 9.6 m to 11.7 m





Project	Smooth Hill Landfill Consenting	
Client	Dunedin City Council	
Job number	12506381	Page 2 of 2
Borehole ID	BH09	



Box 3 of 3: 11.7 m to 16.5 m (EOH)



Project : Smooth Hill Landfill Consenting  
Client : Dunedin City Council  
Site : Eastern Ridge  
Job Number: 12506381

Commenced: 4/06/2019

Completed: 5/06/2019

Hole No. : BH10a

Sheet : 1 of 2

Hole Length : 20.00m

Scale @ A4 : 1:50

Logged : MF

Processed : HB

Checked : JHS

Easting: 396788.26

Northing: 788118.5

System: TAIETM2000

RL: 139.07

Datum: NZVD2016

Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect (mm)	Instrumentation	Water level
						Number / Type	Result									
0		TOPSOIL; silt, trace fine sand, trace clay; brown. Firm to stiff, moist, low plasticity 0.25 - 1.60 m: CORELOSS	TS	M	F-St				PQTT				12			
1			LOESS						PQTT							
2		Fine sandy SILT; light grey and yellow-brown. Stiff, moist, low plasticity (LOESS)		M	St				PQTT				100			
3		Fine to coarse SAND, minor silt. Poorly graded; highly weathered rock (TARATU FORMATION) Pebbly coarse SAND. Poorly graded, highly weathered rock							PQTT		HW		93 93 93			
4		Moderately weathered, grey, orange-brown and yellow-brown SILTSTONE; very weak to weak, no defects 4.30 - 4.60 m: SANDSTONE							PQTT							
5		Moderately weathered, light grey and orange-brown fine SANDSTONE; very weak to weak; no defects; occasional fine gravel sized lignite fragments 5.50 m: 140 mm SILTSTONE	TARATU FORMATION						PQTT				86 86 86			
6									PQTT		MW		93 93 93			
7		Moderately weathered, orange-brown, white and grey CONGLOMERATE; very weak to weak; coarse sand matrix, matrix supported. Clasts: fine to medium gravel, quartz and schist, sub-angular to rounded Moderately weathered, light grey and orange-brown SILTSTONE; very weak to weak							PQTT				28 28 28			
8		Moderately weathered, orange-brown, white and grey CONGLOMERATE; very weak to weak; coarse sand matrix, matrix supported; clasts: fine to medium gravel, quartz and schist, sub-angular to rounded 7.72 - 8.80 m: CORELOSS							PQTT				75 75 75			
9		8.80 - 9.10 m: CORELOSS Moderately weathered, orange-brown, white and grey CONGLOMERATE; very weak to weak; coarse sand matrix, matrix supported. Clasts: fine to medium gravel, quartz and schist, sub-angular to rounded							PQTT		SW					

#### Notes and Comments:

End of Hole @ 20.00m, Target Depth.

Hole extended to find groundwater.  
Groundwater at 10.17 mbgl 07/06/2019.

Refer to explanation sheets for abbreviation and symbols

Inclination: Vertical

Orientation:

Contractor: McNeills


Equipment: UDR600 (truck mounted)

Shear Vane Id:

#### Ground Water Level

Date Time Reading (mbgl) Hole depth (mbgl)



<div></div> <div>Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Eastern Ridge Job Number: 12506381 Commenced: 4/06/2019 Completed: 5/06/2019</div>										Hole No. : BH10a Sheet : 2 of 2 Hole Length : 20.00m Scale @ A4 : 1:50 Logged : MF Processed : HB Checked : JHS									
Easting: 396788.26 RL: 139.07					Northing: 788118.5 Datum: NZVD2016					System: TAIETM2000									
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQR (%)	Defect Spacing (mm)	Instrumentation Installation	Water level		
							Number / Type	Result											
128	11		Slightly weathered, laminated to very thinly bedded, grey fine SANDSTONE; very weak to weak; occasional lignite (HENLEY BRECCIA) <i>(continued from layer starting at 9.6m )</i> 10.40 m: yellow-brown, fine to coarse sand grain size  11.40 - 11.60 m: weak to moderately strong 11.40 - 17.40 m: very weak to weak	HENLEY BRECCIA						PQTT				100 81 81					
127	12									PQTT					100 96 96				
126	13									PQTT					92 92 92				
125	14									PQTT					92 92 92				
124	15		15.50 m: with closely spaced thin to very thin beds of lignite							PQTT					89 89 89				
123	16									PQTT					100 100 100				
122	17		17.40 m: moderately strong to strong, well indurated 17.40 - 17.65 m: moderately strong to strong 17.65 - 19.20 m: very weak to weak							PQTT					100 100 100				
121	18									PQTT					100 100 100				
120	19		19.20 - 19.80 m: weak to moderately strong							PQTT					100 100 100				
			19.80 - 20.00 m: moderately strong to strong							PQTT					100 100 100				
Notes and Comments: End of Hole @ 20.00m, Target Depth. End of Hole @ 20.00m, Target Depth. Hole extended to find groundwater. Groundwater at 10.17 mbgl 07/06/2019. Refer to explanation sheets for abbreviation and symbols					Inclination: Vertical Orientation: Contractor: McNeills Equipment: UDR600 (truck mounted) Shear Vane Id:					Ground Water Level Date: 07/06/19 Time: 00:00 Reading (mbgl): 10.17 Hole depth (mbgl): 20									



Project : Smooth Hill Landfill Consenting  
 Client : Dunedin City Council  
 Site : Eastern Ridge  
 Job Number: 12506381

Commenced: 4/06/2019

Completed: 5/06/2019

Hole No. : BH10b

Sheet : 1 of 2

Hole Length : 20.00m

Scale @ A4 : 1:50

Logged : MF

Processed : HB

Checked : JHS

Easting: 396788.26

Northing: 788118.5

System: TAIETM2000

RL: 139.07

Datum: NZVD2016

Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect	Instrumentation	Water level
						Number / Type	Result									
0		TOPSOIL; silt, trace fine sand, trace clay; brown. Firm to stiff, moist, low plasticity 0.25 - 1.60 m: CORELOSS	TS	M	F-St				PQTT				12			
1			LOESS													
2		Fine sandy SILT; light grey and yellow-brown. Stiff, moist, low plasticity (LOESS)		M	St				PQTT				100			
3		Fine to coarse SAND, minor silt. Poorly graded; highly weathered rock (TARATU FORMATION) Pebbly coarse SAND. Poorly graded, highly weathered rock	TARATU FORMATION						PQTT		HW		93 93 93			
4		Moderately weathered, grey, orange-brown and yellow-brown SILTSTONE; very weak to weak, no defects 4.30 - 4.60 m: SANDSTONE							PQTT							
5		Moderately weathered, light grey and orange-brown fine SANDSTONE; very weak to weak; no defects; occasional fine gravel sized lignite fragments 5.50 m: 140 mm SILTSTONE							PQTT				86 86 86			
6									PQTT		MW		93 93 93			
7		Moderately weathered, orange-brown, white and grey CONGLOMERATE; very weak to weak; coarse sand matrix, matrix supported. Clasts: fine to medium gravel, quartz and schist, sub-angular to rounded Moderately weathered, light grey and orange-brown SILTSTONE; very weak to weak							PQTT				28 28 28			
8		Moderately weathered, orange-brown, white and grey CONGLOMERATE; very weak to weak; coarse sand matrix, matrix supported; clasts: fine to medium gravel, quartz and schist, sub-angular to rounded 7.72 - 8.80 m: CORELOSS							PQTT				75 75 75			
9		8.80 - 9.10 m: CORELOSS Moderately weathered, orange-brown, white and grey CONGLOMERATE; very weak to weak; coarse sand matrix, matrix supported. Clasts: fine to medium gravel, quartz and schist, sub-angular to rounded							PQTT		SW					

#### Notes and Comments:

End of Hole @ 20.00m, Target Depth.

Hole extended to find groundwater.  
 Groundwater at 18.36 mbgl 07/06/2019.

Refer to explanation sheets for abbreviation and symbols

Inclination: Vertical

Orientation:

Contractor: McNeills


Equipment: UDR600 (truck mounted)

Shear Vane Id:

#### Ground Water Level

Date Time Reading (mbgl) Hole depth (mbgl)



			Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Eastern Ridge Job Number: 12506381 Commenced: 4/06/2019 Completed: 5/06/2019						Hole No. : BH10b Sheet : 2 of 2 Hole Length : 20.00m Scale @ A4 : 1:50 Logged : MF Processed : HB Checked : JHS									
Easting: 396788.26 RL: 139.07			Northing: 788118.5 Datum: NZVD2016			System: TAIETM2000												
Elev (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQR (%)	Defect mm Spacing (mm)	Instrumentation Installation	Water level	
							Number / Type	Result										
128	11		Slightly weathered, laminated to very thinly bedded, grey fine SANDSTONE; very weak to weak; occasional lignite (HENLEY BRECCIA) <i>(continued from layer starting at 9.6m )</i> 10.40 m: yellow-brown, fine to coarse sand grain size	HENLEY BRECCIA						PQTT				100 81 81				
127	12		11.40 - 11.60 m: weak to moderately strong 11.40 - 17.40 m: very weak to weak							PQTT					100 96 96			
126	13									PQTT								
125	14									PQTT					92 92 92			
124	15		15.50 m: with closely spaced thin to very thin beds of lignite							PQTT					89 89 89			
123	16									PQTT								
122	17		17.40 m: moderately strong to strong, well indurated 17.40 - 17.65 m: moderately strong to strong 17.65 - 19.20 m: very weak to weak							PQTT					100 100 100			
121	18									PQTT					100 100 100			
120	19		19.20 - 19.80 m: weak to moderately strong							PQTT					100 100 100			
	20		19.80 - 20.00 m: moderately strong to strong							PQTT					100 100 100			
Notes and Comments: End of Hole @ 20.00m, Target Depth. End of Hole @ 20.00m, Target Depth. Hole extended to find groundwater. Groundwater at 18.36 mbgl 07/06/2019. Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical Orientation: Contractor: McNeills Equipment: UDR600 (truck mounted) Shear Vane Id:				Ground Water Level Date Time Reading (mbgl) Hole depth (mbgl)										
								07/06/19 00:00 18.36 20										



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Project	Smooth Hill Landfill Consenting	
Client	Dunedin City Council	
Job number	12506381	Page 1 of 4
Borehole ID	BH10	



Box 1 of 8: 0.0 m to 3.2 m

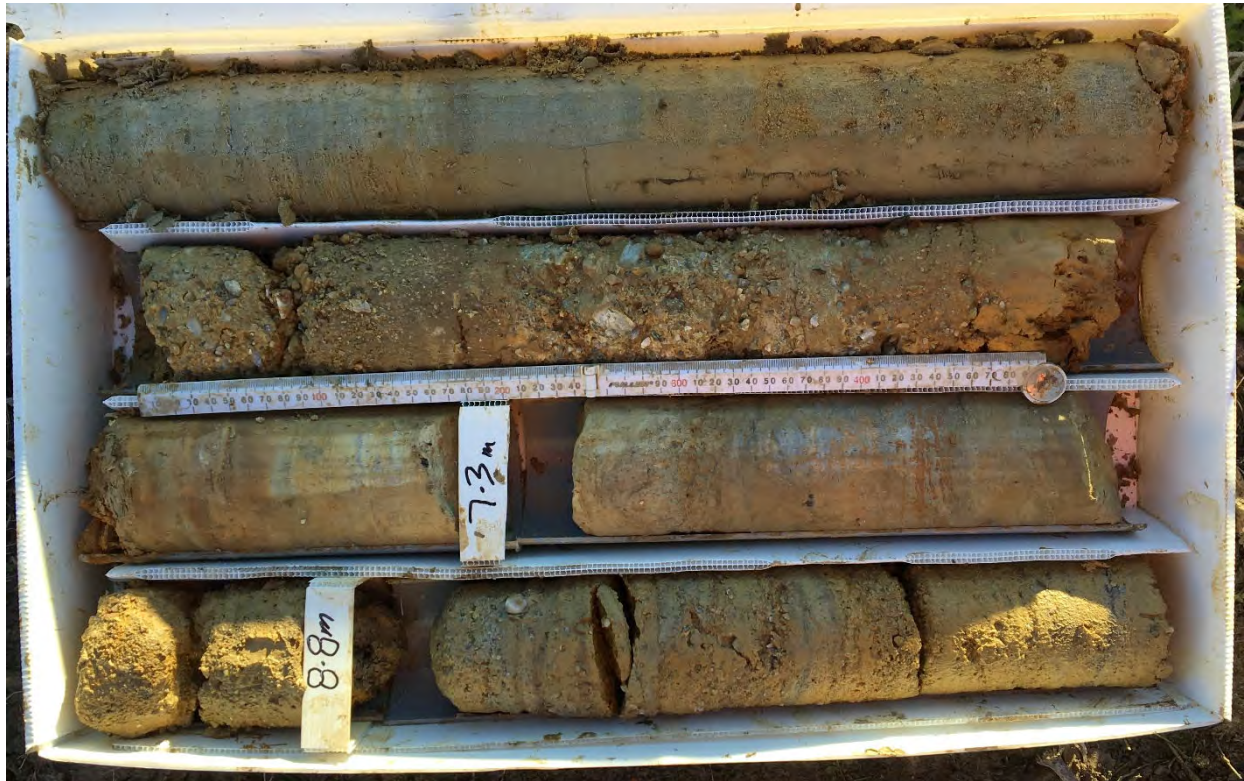


Box 2 of 8: 3.2 m to 5.8 m

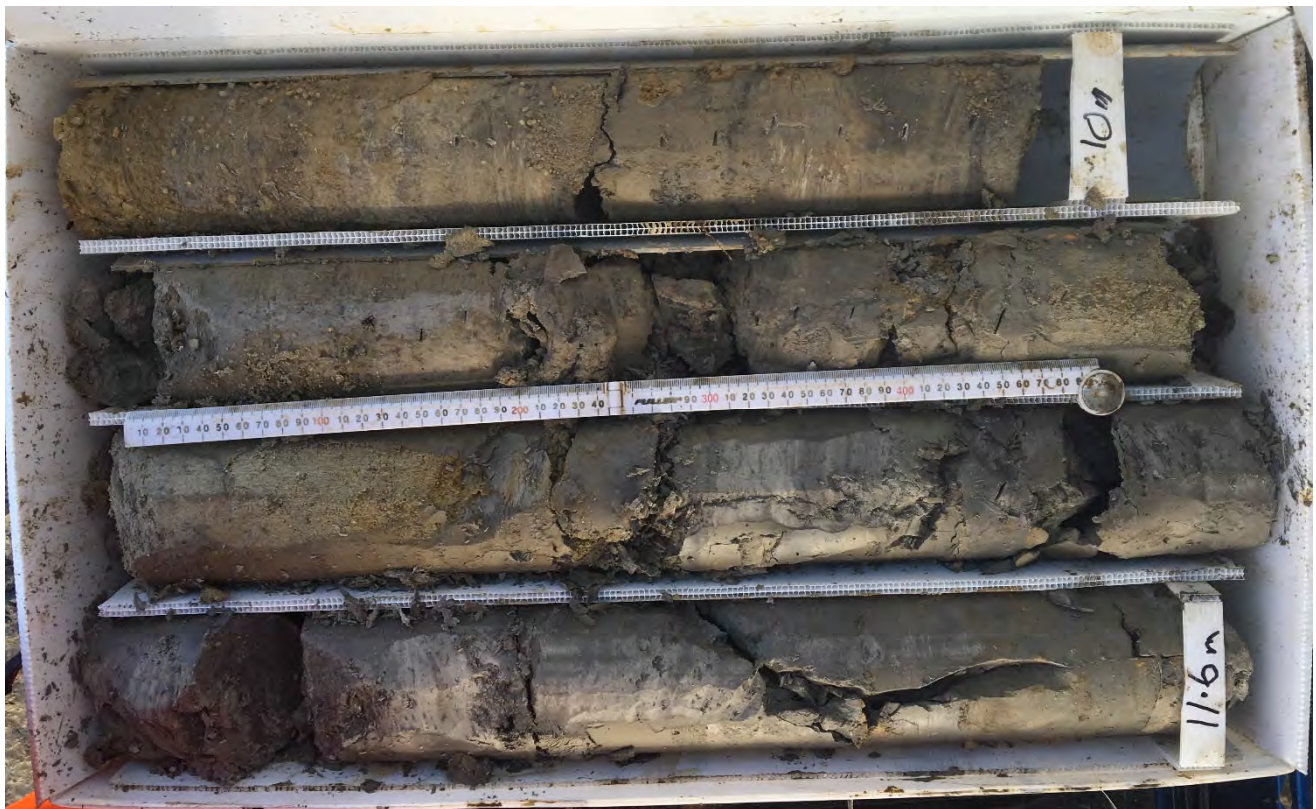




Project	Smooth Hill Landfill Consenting	
Client	Dunedin City Council	
Job number	12506381	Page 2 of 4
Borehole ID	BH10	



Box 3 of 8: 5.8 m to 9.2 m



Box 4 of 8: 9.2 m to 11.6 m

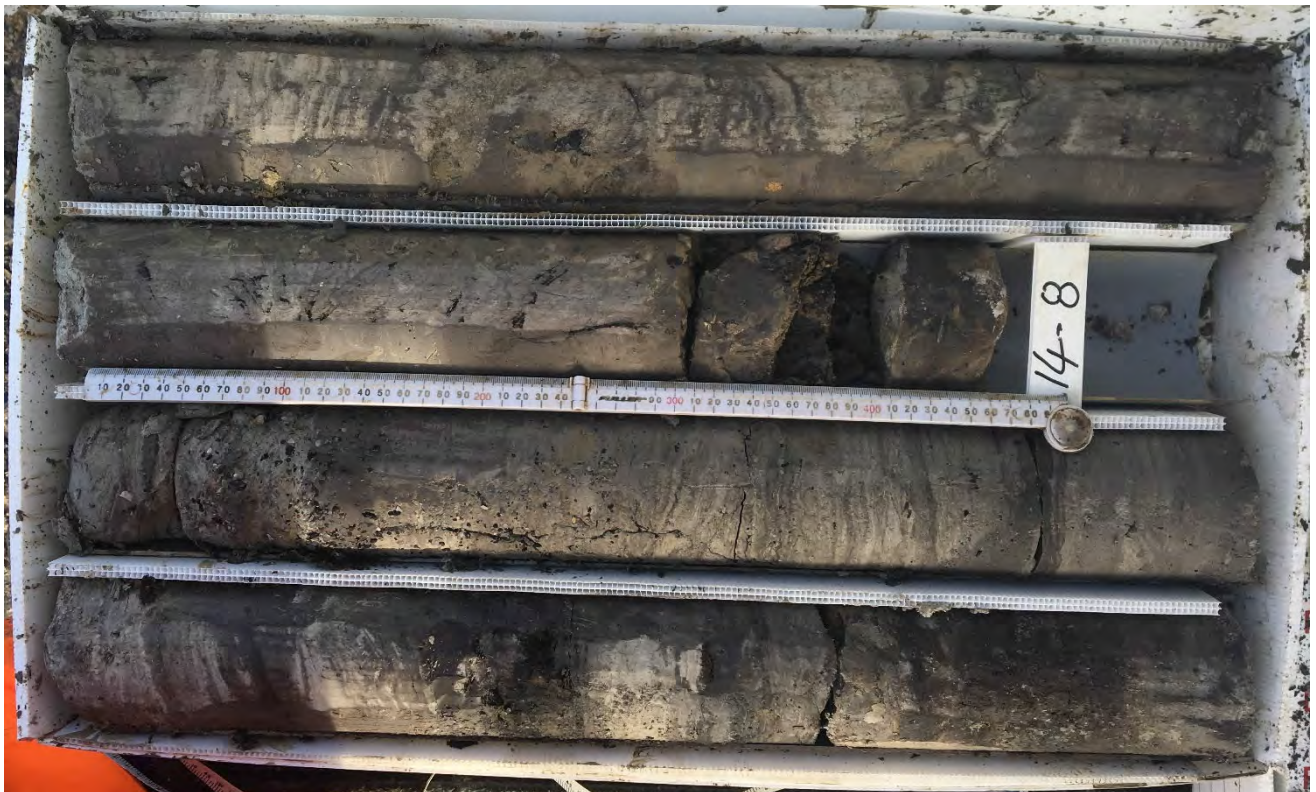




Project	Smooth Hill Landfill Consenting	
Client	Dunedin City Council	
Job number	12506381	Page 3 of 4
Borehole ID	BH10	



Box 5 of 8: 11.6 m to 13.8 m

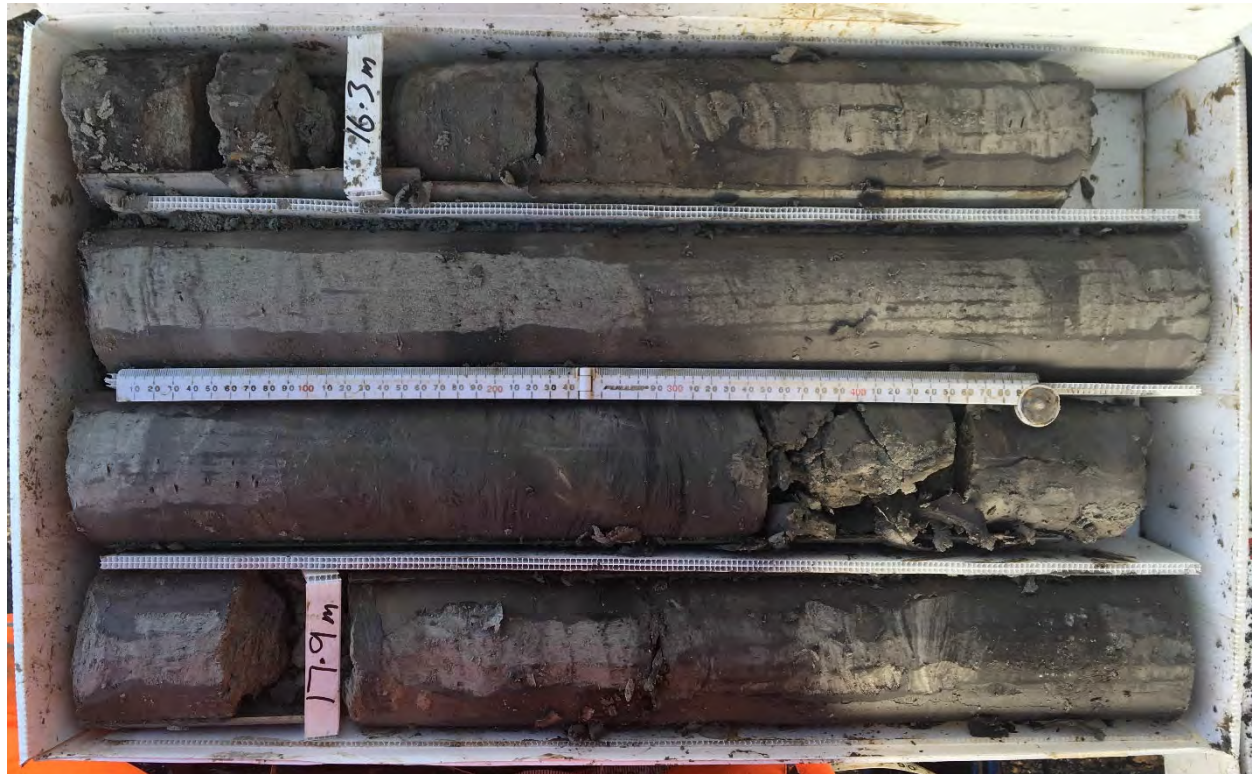


Box 6 of 8: 13.8 m to 16.0 m












Project	Smooth Hill Landfill Consenting	
Client	Dunedin City Council	
Job number	12506381	Page 4 of 4
Borehole ID	BH10	



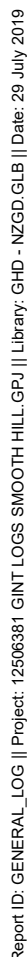
Box 7 of 8: 16.0 m to 18.4 m


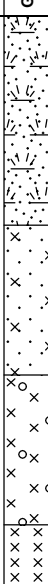


Box 8 of 8: 18.4 m to 20.0 m (EOH)

		Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Manuka Gully (Stockpile Area) Job Number: 12506381 Commenced: 12/06/2019 Completed: 12/06/2019						Hole No. : TP01 Sheet : 1 of 1 Hole Length : 2.50m Scale @ A4 : 1:25 Logged : MF Processed : HB Checked : MF									
Easting: 395988.85 RL: 121.2		Northing: 788077.23		System: TAIETM2000													
Datum: NZVD2016																	
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Number / Type	Result	Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQR (%)	Defect Spacing (mm)	Instrumentation Installation	Water level
121	0.2		SILT, trace to minor clay; dark grey-brown. Firm, wet, low plasticity; minor to some organics/roots (TOPSOIL)	TS	W	F											
	0.7		SILT, trace clay, trace fine to medium sand; light grey with orange streaks. Stiff to very stiff, moist, low plasticity; iron-stained organics throughout (ALLUVIUM)	ALLUVIUM	M	St-VSt											
120	1.0		Gravelly SILT, minor clay, minor fine to coarse sand; orange-brown. Very stiff, moist to wet, low plasticity; gravel: fine to medium, quartz and schist, sub-angular to rounded; highly weathered rock (HENLEY BRECCIA)		M-W	VSt											
	1.10		1.10 m: grey with some orange-brown	HENLEY BRECCIA													
119	2.0		Slightly weathered, grey with black streaks SILTSTONE; very weak; ripped easily with toothed excavator bucket														
	2.50		End of Hole @ 2.50m, Target Depth.														
118	3.0																
	4.0																
117	5.0																
Notes and Comments: End of Hole @ 2.50m, Target Depth.  Soils too gravelly for shear vane. Groundwater seepage into test pit at 1.0 mbgl  Refer to explanation sheets for abbreviation and symbols			Inclination: Vertical Orientation:  Contractor: Fulton Hogan  Equipment: 22t excavator - toothed bucket  Shear Vane Id:			Ground Water Level  Date Time Reading (mbgl) Hole depth (mbgl)											


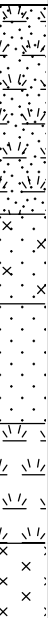





		Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Manuka Gully (Stockpile Area) Job Number: 12506381 Commenced: 12/06/2019      Completed: 12/06/2019						Hole No. : TP03 Sheet : 1 of 1 Hole Length : 2.00m Scale @ A4 : 1:25										
		Easting: 396262.16      Northing: 788048.16      System: TAIETM2000 RL: 102.61      Datum: NZVD2016						Logged : MF Processed : HB Checked : MF										
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQR (%)	Defect	Spacing (mm)	Instrumentation Installation	Water level
							Number / Type	Result										
102	0		SILT, trace to minor fine sand, trace to minor clay; brown. Soft, moist to wet, low plasticity; minor organics/roots (TOPSOIL)	TOPSOIL	M-W	S												
1	0.7		Silty SAND, trace clay; light grey with brown streaks. Moist, poorly graded; sand is fine (ALLUVIUM)	ALLUVIUM	M													
101	1.2		Gravelly SILT; grey. Wet to saturated, well graded; gravel: fine to coarse	ALLUVIUM	W - S													
100	1.7		Slightly weathered, grey SILTSTONE; extremely to very weak; no defects - ripped easily (HENLEY BRECCIA)	HB								SW						
99	2	End of Hole @ 2.00m, Target Depth.																
98	3																	
	4																	
	5																	

<b>Notes and Comments:</b> End of Hole @ 2.00m, Target Depth.  Test pit sides too soft to get shear vane readings. Groundwater encountered at 1.2 mbgl.  Refer to explanation sheets for abbreviation and symbols		Inclination: Vertical		Orientation:		Ground Water Level			
		Contractor: Fulton Hogan		Equipment: 22t excavator - toothed bucket		Date	Time	Reading (mbgl)	Hole depth (mbgl)
		Shear Vane Id:		12/06/19	00:00	1.2	2		




		Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Southwest Gully Base Job Number: 12506381 Commenced: 13/06/2019      Completed: 13/06/2019				Hole No. : TP05 Sheet : 1 of 1 Hole Length : 3.30m Scale @ A4 : 1:25												
		Easting: 396281      Northing: 787868      System: TAIETM2000 RL: 125      Datum: NZVD2016				Logged : MF Processed : HB Checked : MF												
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect Spacing (mm)	Instrumentation Installation	Water level	
							Number / Type	Result										
124	0		SILT, minor clay; brown. Soft, wet to saturated, low plasticity; minor organics throughout (TOPSOIL)	TOPSOIL	W-S	S												
	0.7		Silty fine to medium SAND; grey and yellow-brown. 'Loose', wet, poorly graded (COLLUVIUM)	COLLUVIUM	W	'L'												
	1		Fine to coarse SAND, minor to some silt; grey with black streaks. 'Loose', saturated, poorly graded; organics throughout		S	'L'												
	1.4		Tree trunks and branches with some gravel. Groundwater outflow from base of layer															
	1.8		SILT, minor clay, trace fine sand; grey with yellow-brown streaks. Stiff, moist, low plasticity; highly weathered rock (HENLEY BRECCIA)	HENLEY BRECCIA	M	St												
2	Slightly weathered, SILTSTONE; ripped easily																	
123	2.8		End of Hole @ 3.30m, Target Depth.															
122	3																	
121	4																	
120	5																	


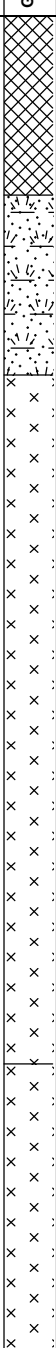
Notes and Comments: End of Hole @ 3.30m, Target Depth.  Refer to explanation sheets for abbreviation and symbols		Inclination: Vertical      Orientation:		Ground Water Level			
		Contractor: Fulton Hogan Equipment: 22t excavator - toothed bucket Shear Vane Id:		Date	Time	Reading (mbgl)	Hole depth (mbgl)
				13/06/19	00:00	1.9	3.3

			Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Gully East of Central Ridge Job Number: 12506381 Commenced: 13/06/2019      Completed: 13/06/2019						Hole No. : TP06 Sheet : 1 of 1 Hole Length : 2.50m Scale @ A4 : 1:25 Logged : MF Processed : HB Checked : MF										
Easting: 396585.7      Northing: 787800.45      System: TAIETM2000 RL: 108.24      Datum: NZVD2016																			
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQR (%)	Defect Spacing (mm)	Instrumentation Installation	Water level		
							Number / Type	Result											
108	0		Organic SILT, minor clay; dark brown-grey. Firm, moist, low plasticity; minor to some roots (TOPSOIL)	TOPSOIL	M	F													
	0.5		SILT, minor clay, light grey with orange-brown streaks. Stiff, moist, low plasticity; minor organic inclusions (ALLUVIUM)	ALLUVIUM	M	St													
	2.3		SILTSTONE; difficult to rip (HENLEY BRECCIA)	HB															
			End of Hole @ 2.50m, Target Depth.																
	3																		
	4																		
Notes and Comments: End of Hole @ 2.50m, Target Depth.  Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical      Orientation: Contractor: Fulton Hogan Equipment: 22t excavator - toothed bucket Shear Vane Id:				Ground Water Level <table border="1"> <tr> <th>Date</th> <th>Time</th> <th>Reading (mbgl)</th> <th>Hole depth (mbgl)</th> </tr> <tr> <td>13/06/19</td> <td>00:00</td> <td></td> <td>2.5</td> </tr> </table>				Date	Time	Reading (mbgl)	Hole depth (mbgl)	13/06/19	00:00		2.5
Date	Time	Reading (mbgl)	Hole depth (mbgl)																
13/06/19	00:00		2.5																




			Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Southwest Gully Base Job Number: 12506381 Commenced: 28/05/2019      Completed: 28/05/2019						Hole No. : TP07 Sheet : 1 of 1 Hole Length : 2.50m Scale @ A4 : 1:25 Logged : MF Processed : HB Checked : MF									
Easting: 396182      Northing: 787790      System: TAIETM2000 RL: 120      Datum: NZVD2016																		
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQR (%)	Defect	Spacing (mm)	Instrumentation Installation	Water level
							Number / Type	Result										
119	0		SILT/organic matter, brown. Soft, moist to saturated, fibrous, non plastic (TOPSOIL)	TOPSOIL	M-W	S												
119	0.5		SILT, minor clay, trace fine sand; light grey and yellow-brown. Stiff to very stiff, moist, low plasticity (LOESS)	LOESS	M	St-VS				TP								
118	1.4		Slightly weathered, grey SILTSTONE; weak to moderately strong; no defects (HENLEY BRECCIA)	HENLEY BRECCIA							SW							
117	2.4		BRECCIA															
117			End of Hole @ 2.50m, Target Depth.															
116	3																	
115	4																	
Notes and Comments: End of Hole @ 2.50m, Target Depth.  Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical      Orientation:				Ground Water Level										
				Contractor: Fulton Hogan Equipment: 22t excavator - smooth bucket Shear Vane Id:				Date: 28/05/19      Time: 00:00      Reading (mbgl): 1.4      Hole depth (mbgl): 2.5										

Water level at 2.4m depth


		Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Gully Between Southern Ridges Job Number: 12506381 Commenced: 28/05/2019      Completed: 28/05/2019						Hole No. : TP08 Sheet : 1 of 1 Hole Length : 4.50m Scale @ A4 : 1:25										
		Easting: 396303      Northing: 787682      System: TAIETM2000 RL: 115      Datum: NZVD2016						Logged : MF Processed : HB Checked : MF										
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQR (%)	Defect Spacing (mm)	Instrumentation Installation	Water level	
							Number / Type	Result										
114	0		SILT, minor clay, trace fine sand; dark grey. Firm to stiff, moist, low plasticity; minor organic matter (FILL)	FILL	M	F-St												
	0.6		0.50 m: grass and trees - buried surface, saturated		S													
	1		SILT, minor clay, trace fine sand; dark grey. Firm to stiff, wet, low plasticity; trace to minor organics (BURIED TOPSOIL)	BURIED TOPSOIL	W	F-St												
	1.2		SILT, minor to some clay, trace fine sand; light grey and yellow-brown. Stiff to very stiff, moist, low plasticity; trace organics (LOESS)	LOESS	M	St-VSt												
113	2																	
	3																	
112	3																	
	3.5																	
111	4		SILT, some coarse sand, minor fine gravel; light grey. Stiff to very stiff, moist, non-plastic; gravel comprises quartz and schist; highly weathered rock (HENLEY BRECCIA)	HENLEY BRECCIA	M	St-VSt					HW							
	4.5		End of Hole @ 4.50m, End of Reach.															
110	5																	

Notes and Comments: End of Hole @ 4.50m, End of Reach.  Refer to explanation sheets for abbreviation and symbols		Inclination: Vertical      Orientation:		Ground Water Level			
		Contractor: Fulton Hogan Equipment: 22t excavator - smooth bucket Shear Vane Id:		Date	Time	Reading (mbgl)	Hole depth (mbgl)
				28/05/19	00:00		4.5

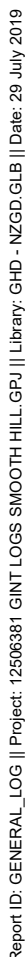





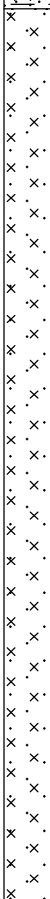
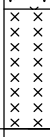
			Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : South East Gully Outflow Job Number: 12506381 Commenced: 13/06/2019      Completed: 13/06/2019						Hole No. : TP09 Sheet : 1 of 1 Hole Length : 3.00m Scale @ A4 : 1:25 Logged : MF Processed : MF Checked : MF								
Easting: 396577.97      Northing: 787947.86      System: TAIETM2000 RL: 101.04      Datum: NZVD2016																	
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQR (%)	Defect Spacing (mm)	Instrumentation Installation	Water level
							Number / Type	Result									
101.04	0		SILT, minor fine to coarse gravel; yellow-brown and grey. Stiff, moist, low plasticity; minor organic content (SLIP DEBRIS)	SLIP DEBRIS	M	St											
	0.5		Branches and grass (BURIED VEGETATION)														
	0.7		SILT, minor clay; brown. Firm to stiff, moist, low plasticity (BURIED TOPSOIL)	BTS	M	F-St											
	1.00		Gravelly silty SAND; orange-brown. Moist, poorly graded; gravel is fine; sand is fine to coarse (ALLUVIUM)		M												
	1.30		1.00 m: light grey and orange-brown 1.30 m: light grey with orange-brown streaks	ALLUVIUM													
	2.7		Fine SANDSTONE; easily ripped (HENLEY BRECCIA)	HB													
	3.00		End of Hole @ 3.00m, Target Depth.														
	4.0																
	5.0																

Notes and Comments: End of Hole @ 3.00m, Target Depth. Test pit dug to side of gully base - too boggy in gully base to excavate Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical		Orientation:		Ground Water Level			
				Contractor:		Equipment: 22t Excavator		Date	Time	Reading (mbgl)	Hole depth (mbgl)
				Shear Vane Id:							

<div></div> <div>Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Future Laydown Area Job Number: 12506381 Commenced: 10/06/2019 Completed: 10/06/2019</div>										<div>Hole No. : TP10</div> <div>Sheet : 1 of 1 Hole Length : 3.60m Scale @ A4 : 1:25</div> <div>Logged : MF Processed : HB Checked : MF</div>																			
Easting: 396820.11 RL: 140.74										Northing: 788079.25 Datum: NZVD2016										System: TAIETM2000									
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect Spacing (mm)	Instrumentation Installation	Water level												
							Number / Type	Result																					
140   <																													






<div></div>			Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Future Laydown Area Job Number: 12506381 Commenced: 10/06/2019					Hole No. : TP12 Sheet : 1 of 1 Hole Length : 4.40m Scale @ A4 : 1:25									
Easting: 396596.93 RL: 142.28			Northing: 787986.46 Datum: NZVD2016		System: TAIETM2000		Completed: 10/06/2019 Logged : MF Processed : HB Checked : MF										
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect Spacing (mm)	Instrumentation Installation	Water level
							Number / Type	Result									
142	0		SILT, minor clay, trace fine sand, dark grey and brown. Stiff to very stiff, moist, low plasticity. Trace to minor roots (FILL)	FILL	M	St-VSt		SV@0.4m 139/44 kPa									
	0.7		Sandy SILT, grey. Very stiff, dry, non-plastic; some large roots extend to approximately 1.2 m bgl; trace organics; sand is fine (BURIED TOPSOIL).	BTS	D	VSt		SV@1m UTP									
141	1		Sandy SILT; light grey, light yellow-brown and orange-brown. Very stiff, dry, non-plastic; sand is fine; occasional roots to 1.2 m bgl; strength increases with depth (LOESS)		D	VSt		SV@2m UTP									
	2		2.50 m: 50-100 mm iron-stained layer	LOESS				SV@2.9m UTP									
	3		3.60 m: 50-100 mm iron-stained layer														
139	4		Highly weathered, SILTSTONE (HENLEY BRECCIA)	HB				SV@4.4m UTP				HW					
	4		End of Hole @ 4.40m, Target Depth.														
Notes and Comments:				Inclination: Vertical		Orientation:		Ground Water Level									
End of Hole @ 4.40m, Target Depth.				Contractor: Fulton Hogan		Equipment: 22t excavator		Date		Time		Reading (mbgl)		Hole depth (mbgl)			
EOH at 4.4 mbgl, deepest excavator could excavate soil. Groundwater not encountered.				Shear Vane Id: GEO2288				10/06/19		00:00				4.4			
Refer to explanation sheets for abbreviation and symbols																	







		Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Southern Boundary Job Number: 12506381 Commenced: 29/10/2019 Completed: 1/11/2019						Hole No. : BH201 Sheet : 2 of 7 Hole Length : 61.00m Scale @ A4 : 1:50 Logged : MF Processed : MF Checked : JHS									
Easting: 396596 RL: 144		Northing: 787540 Datum: NZVD2016		System: TAIETM2000													
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SQR RQD (%)	Defect	Instrumentation	Water level
			Slightly weathered, grey and light grey BRECCIA; weak to moderately strong; no defects; matrix supported; fine to medium sand matrix; clasts are angular to subrounded, quartz and schist, fine to coarse gravel ( <i>continued from layer starting at 8.6m</i> ) 10.8 m to 61.0 m: Wash drilling				Number / Type	Result		PQTT		SW		100 29			
10.8																	
11																	
12																	
13																	
14																	
15																	
16																	
17																	
18																	
19																	
20																	


Notes and Comments: End of Hole @ 61.00m, Target Depth.  0.0 - 10.8 m PQTT coring 10.8 - 61.0 m wash drilling  Refer to explanation sheets for abbreviation and symbols		Inclination: Vertical		Orientation:		Ground Water Level			
		Contractor: Speight Drilling		Equipment: Track Mounted Rig		Date	Time	Reading (mbgl)	Hole depth (mbgl)
		Shear Vane Id:							




		Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Southern Boundary Job Number: 12506381 Commenced: 29/10/2019      Completed: 1/11/2019						Hole No. : BH201 Sheet : 3 of 7 Hole Length : 61.00m Scale @ A4 : 1:50									
		Easting: 396596      Northing: 787540      System: TAIETM2000 RL: 144      Datum: NZVD2016						Logged : MF Processed : MF Checked : JHS									
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR / SCR / RQR (%)	Defect / Spacing (mm)	Instrumentation	Water level
			10.8 m to 61.0 m: Wash drilling (continued from layer starting at 10.8m )							Wash drilled							
123	21																20
122	22																21
121	23																22
120	24																23
119	25																24
118	26																25
117	27																26
116	28																27
115	29																28
114																	29
<b>Notes and Comments:</b> End of Hole @ 61.00m, Target Depth. 0.0 - 10.8 m PQTT coring 10.8 - 61.0 m wash drilling Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical      Orientation:				Ground Water Level				Date      Time      Reading (mbgl)      Hole depth (mbgl)					
				Contractor: Speight Drilling Equipment: Track Mounted Rig Shear Vane Id:													

			Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Southern Boundary Job Number: 12506381 Commenced: 29/10/2019      Completed: 1/11/2019						Hole No. : BH201 Sheet : 4 of 7 Hole Length : 61.00m Scale @ A4 : 1:50 Logged : MF Processed : MF Checked : JHS								
Easting: 396596      Northing: 787540      System: TAIETM2000 RL: 144      Datum: NZVD2016																	
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQR (%)	Defect Spacing (mm)	Instrumentation Installation	Water level
							Number / Type	Result									
			10.8 m to 61.0 m: Wash drilling (continued from layer starting at 10.8m )														
113	31																30
112	32																31
111	33																32
110	34																33
109	35																34
108	36																35
107	37																36
106	38																37
105	39																38
104	40																39
Notes and Comments: End of Hole @ 61.00m, Target Depth. 0.0 - 10.8 m PQTT coring 10.8 - 61.0 m wash drilling Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical      Orientation:				Ground Water Level									
				Contractor: Speight Drilling Equipment: Track Mounted Rig Shear Vane Id:				Date      Time      Reading (mbgl)      Hole depth (mbgl)									




			Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Southern Boundary Job Number: 12506381 Commenced: 29/10/2019      Completed: 1/11/2019						Hole No. : BH201 Sheet : 5 of 7 Hole Length : 61.00m Scale @ A4 : 1:50 Logged : MF Processed : MF Checked : JHS								
Easting: 396596      Northing: 787540      System: TAIETM2000 RL: 144      Datum: NZVD2016																	
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR / SCR / RQR (%)	Defect / Spacing (mm)	Instrumentation	Water level
			10.8 m to 61.0 m: Wash drilling (continued from layer starting at 10.8m )							Wash drilled							
103	41																41
102	42																42
101	43																43
100	44																44
99	45																45
98	46																46
97	47																47
96	48																48
95	49																49
94																	50

<b>Notes and Comments:</b> End of Hole @ 61.00m, Target Depth.  0.0 - 10.8 m PQTT coring 10.8 - 61.0 m wash drilling  Refer to explanation sheets for abbreviation and symbols			Inclination: Vertical      Orientation:		Ground Water Level			
			Contractor: Speight Drilling		Date	Time	Reading (mbgl)	Hole depth (mbgl)
			Equipment: Track Mounted Rig					
		Shear Vane Id:						

			Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Southern Boundary Job Number: 12506381 Commenced: 29/10/2019 Completed: 1/11/2019						Hole No. : BH201 Sheet : 6 of 7 Hole Length : 61.00m Scale @ A4 : 1:50 Logged : MF Processed : MF Checked : JHS											
Easting: 396596 RL: 144			Northing: 787540 Datum: NZVD2016			System: TAIETM2000														
RL (m)	Depth (m)	Graphic	Material Description				Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQR (%)	Defect Spacing (mm)	Instrumentation Installation	Water level
			10.8 m to 61.0 m: Wash drilling (continued from layer starting at 10.8m )																	
51																				
52																				
53																				
54																				
55																				
56																				
57																				
58																				
59																				
60																				

Notes and Comments:		Inclination: Vertical		Orientation:		Ground Water Level	
End of Hole @ 61.00m, Target Depth.		Contractor: Speight Drilling		Equipment: Track Mounted Rig		Date	Time
0.0 - 10.8 m PQTT coring 10.8 - 61.0 m wash drilling		Shear Vane Id:				Reading (mbgl)	Hole depth (mbgl)
Refer to explanation sheets for abbreviation and symbols							



		Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Southern Boundary Job Number: 12506381 Commenced: 29/10/2019      Completed: 1/11/2019						Hole No. : BH201 Sheet : 7 of 7 Hole Length : 61.00m Scale @ A4 : 1:50									
		Easting: 396596      Northing: 787540      System: TAIETM2000 RL: 144      Datum: NZVD2016						Logged : MF Processed : MF Checked : JHS									
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect Spacing (mm)	Instrumentation Installation	Water level
							Number / Type	Result									
61	0.0		10.8 m to 61.0 m: Wash drilling (continued from layer starting at 10.8m )							Wash drilled							
61	0.0		End of Hole @ 61.00m, Target Depth.														
62	0.0																
63	0.0																
64	0.0																
65	0.0																
66	0.0																
67	0.0																
68	0.0																
69	0.0																
70	0.0																

Notes and Comments: End of Hole @ 61.00m, Target Depth.  0.0 - 10.8 m PQTT coring 10.8 - 61.0 m wash drilling  Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical      Orientation:		Ground Water Level			
				Contractor: Speight Drilling Equipment: Track Mounted Rig Shear Vane Id:		Date	Time	Reading (mbgl)	Hole depth (mbgl)

## Report of Photographs

Site Identification: BH201

Project	Waste Futures WS3 – Smooth Hill	Commenced	28/10/2019	Completed	01/11/2019
Site	Southern Boundary	Logged By	MF		
Job #	12506381	Checked By			
Client	Dunedin City Council	Core Depth	0.0 m – 10.8 m		



Box 1 of 5: 0.00 m to 2.30 m



Box 2 of 5: 2.30 m to 4.50 m



## Report of Photographs

### Site Identification: BH201

<b>Project</b>	Waste Futures WS3 – Smooth Hill	<b>Commenced</b>	28/10/2019	<b>Completed</b>	01/11/2019
<b>Site</b>	Southern Boundary	<b>Logged By</b>	MF		
<b>Job #</b>	12506381	<b>Checked By</b>			
<b>Client</b>	Dunedin City Council	<b>Core Depth</b>	0.0 m – 10.8 m		



Box 3 of 5: 4.50 m to 6.80 m



Box 4 of 5: 6.80 m to 9.00 m





## Report of Photographs

Site Identification: BH201


Project	Waste Futures WS3 – Smooth Hill	Commenced	28/10/2019	Completed	01/11/2019
Site	Southern Boundary	Logged By	MF		
Job #	12506381	Checked By			
Client	Dunedin City Council	Core Depth	0.0 m – 10.8 m		




Box 5 of 5: 9.00 m to 10.80 m

10.80 m to 61.00 m (EOH) – Wash drilled, no core recovered



		Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Southern Boundary Job Number: 12506381 Commenced: 2/11/2019 Completed: 4/11/2019						Hole No. : BH202 Sheet : 1 of 7 Hole Length : 60.60m Scale @ A4 : 1:50 Logged : MF Processed : MF Checked : JHS									
Easting: 396181 RL: 144		Northing: 787498 Datum: NZVD2016		System: TAIETM2000													
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect Spacing (mm)	Instrumentation Installation	Water level
			SILT, trace to minor clay; grey and orange-brown. Firm to stiff, moist, low plasticity (FILL/COLLUVIUM?) 0.30 - 0.80 m: brown and grey, very stiff	FILL/COLLUVIUM?	M	F-St											
			0.80 - 0.90 m: trace clay, grey, stiff, minor to some organic matter (roots) 0.90 - 1.20 m: grey and brown, very stiff, trace iron-oxide nodules, "chaotic" texture 1.20 - 1.60 minor iron-oxide nodules			VSt				PQTT				100			
			SILT, trace to minor clay; dark brown. Very stiff, moist, low plasticity; small branches (BURIED TOPSOIL) SILT, trace clay; orange-brown and grey; Very stiff to hard, moist, non-plastic; trace iron-oxide nodules; completely weathered (HENLEY BRECCIA) 2.60 - 2.80 m: minor to some iron-oxide nodules - increases with depth		M	VSt-H				PQTT				100			
			Gravelly, sandy SILT; orange-brown, brown and grey; Very stiff to hard, moist, non-plastic; sand is medium to coarse; gravel is fine to medium, angular to subrounded, quartz and schist; completely weathered 4.10 - 4.50 m: CORE LOSS 4.50 - 5.70 m: CORE LOSS	HENLEY BRECCIA	M	VSt-H				PQTT				94	0		
										PQTT				0			
										PQTT				0			
			Gravelly, sandy SILT; orange-brown, brown and grey; Very stiff to hard, moist, non-plastic; sand is medium to coarse; gravel is fine to medium, angular to subrounded, quartz and schist; completely weathered SILT, trace to minor clay; grey and brown. Very stiff to hard, moist, non-plastic; completely weathered		M	VSt-H				PQTT				100			
			Gravelly, silty SAND; orange-brown. Moist; well sorted; sand is fine to coarse; gravel is fine to medium, angular to subrounded, quartz and schist; completely weathered Gravelly, sandy SILT; orange-brown and grey. Firm, moist, non-plastic; sand is fine to coarse; gravel is fine to medium, angular to subrounded, quartz and schist; completely weathered		M					PQTT				95			
			Highly weathered, light brown, thinly to moderately thickly bedded SILTSTONE; extremely weak; no defects. Soil description: SILT, minor clay; hard Moderately weathered, dark grey, thinly bedded SILTSTONE; extremely to very weak; no defects; trace to minor lignite Moderately weathered, light grey, fine to coarse grained SANDSTONE; very weak; no defects Moderately weathered, black, LIGNITE; very weak, no defects Slightly weathered, light grey and grey, thinly to moderately thickly bedded, fine to medium grained SANDSTONE; very weak; no defects; occasional lignite layers 8.60 - 8.85 m: fine to coarse sand 8.99 - 9.02 m: 30 mm lignite		M	F				PQTT				100	83		
									PQTT					100	90		
Notes and Comments: End of Hole @ 60.60m, Target Depth. 0.0 - 10.6 m PQTT coring 10.6 - 60.6 m wash drilling Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical		Orientation:		Ground Water Level									
				Contractor: Speight Drilling								Date	Time	Reading (mbgl)	Hole depth (mbgl)		
				Equipment: Track mounted rig													
				Shear Vane Id:													

<div></div> <div>Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Southern Boundary Job Number: 12506381 Commenced: 2/11/2019 Completed: 4/11/2019</div>										<div>Hole No. : BH202 Sheet : 2 of 7 Hole Length : 60.60m Scale @ A4 : 1:50</div> <div>Logged : MF Processed : MF Checked : JHS</div>									
Easting: 396181 RL: 144					Northing: 787498 Datum: NZVD2016					System: TAIETM2000									
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SQR RQR (%)	Defect Spacing (mm)	Instrumentation Installation	Water level		
							Number / Type	Result											
			9.84 - 9.89 m: 50 mm lignite Slightly weathered, light grey and grey, thinly to moderately thickly bedded, fine to medium grained SANDSTONE; very weak; no defects; occasional lignite layers ( <i>continued from layer starting at 8.5m</i> ) 10.30 - 10.45 m: 150 mm lignite 10.60 m to 60.60 m: Wash drilled	HENLEY BRECCIA						PQTT		SW		100 90					
										Wash drilling									

## Notes and Comments:

End of Hole @ 60.60m, Target Depth.

0.0 - 10.6 m PQTT coring  
10.6 - 60.6 m wash drilling

Refer to explanation sheets for abbreviation and symbols

Inclination: Vertical

Orientation:

Contractor: Speight Drilling


Equipment: Track mounted rig

Shear Vane Id:


## Ground Water Level

Date      Time      Reading (mbgl)      Hole depth (mbgl)





			Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Southern Boundary Job Number: 12506381 Commenced: 2/11/2019      Completed: 4/11/2019						Hole No. : BH202 Sheet : 3 of 7 Hole Length : 60.60m Scale @ A4 : 1:50								
			Easting: 396181      Northing: 787498      System: TAIETM2000 RL: 144      Datum: NZVD2016						Logged : MF Processed : MF Checked : JHS								
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR / SCR / RQR (%)	Defect / Spacing (mm)	Instrumentation	Water level
			10.60 m to 60.60 m: Wash drilled (continued from layer starting at 10.6m )	HENLEY BRECCIA						Wash drilling							
123	21																20
122	22																21
121	23																22
120	24																23
119	25																24
118	26																25
117	27																26
116	28																27
115	29																28
114																	29
																	30

<b>Notes and Comments:</b> End of Hole @ 60.60m, Target Depth.  0.0 - 10.6 m PQTT coring 10.6 - 60.6 m wash drilling  Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical      Orientation:		Ground Water Level			
				Contractor: Speight Drilling Equipment: Track mounted rig Shear Vane Id:		Date	Time	Reading (mbgl)	Hole depth (mbgl)

			Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Southern Boundary Job Number: 12506381 Commenced: 2/11/2019 Completed: 4/11/2019						Hole No. : BH202 Sheet : 4 of 7 Hole Length : 60.60m Scale @ A4 : 1:50 Logged : MF Processed : MF Checked : JHS											
Easting: 396181 RL: 144			Northing: 787498 Datum: NZVD2016			System: TAIETM2000														
RL (m)	Depth (m)	Graphic	Material Description				Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect Spacing (mm)	Instrumentation Installation	Water level
			10.60 m to 60.60 m: Wash drilled (continued from layer starting at 10.6m )				HENLEY BRECCIA						Wash drilling							
113	31																			30
112	32																			31
111	33																			32
110	34																			33
109	35																			34
108	36																			35
107	37																			36
106	38																			37
105	39																			38
104	40																			39
Notes and Comments: End of Hole @ 60.60m, Target Depth.  0.0 - 10.6 m PQTT coring 10.6 - 60.6 m wash drilling  Refer to explanation sheets for abbreviation and symbols			Inclination: Vertical			Orientation:			Ground Water Level											
			Contractor: Speight Drilling						Date	Time	Reading (mbgl)	Hole depth (mbgl)								
			Equipment: Track mounted rig																	
			Shear Vane Id:																	




			Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Southern Boundary Job Number: 12506381 Commenced: 2/11/2019      Completed: 4/11/2019						Hole No. : BH202 Sheet : 5 of 7 Hole Length : 60.60m Scale @ A4 : 1:50 Logged : MF Processed : MF Checked : JHS										
Easting: 396181      Northing: 787498      System: TAIETM2000 RL: 144      Datum: NZVD2016																			
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR / SCR / RQR (%)	Defect / Spacing (mm)	Instrumentation	Water level		
			10.60 m to 60.60 m: Wash drilled (continued from layer starting at 10.6m )	HENLEY BRECCIA						Wash drilling									
103	41																41		
102	42																42		
101	43																43		
100	44																44		
99	45																45		
98	46																46		
97	47																47		
96	48																48		
95	49																49		
94																	50		
Notes and Comments: End of Hole @ 60.60m, Target Depth. 0.0 - 10.6 m PQTT coring 10.6 - 60.6 m wash drilling Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical      Orientation:				Ground Water Level											
				Contractor: Speight Drilling Equipment: Track mounted rig Shear Vane Id:				<table border="1"> <tr> <th>Date</th> <th>Time</th> <th>Reading (mbgl)</th> <th>Hole depth (mbgl)</th> </tr> <tr> <td></td> <td></td> <td></td> <td></td> </tr> </table>				Date	Time	Reading (mbgl)	Hole depth (mbgl)				
Date	Time	Reading (mbgl)	Hole depth (mbgl)																

			Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Southern Boundary Job Number: 12506381 Commenced: 2/11/2019 Completed: 4/11/2019						Hole No. : BH202 Sheet : 6 of 7 Hole Length : 60.60m Scale @ A4 : 1:50 Logged : MF Processed : MF Checked : JHS											
Easting: 396181 RL: 144			Northing: 787498 Datum: NZVD2016			System: TAIETM2000														
RL (m)	Depth (m)	Graphic	Material Description				Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQR (%)	Defect Spacing (mm)	Instrumentation Installation	Water level
			10.60 m to 60.60 m: Wash drilled (continued from layer starting at 10.6m )				HENLEY BRECCIA						Wash drilling							
83	51																			50
																				51
92	52																			52
																				53
91	53																			54
																				55
90	54																			56
																				57
89	55																			58
																				59
88	56																			60
87	57																			
86	58																			
85	59																			
84	60																			

Notes and Comments: End of Hole @ 60.60m, Target Depth.  0.0 - 10.6 m PQTT coring 10.6 - 60.6 m wash drilling  Refer to explanation sheets for abbreviation and symbols		Inclination: Vertical		Orientation:		Ground Water Level			
		Contractor: Speight Drilling		Equipment: Track mounted rig		Date		Time	
		Shear Vane Id:				Reading (mbgl)		Hole depth (mbgl)	



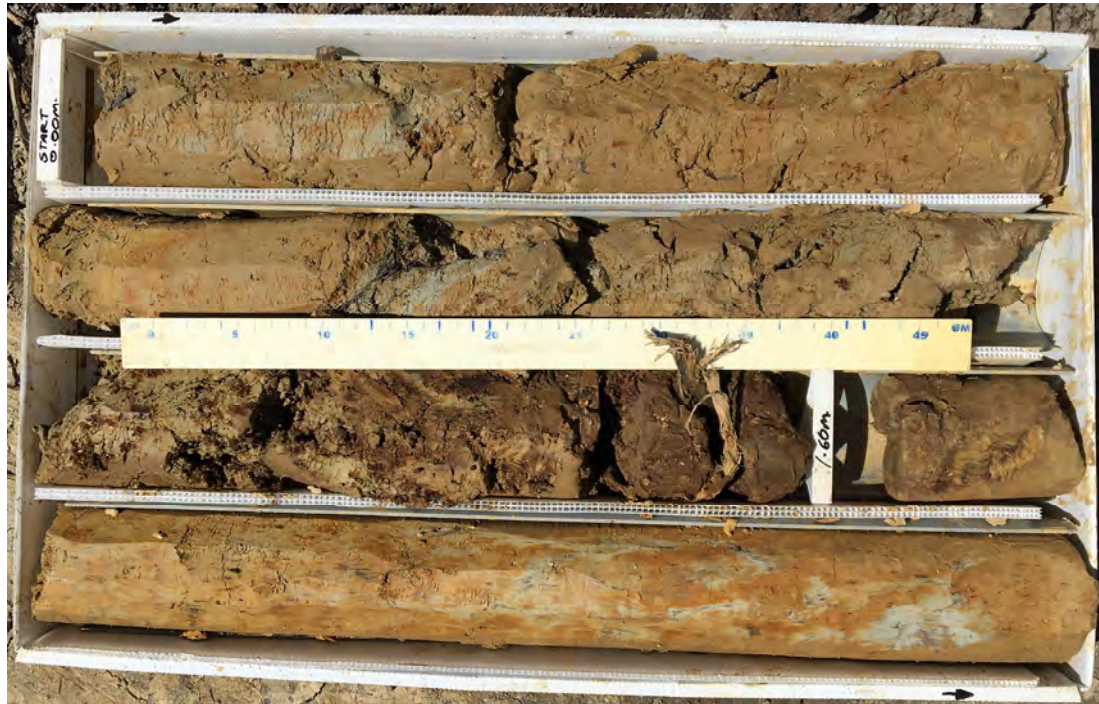
		Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Southern Boundary Job Number: 12506381 Commenced: 2/11/2019 Completed: 4/11/2019						Hole No. : BH202 Sheet : 7 of 7 Hole Length : 60.60m Scale @ A4 : 1:50 Logged : MF Processed : MF Checked : JHS									
Easting: 396181 RL: 144		Northing: 787498 Datum: NZVD2016		System: TAIETM2000													
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample Number / Type Result		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect Spacing (mm)	Instrumentation Installation	Water level
			10.60 m to 60.60 m: Wash drilled (continued from layer starting at 10.6m )														
			End of Hole @ 60.60m,Target Depth.														
61																	
62																	
63																	
64																	
65																	
66																	
67																	
68																	
69																	
70																	

Notes and Comments: End of Hole @ 60.60m, Target Depth.  0.0 - 10.6 m PQTT coring 10.6 - 60.6 m wash drilling  Refer to explanation sheets for abbreviation and symbols		Inclination: Vertical Orientation:  Contractor: Speight Drilling Equipment: Track mounted rig Shear Vane Id:		Ground Water Level  Date Time Reading (mbgl) Hole depth (mbgl)	
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## Report of Photographs

Site Identification: BH202

Project	Waste Futures WS3 – Smooth Hill	Commenced	02/11/2019	Completed	04/11/2019
Site	Southern Boundary	Logged By	MF		
Job #	12506381	Checked By			
Client	Dunedin City Council	Core Depth	0.0 m – 10.6 m		



Box 1 of 4: 0.00 m to 2.30 m



Box 2 of 4: 2.30 m to 6.10 m



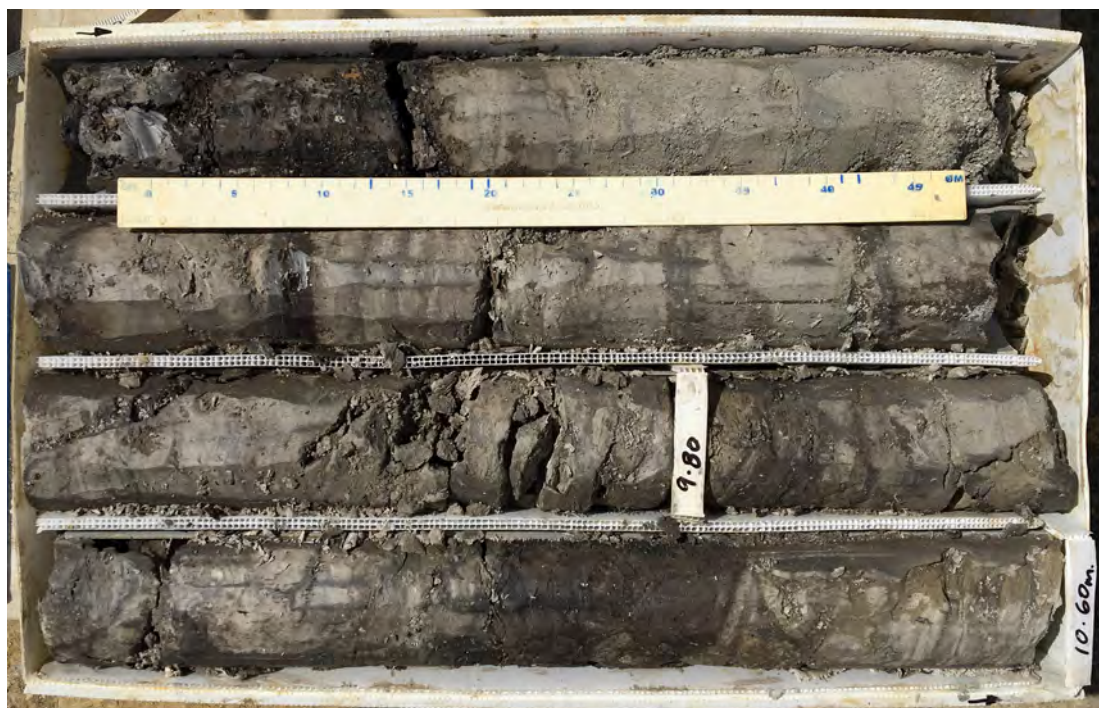
## Report of Photographs

Site Identification: BH202

Project	Waste Futures WS3 – Smooth Hill	Commenced	02/11/2019	Completed	04/11/2019
Site	Southern Boundary	Logged By	MF		
Job #	12506381	Checked By			
Client	Dunedin City Council	Core Depth	0.0 m – 10.6 m		



Box 3 of 4: 6.10 m to 8.30 m



Box 4 of 4: 8.30 m to 10.60 m



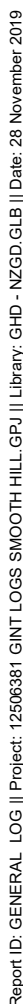
## Report of Photographs


Site Identification: BH202

<b>Project</b>	Waste Futures WS3 – Smooth Hill	<b>Commenced</b>	02/11/2019	<b>Completed</b>	04/11/2019
<b>Site</b>	Southern Boundary	<b>Logged By</b>	MF		
<b>Job #</b>	12506381	<b>Checked By</b>			
<b>Client</b>	Dunedin City Council	<b>Core Depth</b>	0.0 m – 10.6 m		

10.60 m to 60.60 m (EOH) – Wash drilled, no core recovered





<div></div> <div>Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Southwest Boundary Job Number: 12506381 Commenced: 7/11/2019 Completed: 7/11/2019</div>										<div>Hole No. : BH203 Sheet : 2 of 2 Hole Length : 19.70m Scale @ A4 : 1:50</div> <div>Logged : MF Processed : MF Checked : JHS</div>																			
Easting: 395779 RL: 182					Northing: 787672 Datum: NZVD2009					System: TAIETM2000																			
Material Description										Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect Spacing (mm)	Instrumentation Installation	Water level						
													Number / Type	Result															
RL (m)	Depth (m)	Graphic																											
171	11	XXXXXX	Moderately weathered, orange-brown, very thinly to moderately thickly bedded SILTSTONE; very weak; no defects; moderately widely spaced lignite layers 5-20 mm thick ( <i>continued from layer starting at 7.5m</i> ) 10.00 - 10.10 m: fine SANDSTONE																										
170	12	XXXXXX	Unweathered, light grey, laminated to moderately thickly bedded, fine grained SANDSTONE; very weak; no defects; moderately widely spaced lignite layers 5-30 mm thick																										
169	13	XXXXXX	HENLEY BRECCIA																										
168	14	XXXXXX																											
167	15	△△△△	14.45 - 14.55 m: grades into BRECCIA Unweathered, light grey, BRECCIA; weak to moderately strong; no defects; clast supported; matrix is fine to coarse sand; clasts are fine, quartz and schist, angular to subangular gravel																										
166	16	△△△△	Unweathered, light grey, laminated to moderately thickly bedded, fine grained SANDSTONE; very weak to weak; no defects 16.00 - 16.15 m: moderately strong																										
165	17	△△△△	17.30 - 17.60 m: grades into BRECCIA																										
164	18	△△△△	Slightly weathered, light grey and white, BRECCIA; moderately strong to strong; no defects; clast supported; matrix is fine to coarse sand; clasts are fine to coarse, angular to subrounded, quartz and schist gravel																										
163	19	△△△△																											
162			End of Hole @ 19.70m, Target Depth.																										
Notes and Comments: End of Hole @ 19.70m, Target Depth. Groundwater not encountered  Refer to explanation sheets for abbreviation and symbols										Inclination: Vertical					Orientation:					Ground Water Level									
										Contractor: Speight Drilling										Date					Time				
										Equipment: Track mounted rig										Reading (mbgl)					Hole depth (mbgl)				
										Shear Vane Id:																			



## Report of Photographs

### Site Identification: BH203

Project	Waste Futures WS3 – Smooth Hill	Commenced	07/11/2019	Completed	07/11/2019
Site	Smooth Hill	Logged By	MF		
Job #	12506381	Checked By			
Client	Dunedin City Council	Core Depth	0.0 m – 19.7 m		



0.00 m to 2.30 m



2.30 m to 4.60 m



## Report of Photographs

Site Identification: BH203

Project	Waste Futures WS3 – Smooth Hill	Commenced	07/11/2019	Completed	07/11/2019
Site	Smooth Hill	Logged By	MF		
Job #	12506381	Checked By			
Client	Dunedin City Council	Core Depth	0.0 m to 19.7 m		



4.60 m to 6.90 m



6.90 m to 9.20 m



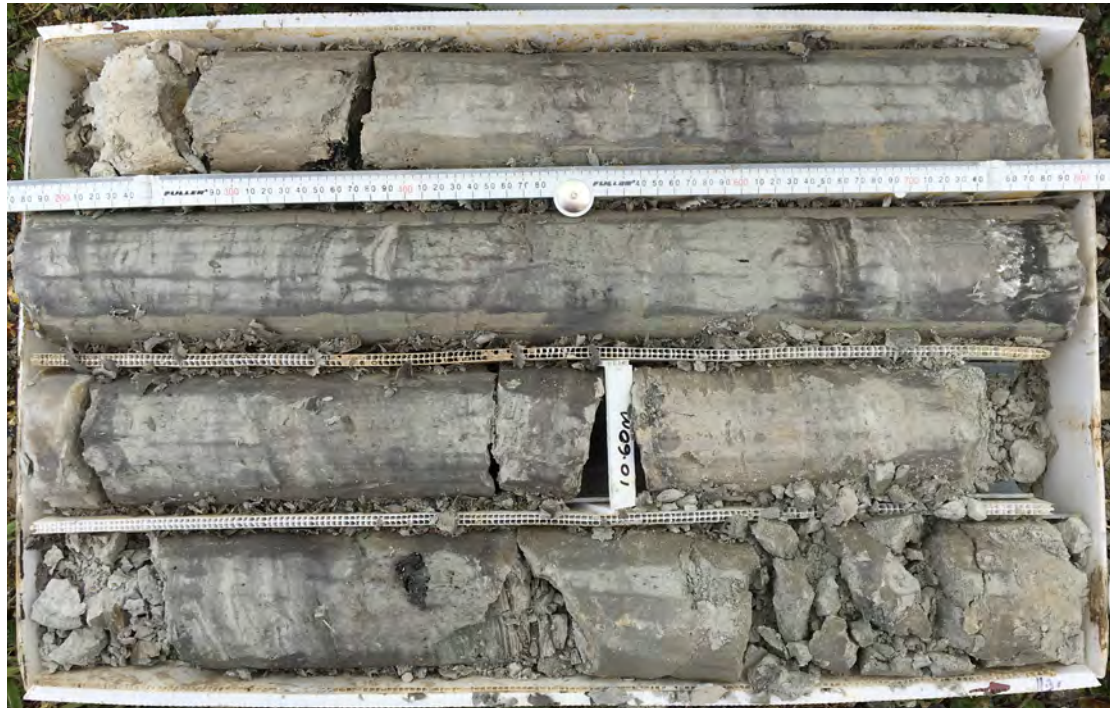


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## Report of Photographs

Site Identification: BH203

Project	Waste Futures WS3 – Smooth Hill	Commenced	07/11/2019	Completed	07/11/2019
Site	Smooth Hill	Logged By	MF		
Job #	12506381	Checked By			
Client	Dunedin City Council	Core Depth	0.0 m to 19.7 m		



9.20 m to 11.30 m



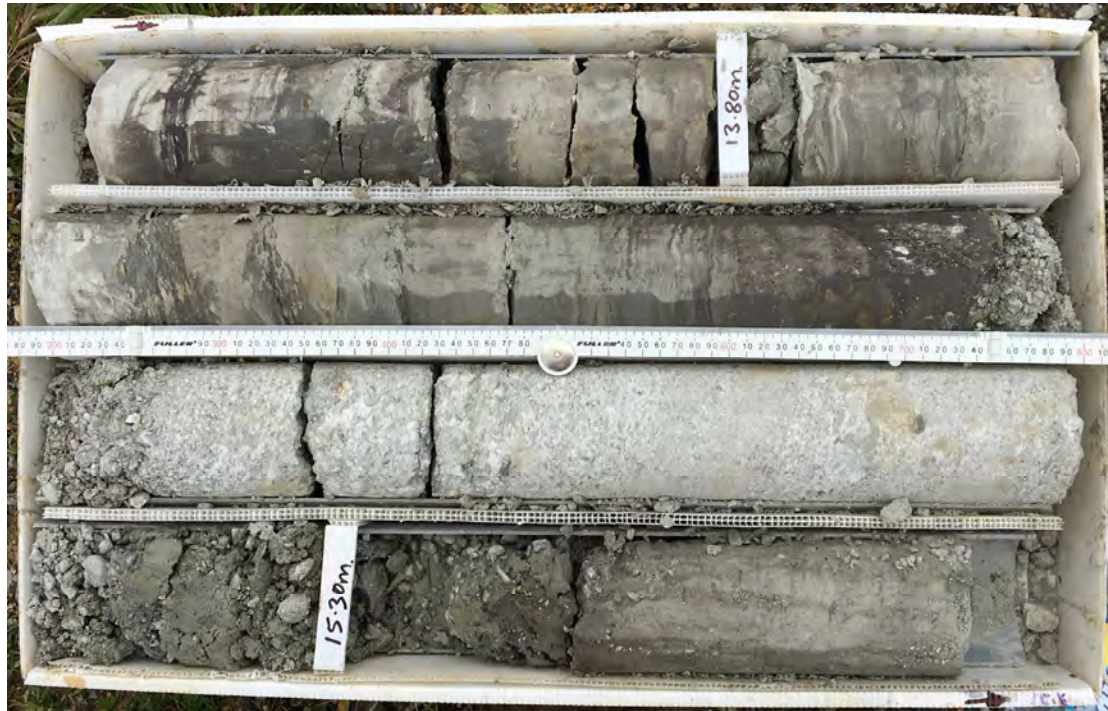
11.30 m to 13.40 m



## Report of Photographs

### Site Identification: BH203

<b>Project</b>	Waste Futures WS3 – Smooth Hill	<b>Commenced</b>	07/11/2019	<b>Completed</b>	07/11/2019
<b>Site</b>	Smooth Hill	<b>Logged By</b>	MF		
<b>Job #</b>	12506381	<b>Checked By</b>			
<b>Client</b>	Dunedin City Council	<b>Core Depth</b>	0.0 m to 19.7 m		



13.40 m to 15.60 m



15.60 m to 17.90 m






## Report of Photographs

Site Identification: BH203

Project	Waste Futures WS3 – Smooth Hill	Commenced	07/11/2019	Completed	07/11/2019
Site	Smooth Hill	Logged By	MF		
Job #	12506381	Checked By			
Client	Dunedin City Council	Core Depth	0.0 m to 19.7 m		



17.90 m to 19.70 m (EOH)

		Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Western Boundary Job Number: 12506381 Commenced: 24/10/2019      Completed: 24/10/2019						Hole No. : BH209 Sheet : 1 of 1 Hole Length : 10.00m Scale @ A4 : 1:50									
		Easting:      Northing:      System: RL:      Datum:						Logged : MF Processed : MF Checked : JHS									
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect Spacing (mm)	Instrumentation Installation	Water level
							Number / Type	Result									
	0		Intermixed: clayey SILT, sandy SILT, and SILT with minor clay; grey and brown. Soft to firm, moist to wet, low to high plasticity; wood fragments throughout (FILL)	FILL	M-W	S-F											
	0.80		SILT, minor clay, trace fine sand; light grey and orange-brown. Very stiff, moist, low plasticity; trace fine gravel sized iron oxide nodules (LOESS)	LOESS	M	VSt				PQTT				92			
	0.80 - 3.15 m		grey-brown and orange-brown								PQTT				100		
	3.15		Gravelly SILT; grey, cream and brown. Very stiff to hard, dry to moist, non-plastic; gravel is fine to medium, rounded to subangular quartz and schist; highly weathered (TARATU FORMATION)		D-M	VSt-H					PQTT				96		
	3.6		SILT, minor to some clay; grey with orange-brown streaks. Firm to stiff, moist, high plasticity	TARATU FORMATION	M	F-St						HW					
	4		Moderately weathered, orange-brown, very thinly to moderately thickly bedded fine-grained SANDSTONE; very weak; no defects								PQTT				100 86		
	4.80 - 5.30 m		light grey								PQTT						
	5.3		Moderately weathered, brown and orange-brown CONGLOMERATE; extremely weak; no defects; matrix supported; clasts are fine to medium, rounded to subangular, quartz and schist gravel; silty sand matrix. Soil description: gravelly silty sand	TARATU FORMATION								MW		100 60			
	5.9		Moderately weathered, orange-brown, very thinly to moderately thickly bedded fine-grained SANDSTONE; very weak; no defects								PQTT						
	6.65		Slightly weathered, grey, laminated to moderately thin bedded fine-grained SANDSTONE; very weak; no defects								PQTT				100 100		
	7.3		Slightly weathered, brown with occasional orange-brown and white, moderately thin to moderately thickly bedded, fine to medium-grained SANDSTONE; very weak; no defects; minor fine quartz and schist gravel								PQTT						
	7.8		Slightly weathered, brown and orange-brown CONGLOMERATE; extremely weak; no defects; clast supported; silty sand matrix; clasts are fine to coarse, rounded to subangular, quartz and schist gravel. Soil description: silty, sandy, fine to coarse gravel	TARATU FORMATION								SW		100 0			
											PQTT				94 0		
	10		End of Hole @ 10.00m, Target Depth.														

Notes and Comments: End of Hole @ 10.00m, Target Depth. Groundwater not encountered  Refer to explanation sheets for abbreviation and symbols		Inclination: Vertical		Orientation:		Ground Water Level			
		Contractor: Speight Drilling				Date	Time	Reading (mbgl)	Hole depth (mbgl)
		Equipment: Tracked Rig							
		Shear Vane Id:							



## Report of Photographs

### Site Identification: BH209

Project	Waste Futures WS3 – Smooth Hill	Commenced	24/10/2019	Completed	24/10/2019
Site	Western Boundary	Logged By	MF		
Job #	12506381	Checked By			
Client	Dunedin City Council	Core Depth	0.0 m – 10.0 m		



Box 1 of 5: 0.00 m to 2.40 m



Box 2 of 5: 2.40 m to 4.70 m



## Report of Photographs

Site Identification: BH209

Project	Waste Futures WS3 – Smooth Hill	Commenced	24/10/2019	Completed	24/10/2019
Site	Southern Boundary	Logged By	MF		
Job #	12506381	Checked By			
Client	Dunedin City Council	Core Depth	0.0 m – 10.0 m		



Box 3 of 5: 4.70 m to 7.15 m



Box 4 of 5: 7.15 m to 9.20 m

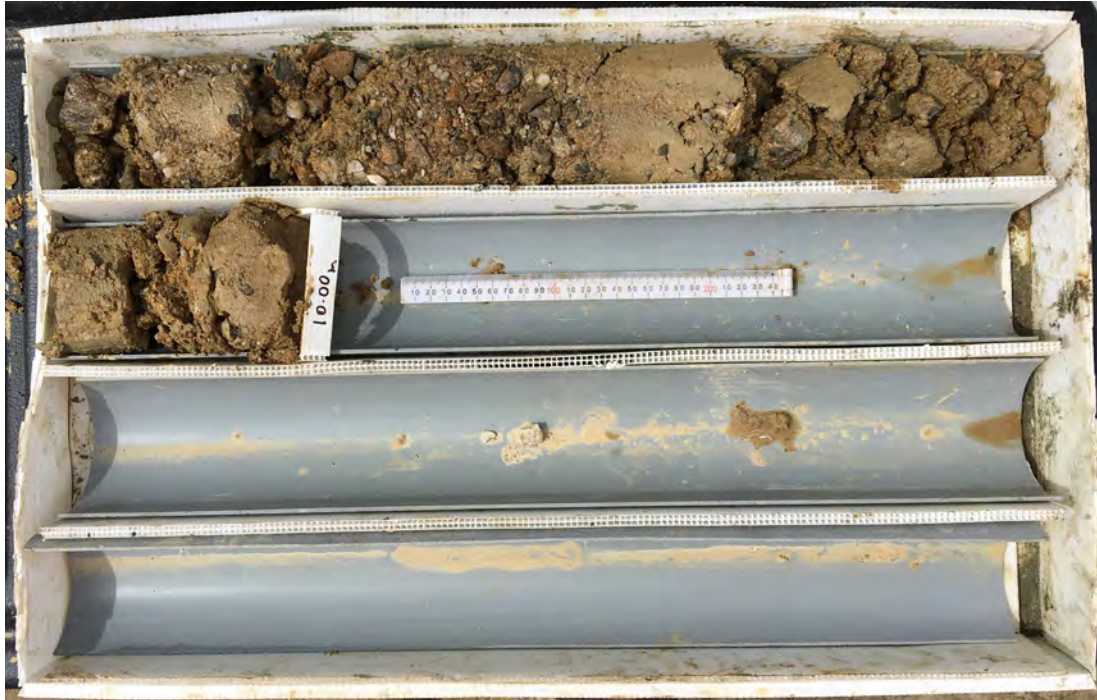





## Report of Photographs

Site Identification: BH209

Project	Waste Futures WS3 – Smooth Hill	Commenced	24/10/2019	Completed	24/10/2019
Site	Southern Boundary	Logged By	MF		
Job #	12506381	Checked By			
Client	Dunedin City Council	Core Depth	0.0 m – 10.0 m		



Box 5 of 5: 9.20 m to 10.0 m (EOH)

			Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Eastern gully base Job Number: 12506381 Commenced: 4/11/2019 Completed: 6/11/2019						Hole No. : BH211A Sheet : 1 of 3 Hole Length : 25.20m Scale @ A4 : 1:50 Logged : MF Processed : MF Checked : JHS									
Easting: 396598 RL: 107			Northing: 787965 Datum: NZVD2009			System: TAIETM2000												
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Sample		Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect Spacing (mm)	Instrumentation Installation	Water level	
							Number / Type	Result										
	0		0.00 - 0.35 m: CORE LOSS (inferred at top of run)	FILL	M	St-VSt												
	0.35		SILT, minor fine to medium sand, trace clay; grey, orange-brown and dark grey intermixed. Stiff to very stiff, moist, low plasticity (FILL)	LOESS	M	VSt				PQTT				78				
	0.7		SILT, trace to minor fine to medium sand; grey and orange-brown. Very stiff, moist, low plasticity; trace to minor iron-oxide nodules (LOESS)															
	1		1.60 - 1.80 m: CORE LOSS (inferred depth)															
	1.6		SILT (continued from 0.7 m)															
	1.95		Sandy SILT, minor fine gravel; orange-brown and grey. Very stiff to hard, moist, non-plastic; sand is fine to medium; gravel is angular to rounded quartz and schist; completely weathered breccia (HENLEY BRECCIA)		M	VSt-H				PQTT				75				
	2		2.40 - 2.80 m: firm to stiff			F-St												
	2.4		2.80 - 3.20 m: very stiff			VSt					PQTT				62			
	3		3.20 - 3.70 m: CORE LOSS															
	3.2		SILT; dark grey. Firm to stiff, moist, low plasticity; completely weathered siltstone		M	F-St												
	3.7		Gravelly SAND; grey. Moist; sand is fine to coarse; gravel is fine, angular to subrounded, quartz and schist; completely weathered breccia		M	VSt-H								100				
	4		4.60 - 5.20 m: CORE LOSS															
	4.2		Gravelly SAND (continued from 4.2 m)															
	4.6		Highly weathered, orange-brown and grey, moderately thickly bedded BRECCIA; extremely weak; no defects; matrix supported; matrix is fine to coarse sand; clasts are fine to medium, angular to rounded, quartz and schist gravel. Soil description: gravelly sand	HENLEY BRECCIA										59				
	5		Slightly weathered, light grey and white, moderately thickly bedded SILTSTONE; very weak to weak; no defects			VSt-H					PQTT		HW		25			
	5.2		Slightly weathered, light grey and white BRECCIA; weak to moderately strong; no defects; clast supported; matrix is fine to coarse sand; clasts are fine to coarse, angular to subrounded, quartz and schist gravel								PQTT				100			
	5.4		8.80 - 9.90 m: very weak to weak															
	5.6																	
	5.8																	
	6																	
	6.2																	
	6.4																	
	6.6																	
	6.8																	
	7																	
	7.2																	
	7.4																	
	7.6																	
	7.8																	
	8																	
	8.2																	
	8.4																	
	8.6																	
	8.8																	
	9																	
	9.2																	
	9.4																	
	9.6																	
	9.8																	
	10																	
Notes and Comments: End of Hole @ 25.20m, Target Depth. Groundwater at 2.81 m bgl in shallow piezo (25/11/19) Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical		Orientation:		Ground Water Level										
				Contractor: Speight Drilling				Date	Time	Reading (mbgl)	Hole depth (mbgl)							
				Equipment: Track mounted rig														
				Shear Vane Id:														


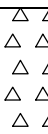
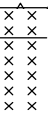
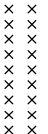

















<div></div> <div>Project : Smooth Hill Landfill Consenting Client : Dunedin City Council Site : Eastern gully base Job Number: 12506381 Commenced: 4/11/2019</div>										<div>Hole No. : BH211B</div> <div>Sheet : 2 of 3 Hole Length : 25.20m Scale @ A4 : 1:50</div> <div>Logged : MF Processed : MF Checked : JHS</div>									
Easting: 396598					Northing: 787965					System: TAIETM2000									
RL: 107					Datum: NZVD2009														
RL (m)	Depth (m)	Graphic	Material Description	Geological Unit	Moisture condition	Consistency / Relative density	Number / Type	Result	Casing	Method	Flush Return (%)	Weathering	Estimated Strength (MPa)	TCR SCR RQD (%)	Defect Spacing (mm)	Instrumentation Installation	Water level		
11.8	9.90 - 11.80		9.90 - 11.80 m: weak to moderately strong Slightly weathered, light grey and white BRECCIA; weak to moderately strong; no defects; clast supported; matrix is fine to coarse sand; clasts are fine to coarse, angular to subrounded, quartz and schist gravel (continued from layer starting at 6.8m )	HENLEY BRECCIA						PQTT				100 92					
12.1	11.80 - 12.10		Slightly weathered, grey SILTSTONE; very weak; no defects; trace to minor fine gravel, content decreases with depth Slightly weathered, brown, SILTSTONE; very weak; no defects; possible lithified topsoil							PQTT				97 90					
13.0	12.10 - 13.00									PQTT				100 87					
13.6	13.00 - 13.65		13.50 - 13.65 m: grey Slightly weathered, grey, fine to coarse grained SANDSTONE; very weak to weak; no defects							PQTT				100 46					
14.1	13.65 - 14.10		Slightly weathered, grey and white BRECCIA; weak to moderately strong; no defects; matrix supported; matrix is fine to coarse sand; clasts are fine to coarse, angular to subrounded, quartz and schist gravel							PQTT				100 35					
16.1	14.10 - 16.10		Slightly weathered, grey, fine to coarse grained SANDSTONE; very weak; no defects							PQTT				100 84					
17.6	16.10 - 17.60		Slightly weathered, grey and white BRECCIA; very weak; no defects; matrix supported; matrix is fine to coarse sand; clasts are fine to medium, angular to subrounded, quartz and schist gravel							PQTT				100 86					
18.3	17.60 - 18.30		Slightly weathered, grey, fine to medium grained SANDSTONE; very weak; no defects							PQTT				100 71					
19.1	18.30 - 19.10		Slightly weathered, grey and white BRECCIA; moderately strong to strong; no defects; clast supported; fine to coarse sand matrix; clasts are fine to medium, angular to subrounded, quartz and schist gravel								PQTT								
			Slightly weathered, grey and white BRECCIA; very weak to weak; no defects; matrix supported; matrix is fine to coarse sand; clasts are fine to coarse, angular to subrounded, quartz and schist gravel								PQTT								
Notes and Comments: End of Hole @ 25.20m, Target Depth. Groundwater at 13.0 m bgl in deep piezo (25/11/19) Refer to explanation sheets for abbreviation and symbols				Inclination: Vertical				Orientation:				Ground Water Level							
				Contractor: Speight Drilling								Date	Time	Reading (mbgl)	Hole depth (mbgl)				
				Equipment: Track mounted rig															
				Shear Vane Id:															

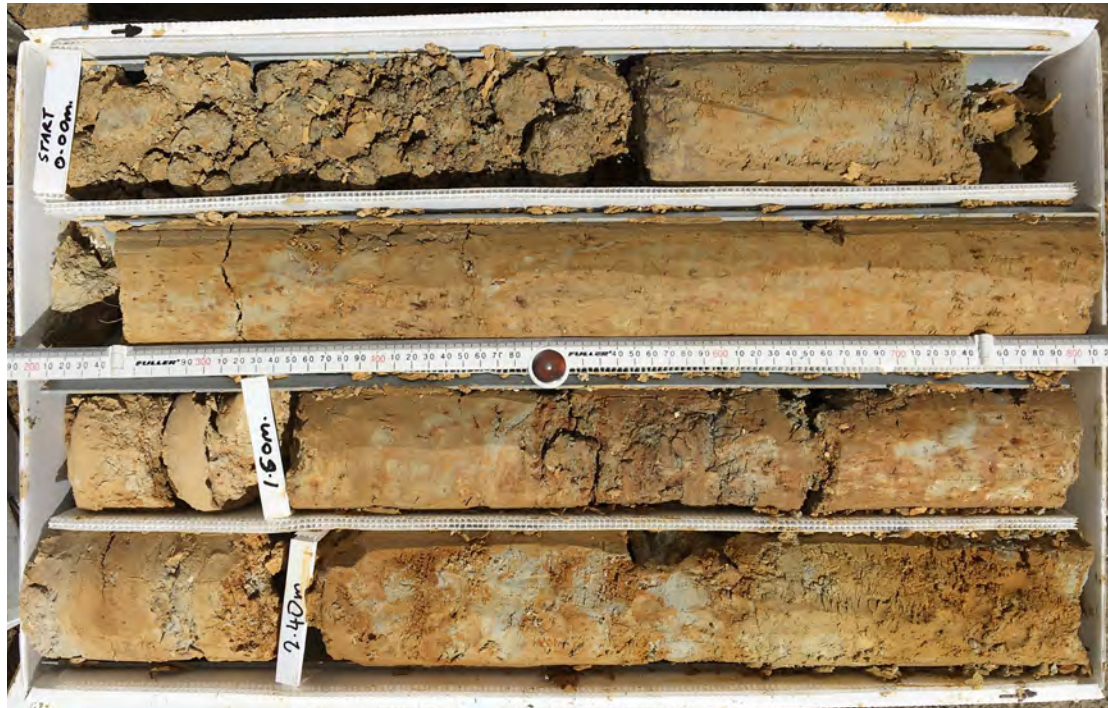




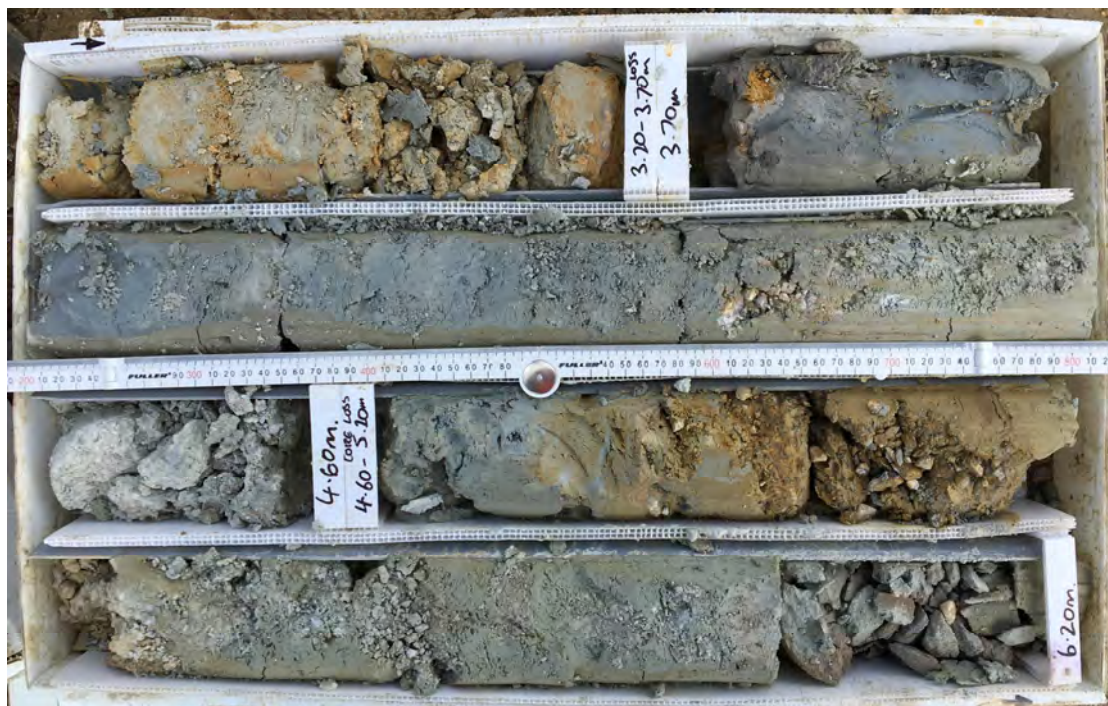
## Report of Photographs

### Site Identification: BH211

Project	Waste Futures WS3 – Smooth Hill	Commenced	04/11/2019	Completed	06/11/2019
Site	Smooth Hill	Logged By	MF		
Job #	12506381	Checked By			
Client	Dunedin City Council	Core Depth	0.0 m – 25.2 m		



0.00 m to 2.80 m



2.80 m to 6.20 m



## Report of Photographs

### Site Identification: BH211

<b>Project</b>	Waste Futures WS3 – Smooth Hill	<b>Commenced</b>	04/11/2019	<b>Completed</b>	06/11/2019
<b>Site</b>	Smooth Hill	<b>Logged By</b>	MF		
<b>Job #</b>	12506381	<b>Checked By</b>			
<b>Client</b>	Dunedin City Council	<b>Core Depth</b>	0.0 m to 25.2 m		



6.20 m to 8.50 m



8.50 m to 10.70 m



## Report of Photographs

Site Identification: BH211

Project	Waste Futures WS3 – Smooth Hill	Commenced	04/11/2019	Completed	06/11/2019
Site	Smooth Hill	Logged By	MF		
Job #	12506381	Checked By			
Client	Dunedin City Council	Core Depth	0.0 m to 25.2 m		



10.70 m to 12.80 m



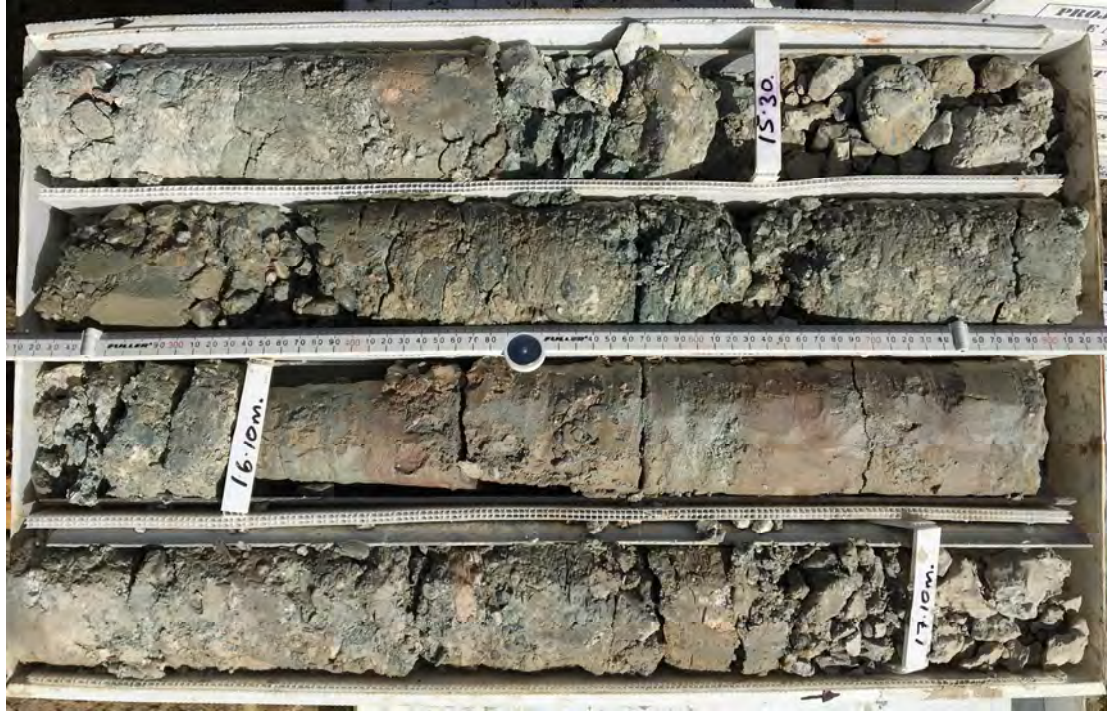
12.80 m to 14.90 m



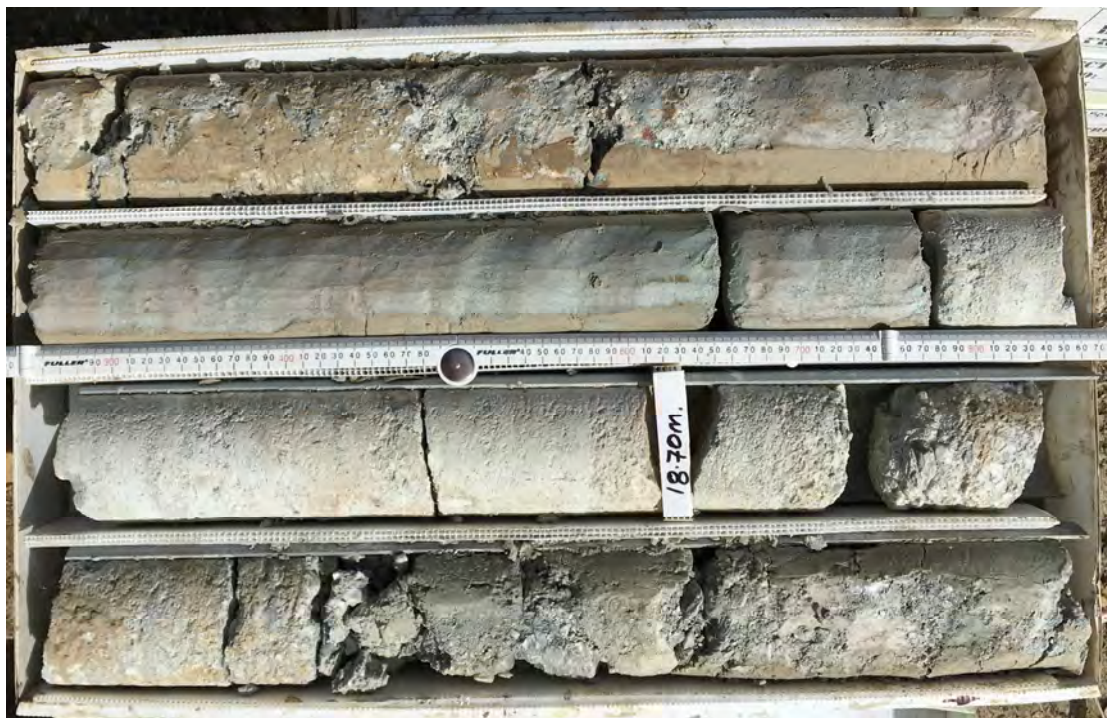
## Report of Photographs

**Site Identification: BH211**

<b>Project</b>	Waste Futures WS3 – Smooth Hill	<b>Commenced</b>	04/11/2019	<b>Completed</b>	06/11/2019
<b>Site</b>	Smooth Hill	<b>Logged By</b>	MF		
<b>Job #</b>	12506381	<b>Checked By</b>			
<b>Client</b>	Dunedin City Council	<b>Core Depth</b>	0.0 m to 25.2 m		



14.90 m to 17.20 m



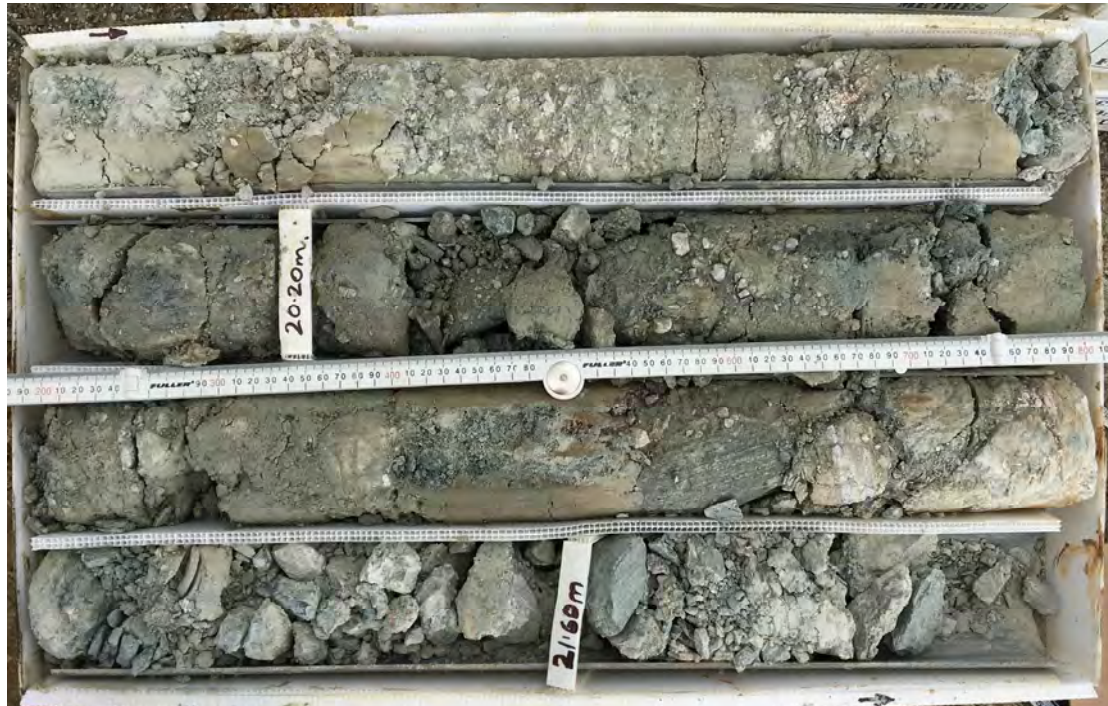
17.20 m to 19.50 m



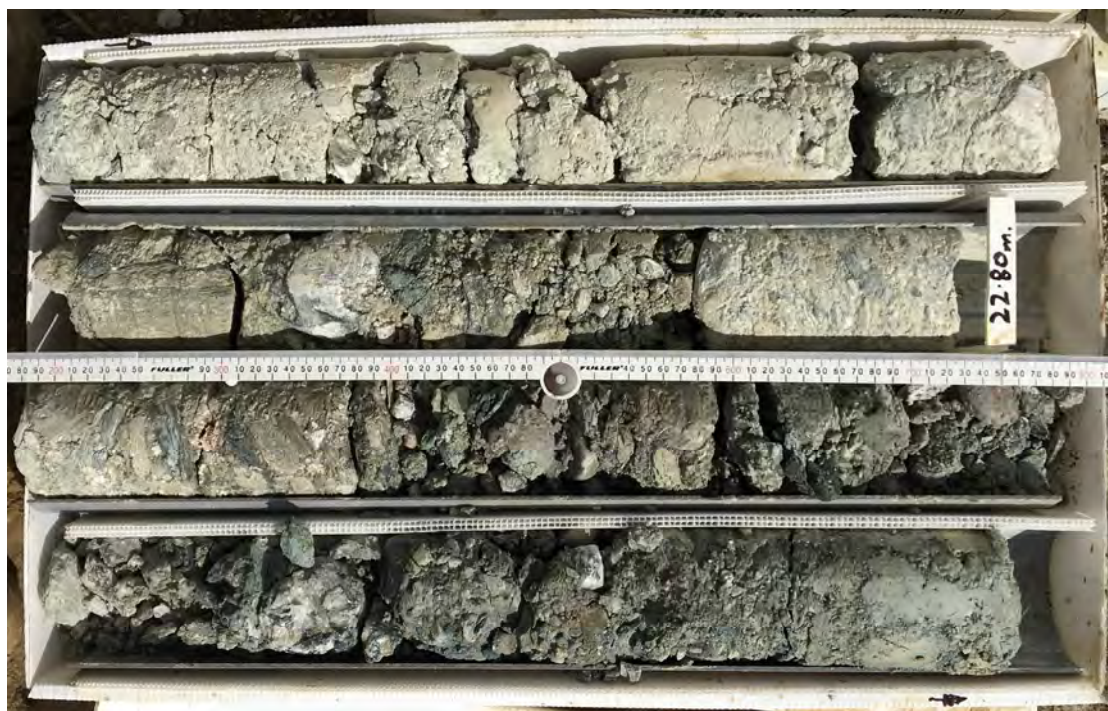
## Report of Photographs

Site Identification: BH211

<b>Project</b>	Waste Futures WS3 – Smooth Hill	<b>Commenced</b>	04/11/2019	<b>Completed</b>	06/11/2019
<b>Site</b>	Smooth Hill	<b>Logged By</b>	MF		
<b>Job #</b>	12506381	<b>Checked By</b>			
<b>Client</b>	Dunedin City Council	<b>Core Depth</b>	0.0 m to 25.2 m		



19.50 m to 21.70 m



21.70 m to 23.90 m

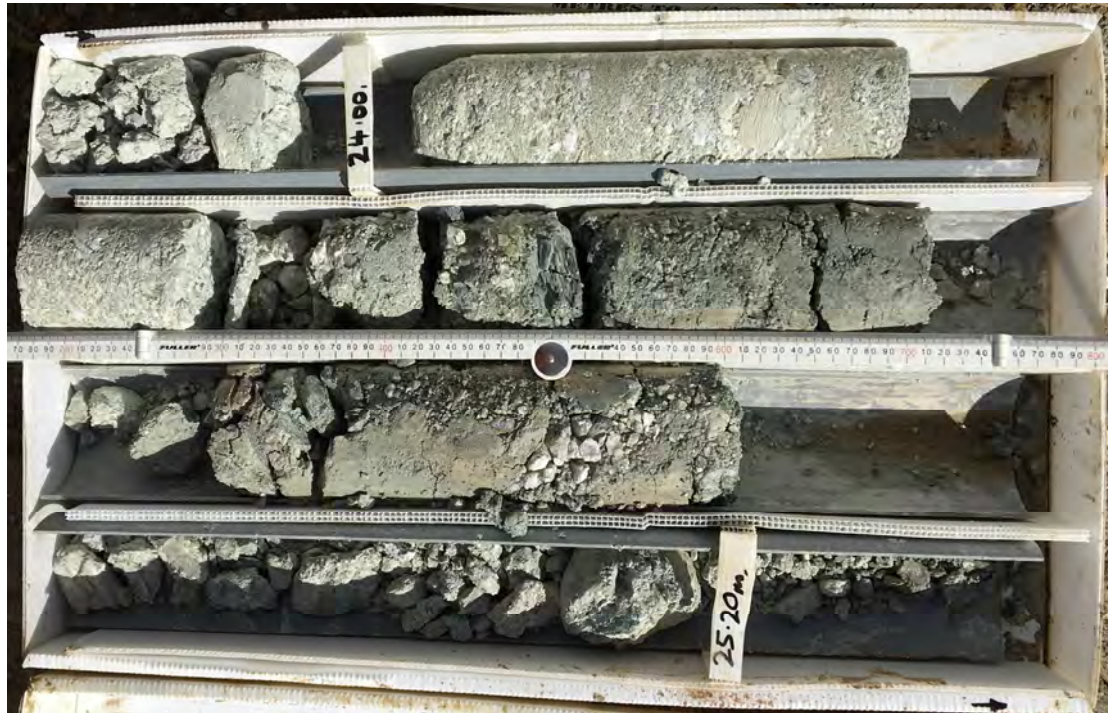




## Report of Photographs

Site Identification: BH211

Project	Waste Futures WS3 – Smooth Hill	Commenced	04/11/2019	Completed	06/11/2019
Site	Smooth Hill	Logged By	MF		
Job #	12506381	Checked By			
Client	Dunedin City Council	Core Depth	0.0 m to 25.2 m		



23.90 m to 25.20 m (EOH)

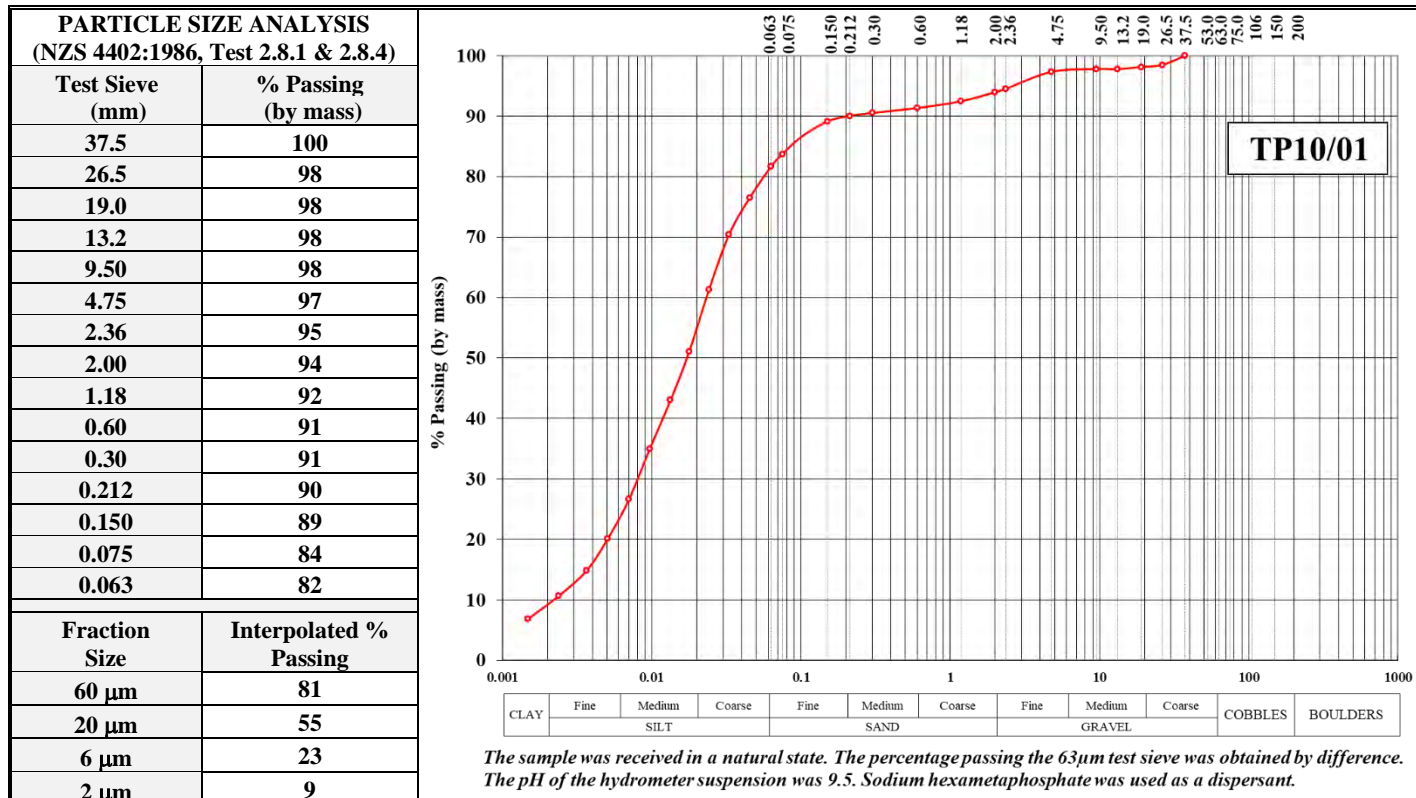
## Appendix C – Laboratory Testing Results





## TEST REPORT: DCC SMOOTH HILL INVESTIGATIONS

Client Details:	GHD, Level 3, 138 Victoria Street, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Loess – SILT with some sand, minor gravel and minor clay	Client Job No:	12506381
Sample Source:	TP10/01	Sample Depth:	2.2m to 3.6m
Date & Time Sampled:	10-Jun-19	Sampled By:	M. Fitzmaurice
Sample Method:	Test Pit / Borehole *	Date Received:	26-Jun-19



PARTICLE SIZE ANALYSIS & HYDROMETER ANALYSIS RESULTS - NZS 4402:1986, Test 2.8.1 & 2.8.4					
Description	Fraction Range	% Within Range	Description	Fraction Range	% Within Range
Coarse Gravel	60.0mm to 20.0mm	2	Fine Sand	200 µm to 60 µm	9
Medium Gravel	20.0mm to 6.0mm	1	Coarse Silt	60 µm to 20 µm	26
Fine Gravel	6.0mm to 2.00 mm	3	Medium Silt	20 µm to 6 µm	32
Coarse Sand	2.00mm to 600 µm	3	Fine Silt	6 µm to 2 µm	14
Medium Sand	600 µm to 200 µm	1	Clay	< 2 µm	9

WATER CONTENT & PLASTICITY INDEX RESULTS - NZS 4402:1986, Test 2.1, 2.2, 2.3 & 2.4	
Water Content: ("All In" As Received)	15.5 %
Liquid Limit: (LL)	39
Plastic Limit: (PL)	28
Plasticity Index: (PI)	11
Note: The sample was received in a natural state. The plasticity index material tested was the fraction passing the 425 µm test sieve.	

Note:

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Tested By: L.T. Smith

Date: 4 to 15-Jul-19

Checked By:

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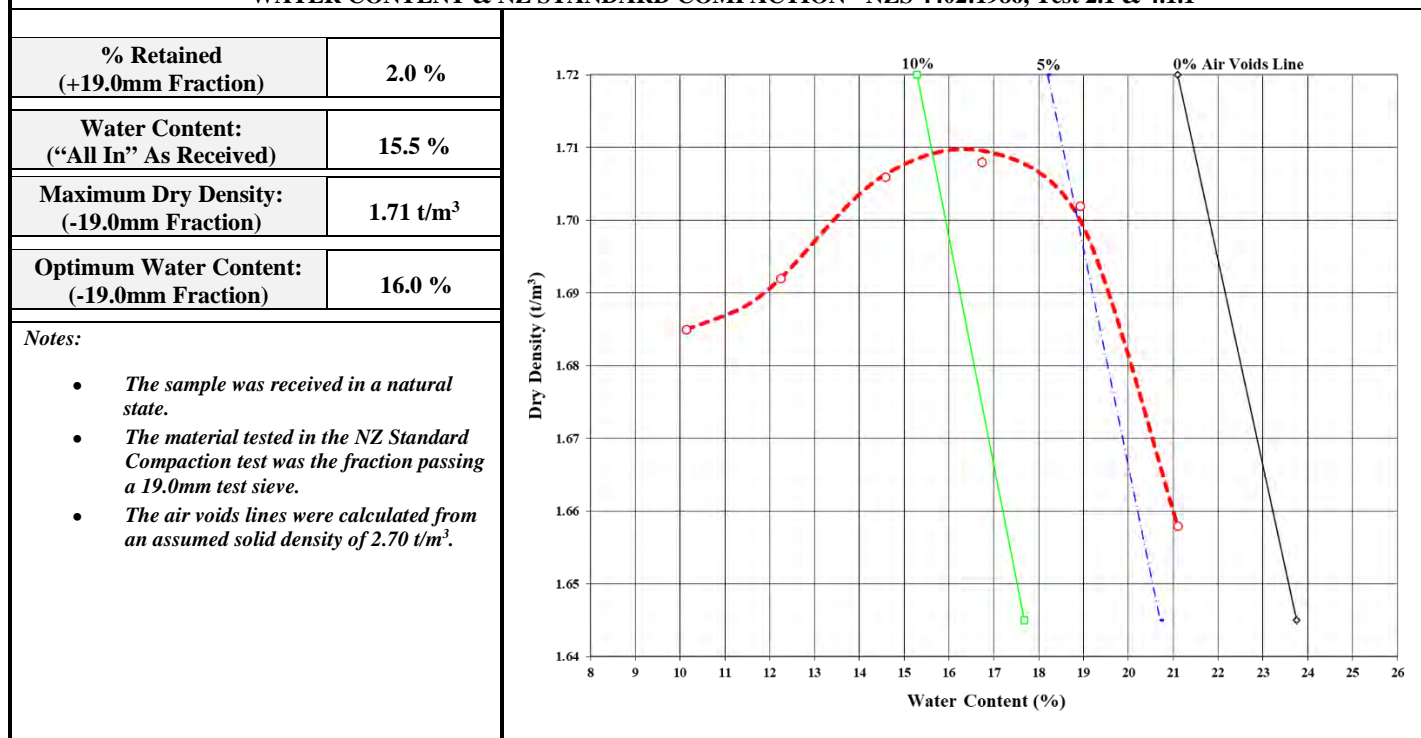
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## TEST REPORT: DCC SMOOTH HILL INVESTIGATIONS

Client Details:	GHD, Level 3, 138 Victoria Street, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Loess – SILT with some sand, minor gravel and minor clay	Client Job No:	12506381
Sample Source:	TP10/01	Sample Depth:	2.2m to 3.6m
Date & Time Sampled:	10-Jun-19	Sampled By:	M. Fitzmaurice
Sample Method:	Test Pit / Borehole *	Date Received:	26-Jun-19

### WATER CONTENT & NZ STANDARD COMPACTION - NZS 4402:1986, Test 2.1 & 4.1.1



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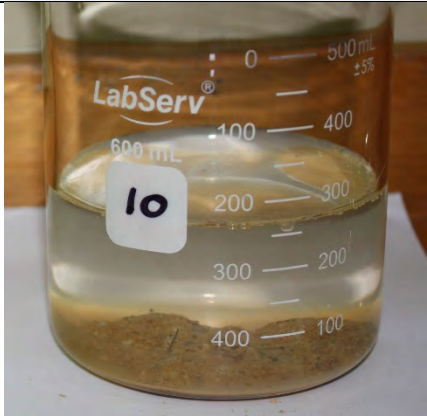




## TEST REPORT: DCC SMOOTH HILL INVESTIGATIONS

Client Details:	GHD, Level 3, 138 Victoria Street, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Loess – SILT with some sand, minor gravel and minor clay	Client Job No:	12506381
Sample Source:	TP10/01	Sample Depth:	2.2m to 3.6m
Date & Time Sampled:	10-Jun-19	Sampled By:	M. Fitzmaurice
Sample Method:	Test Pit / Borehole *	Date Received:	26-Jun-19

PINHOLE DISPERSION TEST: ASTM D4647-13e1			
Head (mm)	Elapsed Time (min)	Flow Rate (ml/s)	Colour of Outflow (Cloudiness)
50	1	0.25	Slightly Dark
50	5	0.27	Moderately Dark
50	10	0.31	Dark
Diameter of Hole at Start of Test:			1.0mm
Diameter of Hole at End of Test:			≈ 2.0mm (4.0mm at exit)
Water Content Prior to Test:			16.2 %
Dry Density of Sample Tested:			1.63 t/m <sup>3</sup>
Pinhole Dispersion Classification – Method B:			Dispersive (D)
CRUMB TEST: ASTM D6572-13e2 (Method B)			
Elapsed Time	Estimated Slaking	Observations Recorded	
2 min	≈ 50 %	No colloidal cloud	
1 hr	≈ 100%	Dense colloidal cloud over	
6 hr	≈ 100 %	Moderate colloidal cloud over	
Crumb Test Classification:		Grade 4 (Highly Dispersive)	
<i>Note:</i>			
<ul style="list-style-type: none"><li>Distilled water was used in the pinhole dispersion and crumb test. Both tests were carried out on remoulded samples.</li><li>The pinhole dispersion sample was compacted to 95% NZ standard compaction.</li><li>Photograph at completion of the crumb test.</li><li>The sample tested was the fraction passing the 2.00mm sieve.</li></ul>			



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Tested By: L.T. Smith

Date: 4 to 15-Jul-19

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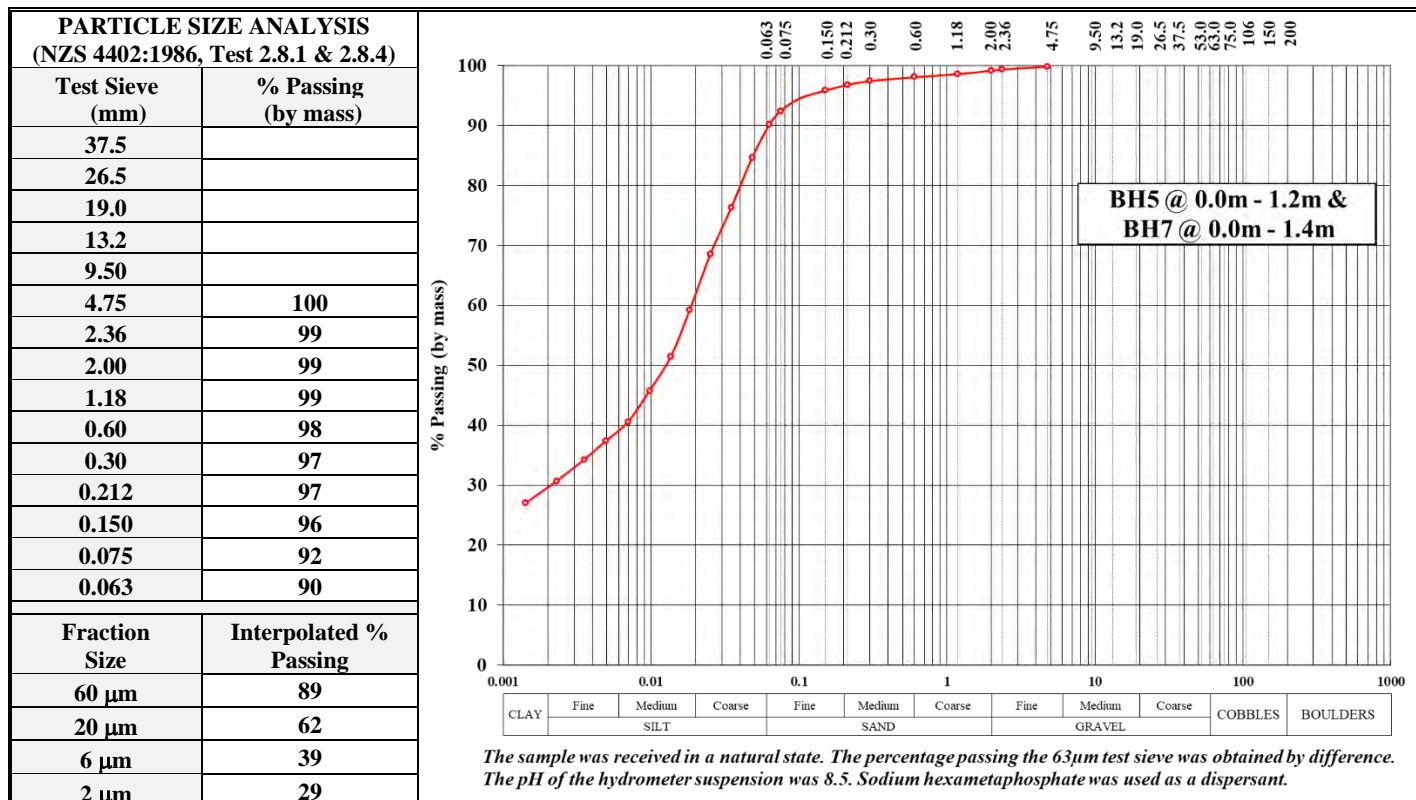
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## TEST REPORT: DCC SMOOTH HILL INVESTIGATIONS

Client Details:	GHD, Level 3, 138 Victoria Street, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Loess – Silty CLAY with minor sand and trace of gravel	Client Job No:	12506381
Sample Source:	BH5 @ 0.0m to 1.2m & BH7 @ 0.0m to 1.4m	Sample Depth:	Combined (0.0m to 1.4m)
Date & Time Sampled:	21-Jun-19	Sampled By:	M. Fitzmaurice
Sample Method:	Test Pit / Borehole *	Date Received:	26-Jun-19



PARTICLE SIZE ANALYSIS & HYDROMETER ANALYSIS RESULTS - NZS 4402:1986, Test 2.8.1 & 2.8.4					
Description	Fraction Range	% Within Range	Description	Fraction Range	% Within Range
Coarse Gravel	60.0mm to 20.0mm	-	Fine Sand	200 µm to 60 µm	8
Medium Gravel	20.0mm to 6.0mm	-	Coarse Silt	60 µm to 20 µm	27
Fine Gravel	6.0mm to 2.00 mm	1	Medium Silt	20 µm to 6 µm	23
Coarse Sand	2.00mm to 600 µm	1	Fine Silt	6 µm to 2 µm	10
Medium Sand	600 µm to 200 µm	1	Clay	< 2 µm	29

WATER CONTENT & PLASTICITY INDEX RESULTS - NZS 4402:1986, Test 2.1, 2.2, 2.3 & 2.4	
Water Content: ("All In" As Received)	23.6 %
Liquid Limit: (LL)	42
Plastic Limit: (PL)	23
Plasticity Index: (PI)	19
Note: The sample was received in a natural state. The plasticity index material tested was the fraction passing the 425 µm test sieve.	

Note:

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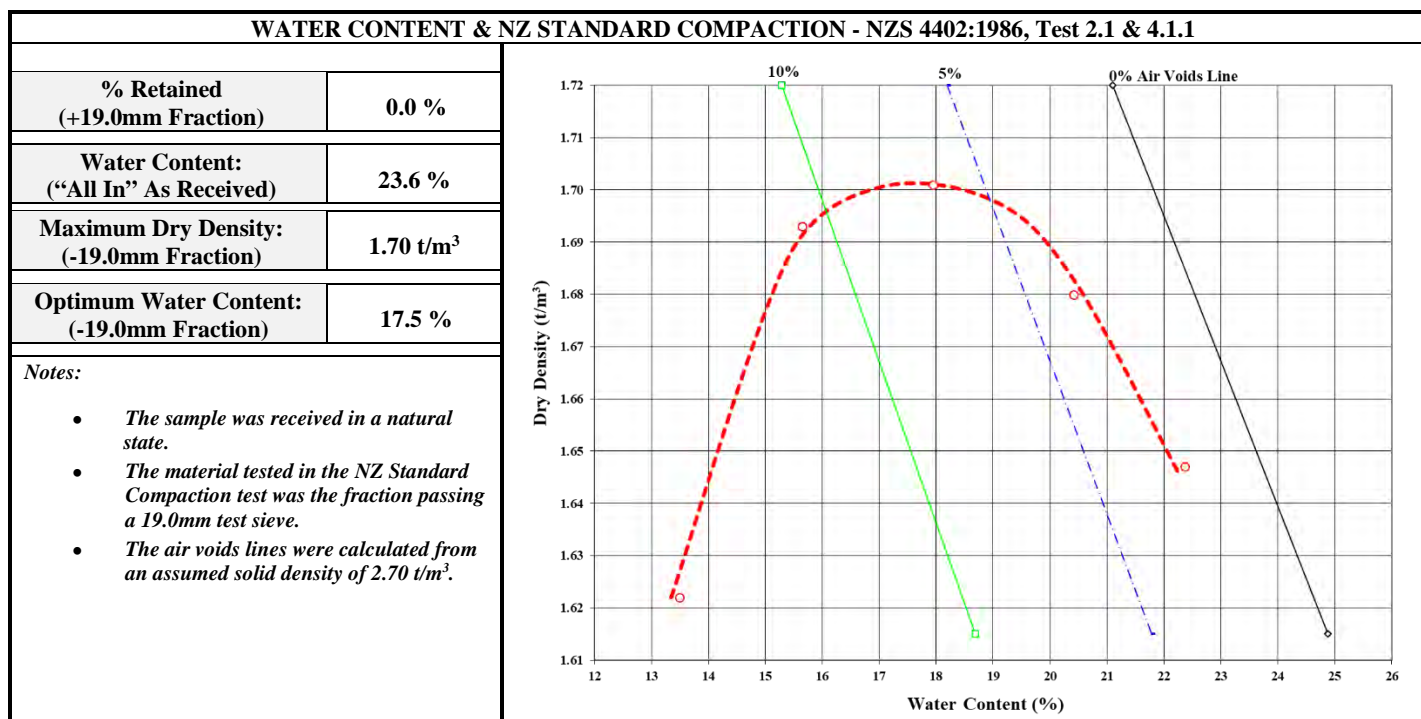
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## TEST REPORT: DCC SMOOTH HILL INVESTIGATIONS

Client Details:	GHD, Level 3, 138 Victoria Street, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Loess – Silty CLAY with minor sand and trace of gravel	Client Job No:	12506381
Sample Source:	BH5 @ 0.0m to 1.2m & BH7 @ 0.0m to 1.4m	Sample Depth:	Combined (0.0m to 1.4m)
Date & Time Sampled:	21-Jun-19	Sampled By:	M. Fitzmaurice
Sample Method:	Test Pit / Borehole *	Date Received:	26-Jun-19



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Tested By: L.T. Smith

Date: 4 to 15-Jul-19

Checked By:


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## TEST REPORT: DCC SMOOTH HILL INVESTIGATIONS

Client Details:	GHD, Level 3, 138 Victoria Street, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Loess – Silty CLAY with minor sand and trace of gravel	Client Job No:	12506381
Sample Source:	BH5 @ 0.0m to 1.2m & BH7 @ 0.0m to 1.4m	Sample Depth:	Combined (0.0m to 1.4m)
Date & Time Sampled:	21-Jun-19	Sampled By:	M. Fitzmaurice
Sample Method:	Test Pit / Borehole *	Date Received:	26-Jun-19

PINHOLE DISPERSION TEST: ASTM D4647-13e1			
Head (mm)	Elapsed Time (min)	Flow Rate (ml/s)	Colour of Outflow (Cloudiness)
50	1	0.25	Barely Visible
50	5	0.27	Moderately Dark
50	10	0.49	Very Dark
Diameter of Hole at Start of Test:			1.0mm
Diameter of Hole at End of Test:			≈ 2.0mm
Water Content Prior to Test:			17.8 %
Dry Density of Sample Tested:			1.62 t/m <sup>3</sup>
Pinhole Dispersion Classification – Method A:			Dispersive (D)
CRUMB TEST: ASTM D6572-13e2 (Method B)			
Elapsed Time	Estimated Slaking	Observations Recorded	
2 min	≈ 20 %	No colloidal cloud	
1 hr	≈ 100%	Dense colloidal cloud over	
6 hr	≈ 100 %	Dense colloidal cloud over	
Crumb Test Classification:		Grade 4 (Highly Dispersive)	
<b>Note:</b> <ul style="list-style-type: none"><li>Distilled water was used in the pinhole dispersion and crumb test. Both tests were carried out on remoulded samples.</li><li>The pinhole dispersion sample was compacted to 95% NZ standard compaction.</li><li>Photograph at completion of the crumb test.</li><li>The sample tested was the fraction passing the 2.00mm sieve.</li></ul>			

## Note:

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Tested By: L.T. Smith

Date: 4 to 15-Jul-19

Checked By: 

Approved Signatory


A.P. Julius  
Laboratory Manager

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## ***TEST REPORT – SMOOTH HILL LANDFILL INVESTIGATIONS***

Client Details:	GHD, Level 3, 138 Victoria Street, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Loess – SILT with some sand, minor gravel and minor clay	Client Job No:	12506381
Sample Source:	TP10/01	Sample Depth:	2.2m to 3.6m
Date & Time Sampled:	10-Jun-19	Sampled By:	M. Fitzmaurice
Sample Method:	Test Pit	Date Received:	26-Jun-19

CONSTANT HEAD PERMEABILITY TEST IN A TRIAXIAL CELL – ASTM D5084-16a			
Cell Pressure: (kPa)	610	Compaction:	95% NZ Standard
Saturation Back Pressure: (kPa)	600	Solid Density: (t/m <sup>3</sup> )	2.67
Effective Confining Pressure: (kPa)	10	Temperature During Test: (°C)	20.5
Saturation by Pore Pressure Response: (B Value)	0.98	Permeant Liquid Used:	De-aired Tap Water
Sample Status:	Initial	Final	
Sample Dimensions: (mm)	105.02 $\phi$ x 115.29	106.02 $\phi$ x 117.05	
Bulk Density: (t/m <sup>3</sup> )	1.92	1.98	
Water Content: (%)	18.0	26.3	
Dry Density: (t/m <sup>3</sup> )	1.62	1.57	
Saturation By Calculation: (%)	75	100	
Void Ratio: (e)	0.65	0.70	
Constant Head: (kPa)	3.0	5.0	
Hydraulic Conductivity: (k <sub>20</sub> )	2.9 x 10 <sup>-8</sup> m/s	3.2 x 10 <sup>-8</sup> m/s	

**Note:**

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Tested By: N.P. Danischewski

Date: 11-Jul-19 to 3-Aug-19

Checked By:

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## **TEST REPORT – SMOOTH HILL LANDFILL INVESTIGATIONS**

Client Details:	GHD, Level 3, 138 Victoria Street, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Loess – SILT with some sand, minor gravel and minor clay	Client Job No:	12506381
Sample Source:	TP10/01	Sample Depth:	2.2m to 3.6m
Date & Time Sampled:	10-Jun-19	Sampled By:	M. Fitzmaurice
Sample Method:	Test Pit	Date Received:	26-Jun-19

CONSTANT HEAD PERMEABILITY TEST IN A TRIAXIAL CELL – ASTM D5084-16a			
Cell Pressure: (kPa)	727	Compaction:	95% NZ Standard
Saturation Back Pressure: (kPa)	650	Solid Density: (t/m <sup>3</sup> )	2.67
Effective Confining Pressure: (kPa)	77	Temperature During Test: (°C)	18.0
Saturation by Pore Pressure Response: (B Value)	0.99	Permeant Liquid Used:	De-aired Tap Water
Sample Status:	Initial	Final	
Sample Dimensions: (mm)	105.04 $\phi$ x 115.72	105.27 $\phi$ x 116.26	
Bulk Density: (t/m <sup>3</sup> )	1.89	1.99	
Water Content: (%)	17.8	24.0	
Dry Density: (t/m <sup>3</sup> )	1.60	1.61	
Saturation By Calculation: (%)	71	97	
Void Ratio: (e)	0.67	0.66	
Constant Head: (kPa)	3.0	10.0	
Hydraulic Conductivity: (k <sub>20</sub> )	2.7 x 10 <sup>-8</sup> m/s	2.8 x 10 <sup>-8</sup> m/s	

**Note:**

- Information contained in this report which is Not IANZ Accredited relates to the sample description based on NZ Geotechnical Society Guidelines 2005 and sampling.
- This report may not be reproduced except in full.

Tested By: N.P. Danischewski

Date: 11-Jul-19 to 3-Aug-19

Checked By:

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## **TEST REPORT – SMOOTH HILL LANDFILL INVESTIGATIONS**

<b>Client Details:</b>	<b>GHD, Level 3, 138 Victoria Street, Christchurch</b>	<b>Attention:</b>	<b>J. Southworth</b>
<b>Job Description:</b>	<b>DCC Smooth Hill Landfill Investigations</b>		
<b>Sample Description:</b>	<b>Loess – Silty CLAY with minor sand and trace of gravel</b>	<b>Client Job No:</b>	<b>12506381</b>
<b>Sample Source:</b>	<b>BH5 @ 0.0m to 1.2m &amp; BH7 @ 0.0m to 1.4m</b>	<b>Sample Depth:</b>	<b>Combined (0.0m to 1.4m)</b>
<b>Date &amp; Time Sampled:</b>	<b>21-Jun-19</b>	<b>Sampled By:</b>	<b>M. Fitzmaurice</b>
<b>Sample Method:</b>	<b>Borehole</b>	<b>Date Received:</b>	<b>26-Jun-19</b>

CONSTANT HEAD PERMEABILITY TEST IN A TRIAXIAL CELL – ASTM D5084-16a			
Cell Pressure: (kPa)	460	Compaction:	95% NZ Standard
Saturation Back Pressure: (kPa)	450	Solid Density: (t/m <sup>3</sup> )	2.71
Effective Confining Pressure: (kPa)	10	Temperature During Test: (°C)	20.0
Saturation by Pore Pressure Response: (B Value)	0.97	Permeant Liquid Used:	De-aired Tap Water
Sample Status:	Initial	Final	
Sample Dimensions: (mm)	105.01 $\phi$ x 115.08	106.2 $\phi$ x 117.40	
Bulk Density: (t/m <sup>3</sup> )	1.94	1.96	
Water Content: (%)	18.7	25.9	
Dry Density: (t/m <sup>3</sup> )	1.63	1.56	
Saturation By Calculation: (%)	77	95	
Void Ratio: (e)	0.66	0.74	
Constant Head: (kPa)	3.0	5.0	
Hydraulic Conductivity: (k <sub>20</sub> )	1.7 x 10 <sup>-9</sup> m/s	2.1 x 10 <sup>-9</sup> m/s	

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**Tested By:** N.P. Danischewski

**Date:** 11-Jul-19 to 3-Aug-19

**Checked By:**

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## **TEST REPORT – SMOOTH HILL LANDFILL INVESTIGATIONS**

Client Details:	GHD, Level 3, 138 Victoria Street, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Loess – Silty CLAY with minor sand and trace of gravel	Client Job No:	12506381
Sample Source:	BH5 @ 0.0m to 1.2m & BH7 @ 0.0m to 1.4m	Sample Depth:	Combined (0.0m to 1.4m)
Date & Time Sampled:	21-Jun-19	Sampled By:	M. Fitzmaurice
Sample Method:	Borehole	Date Received:	26-Jun-19

CONSTANT HEAD PERMEABILITY TEST IN A TRIAXIAL CELL – ASTM D5084-16a			
Cell Pressure: (kPa)	527	Compaction:	95% NZ Standard
Saturation Back Pressure: (kPa)	450	Solid Density: (t/m <sup>3</sup> )	2.71
Effective Confining Pressure: (kPa)	77	Temperature During Test: (°C)	19.5
Saturation by Pore Pressure Response: (B Value)	0.97	Permeant Liquid Used:	De-aired Tap Water
Sample Status:	Initial	Final	
Sample Dimensions: (mm)	104.96 $\phi$ x 114.97	104.52 $\phi$ x 115.45	
Bulk Density: (t/m <sup>3</sup> )	1.94	2.03	
Water Content: (%)	18.7	24.3	
Dry Density: (t/m <sup>3</sup> )	1.63	1.64	
Saturation By Calculation: (%)	77	100	
Void Ratio: (e)	0.66	0.66	
Constant Head: (kPa)	3.0	5.0	
Hydraulic Conductivity: (k <sub>20</sub> )	5.6 x 10 <sup>-10</sup> m/s	5.3 x 10 <sup>-10</sup> m/s	

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Tested By: N.P. Danischewski

Date: 11-Jul-19 to 3-Aug-19

Checked By:

Approved Signatory

A.P. Julius  
Laboratory Manager

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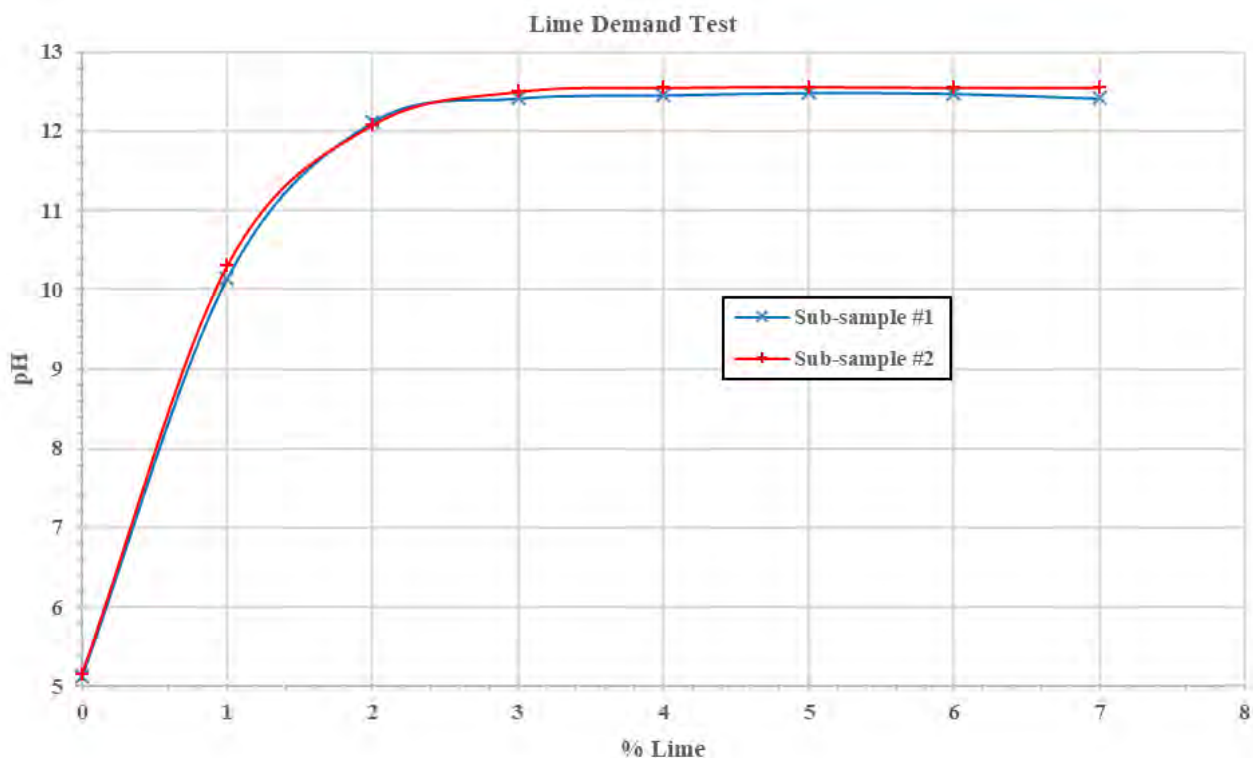




## TEST REPORT – DCC SMOOTH HILL INVESTIGATIONS

Client Details:	GHD Ltd, P.O. Box 13468, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Loess	Client Order No:	N/A
Sample Source:	BS01 @ 0.5m, BS03 @ 0.7m, BS07 @ 0.5m, BS08 @ 0.6m, BS010 @ 0.7m, BS011 @ 1.3m, BS012 @ 0.4m and BS013 @ 1.2m combined		
Date & Time Sampled:	Unknown	Sampled By:	GHD Ltd Staff
Sample Method:	Unknown	Date Received:	6-Dec-19

LIME DEMAND TEST – NSW Transport; Roads & Maritime Services Test Method T144 (Not IANZ Accredited)								
Sample Description:	Loess - Natural Soil Sub-sample #1				Loess - Natural Soil Sub-sample #2			
% Passing 2.36mm Test Sieve:	99.5%				99.5%			
Lime Type:	Taylors Hydrated Lime				Taylors Hydrated Lime			
pH of Lime Solution	12.60				12.63			
% Added Lime: (by dry mass)	0%	1%	2%	3%	4%	5%	6%	7%
pH Sub-sample #1:	5.12	10.15	12.12	12.42	12.46	12.49	12.48	12.42
pH Sub-sample #2:	5.16	10.31	12.08	12.50	12.55	12.56	12.55	12.55



**Note:**

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Tested By: L.T. Smith

Date: 9 to 17-Dec-19

Checked By:

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## ***TEST REPORT – DCC SMOOTH HILL INVESTIGATIONS***

Client Details:	GHD Ltd, P.O. Box 13468, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Loess	Client Order No:	N/A
Sample Source:	BS01 @ 0.5m, BS03 @ 0.7m, BS07 @ 0.5m, BS08 @ 0.6m, BS010 @ 0.7m, BS011 @ 1.3m, BS012 @ 0.4m and BS013 @ 1.2m combined		
Date & Time Sampled:	Unknown	Sampled By:	GHD Ltd Staff
Sample Method:	Unknown	Date Received:	6-Dec-19

WATER CONTENT & PLASTICITY INDEX RESULTS - NZS 4402:1986, Test 2.1, 2.2, 2.3 & 2.4				
Sample Description:	Loess - Natural Soil			
Water Content: (As Received)	25.0 %			
Sub-sample ID	#1	#2	#3	#4
Liquid Limit: (LL)	41	41	41	41
Plastic Limit: (PL)	25	25	25	25
Plasticity Index: (PI)	16	16	16	16
Note: The sample was received in a natural state. The plasticity index material tested was the fraction passing the 425 µm test sieve.				

## Note:

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Tested By: L.T. Smith

Date: 9 to 17-Dec-19

Checked By:

Approved Signatory

A.P. Julius  
Laboratory Manager

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## ***TEST REPORT – DCC SMOOTH HILL INVESTIGATIONS***

Client Details:	GHD Ltd, P.O. Box 13468, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Loess	Client Order No:	N/A
Sample Source:	BS01 @ 0.5m, BS03 @ 0.7m, BS07 @ 0.5m, BS08 @ 0.6m, BS010 @ 0.7m, BS011 @ 1.3m, BS012 @ 0.4m and BS013 @ 1.2m combined		
Date & Time Sampled:	Unknown	Sampled By:	GHD Ltd Staff
Sample Method:	Unknown	Date Received:	6-Dec-19

PLASTICITY INDEX RESULTS - NZS 4402:1986, Tests 2.2, 2.3 & 2.4				
Sample Description:	Loess - Natural Soil			
Sub-sample ID:	#1	#2	#3	#4
Sample Additive (By Dry Mass)	2.5% Lime	2.5% Lime	3.0% Bentonite	3.0% Bentonite
Time Cured For:	1 day	7 days	1 day	7 days
Liquid Limit: (LL)	54	55	42	40
Plastic Limit: (PL)	30	32	23	23
Plasticity Index: (PI)	24	23	19	17
Note: The sample was received in a natural state. The plasticity index material tested was the fraction passing the 425 µm test sieve.				

**Note:**

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Tested By: L.T. Smith

Date: 9 to 17-Jan-20

Checked By: 

Approved Signatory



A.P. Julius  
Laboratory Manager

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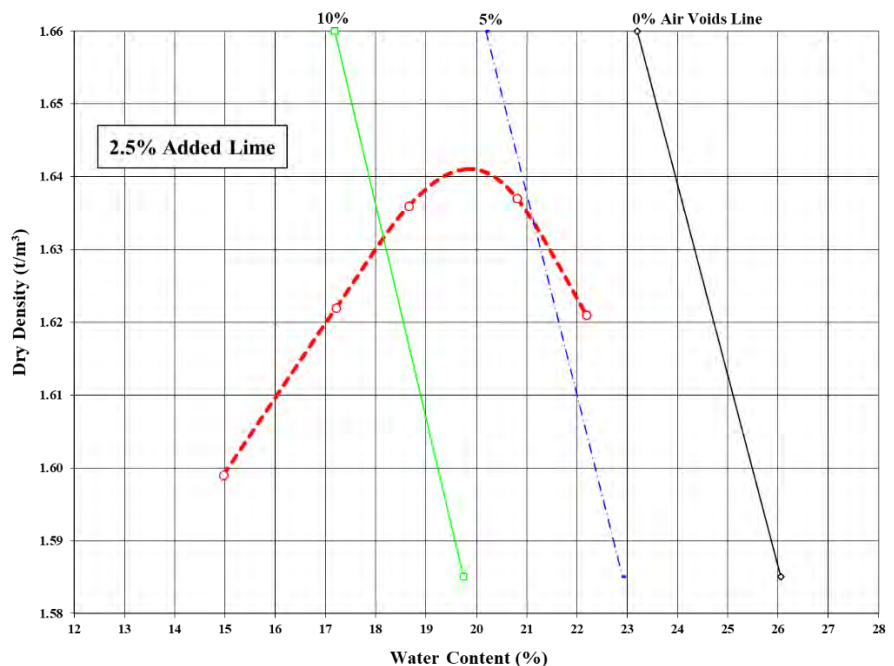
## TEST REPORT – DCC SMOOTH HILL INVESTIGATIONS

Client Details:	GHD Ltd, P.O. Box 13468, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Loess	Order No:	N/A
Sample Source:	BS01 @ 0.5m, BS03 @ 0.7m, BS07 @ 0.5m, BS08 @ 0.6m, BS010 @ 0.7m, BS011 @ 1.3m, BS012 @ 0.4m and BS013 @ 1.2m combined – 2.5% Added Hydrated Lime (by dry mass)		
Date & Time Sampled:	Unknown	Sampled By:	GHD Ltd Staff
Sample Method:	Unknown	Date Received:	6-Dec-19

NZ STANDARD COMPACTION - NZS 4402:1986, Test 4.1.1 SHEAR STRENGTH RESULTS - NZGS 2001			
% Retained: (+19.0mm Test Sieve)		0.0 %	
Maximum Dry Density:		1.64 t/m <sup>3</sup>	
Optimum Water Content:		20.0 %	
Individual Results			
Water Content (%)	Dry Density (t/m <sup>3</sup> )	Shear Strength (kPa)	
		Shear Strength	Residual Strength
15.0	1.599	UTP	UTP
17.2	1.622	UTP	UTP
18.7	1.636	UTP	UTP
20.8	1.637	UTP	UTP
22.2	1.621	> 210	-
Notes: <ul style="list-style-type: none"><li>• The material was received in a natural state.</li><li>• The air voids lines were calculated from an assumed solid density of 2.70 t/m<sup>3</sup>.</li><li>• UTP = unable to penetrate.</li><li>• The material tested was whole soil.</li></ul>			

The graph plots Dry Density (t/m³) on the y-axis (ranging from 1.58 to 1.66) against Water Content (%) on the x-axis (ranging from 12 to 28). It features three main curves: a green solid line for 10% air voids, a blue dashed line for 5% air voids, and a black solid line for 0% air voids. A red dashed curve represents the shear strength, with data points marked by open circles. A box labeled '2.5% Added Lime' is positioned near the peak of the shear strength curve. The peak of the shear strength curve occurs at approximately 20.8% water content and 1.637 t/m³ dry density.

Water Content (%)	Dry Density (t/m³)	Shear Strength (kPa)
15.0	1.599	UTP
17.2	1.622	UTP
18.7	1.636	UTP
20.8	1.637	UTP
22.2	1.621	> 210



### General Notes:

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Tested By: C. Fisher

Date: 24 to 29-Jan-20

Checked By:

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## TEST REPORT – DCC SMOOTH HILL INVESTIGATIONS

Client Details:	GHD Ltd, P.O. Box 13468, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Loess	Order No:	N/A
Sample Source:	BS01 @ 0.5m, BS03 @ 0.7m, BS07 @ 0.5m, BS08 @ 0.6m, BS010 @ 0.7m, BS011 @ 1.3m, BS012 @ 0.4m and BS013 @ 1.2m combined – 2.5% Added Hydrated Lime (by dry mass)		
Date & Time Sampled:	Unknown	Sampled By:	GHD Ltd Staff
Sample Method:	Unknown	Date Received:	6-Dec-19

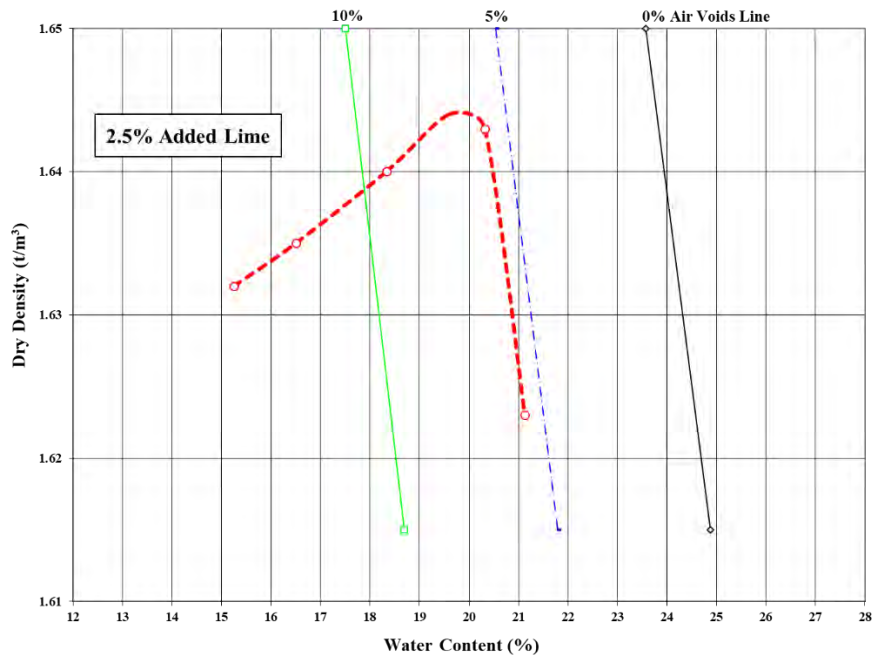
NZ STANDARD COMPACTION - NZS 4402:1986, Test 4.1.1 SHEAR STRENGTH RESULTS - NZGS 2001			
% Retained: (+19.0mm Test Sieve)		0.0 %	
Maximum Dry Density:		1.64 t/m <sup>3</sup>	
Optimum Water Content:		20.0 %	
Individual Results			
Water Content (%)	Dry Density (t/m <sup>3</sup> )	Shear Strength (kPa)	
		Shear Strength	Residual Strength
15.3	1.632	UTP	UTP
16.5	1.635	UTP	UTP
18.3	1.640	UTP	UTP
20.3	1.643	> 210	-
21.1	1.623	183	71

Notes:

- The material was received in a natural state.
- The air voids lines were calculated from an assumed solid density of 2.70 t/m<sup>3</sup>.
- UTP = unable to penetrate.
- The material tested was whole soil.

The graph plots Dry Density (t/m³) on the y-axis (1.61 to 1.65) against Water Content (%) on the x-axis (12 to 28). It includes a red dashed compaction curve peaking at 20.3% water content, a green line for 10% air voids, a blue dashed line for 5% air voids, and a black line for 0% air voids. A box labeled '2.5% Added Lime' points to the peak of the compaction curve.

Water Content (%)	Dry Density (t/m³)	Notes
15.3	1.632	Test point
16.5	1.635	Test point
18.3	1.640	Test point
20.3	1.643	Maximum Dry Density
21.1	1.623	Test point



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## TEST REPORT – DCC SMOOTH HILL INVESTIGATIONS

Client Details:	GHD Ltd, P.O. Box 13468, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Loess	Order No:	N/A
Sample Source:	BS01 @ 0.5m, BS03 @ 0.7m, BS07 @ 0.5m, BS08 @ 0.6m, BS010 @ 0.7m, BS011 @ 1.3m, BS012 @ 0.4m and BS013 @ 1.2m combined – 3.0% Added Bentonite (by dry mass)		
Date & Time Sampled:	Unknown	Sampled By:	GHD Ltd Staff
Sample Method:	Unknown	Date Received:	6-Dec-19

### NZ STANDARD COMPACTION - NZS 4402:1986, Test 4.1.1 SHEAR STRENGTH RESULTS - NZGS 2001

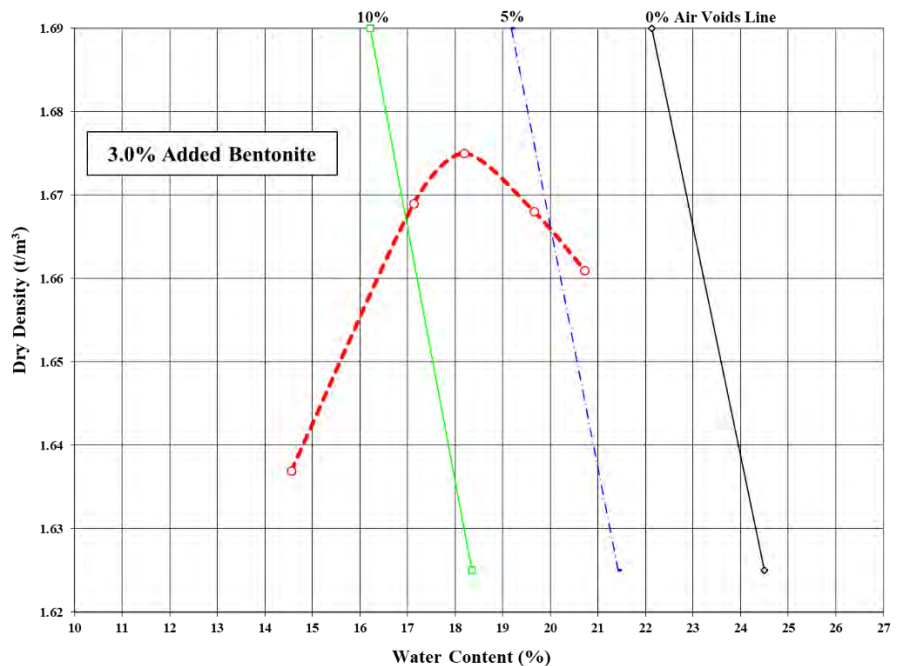
% Retained: (+19.0mm Test Sieve)		0.0 %	
Maximum Dry Density:		1.67 t/m <sup>3</sup>	
Optimum Water Content:		18.0 %	
Individual Results			
Water Content (%)	Dry Density (t/m <sup>3</sup> )	Shear Strength (kPa)	
		Shear Strength	Residual Strength
14.6	1.637	UTP	UTP
17.1	1.669	> 210	-
18.2	1.675	> 210	-
19.7	1.668	> 210	-
20.7	1.661	183	123

Notes:

- The material was received in a natural state.
- The air voids lines were calculated from an assumed solid density of 2.70 t/m<sup>3</sup>.
- UTP = unable to penetrate.
- The material tested was whole soil.

3.0% Added Bentonite

Water Content (%)	Dry Density (t/m <sup>3</sup> )	Series
14.6	1.637	Compaction Curve
16.2	1.690	10% Air Voids Line
17.1	1.669	Compaction Curve
18.2	1.675	Compaction Curve
18.2	1.625	10% Air Voids Line
19.7	1.668	Compaction Curve
19.7	1.680	5% Air Voids Line
20.7	1.661	Compaction Curve
20.7	1.625	5% Air Voids Line
22.0	1.690	0% Air Voids Line
24.5	1.625	0% Air Voids Line



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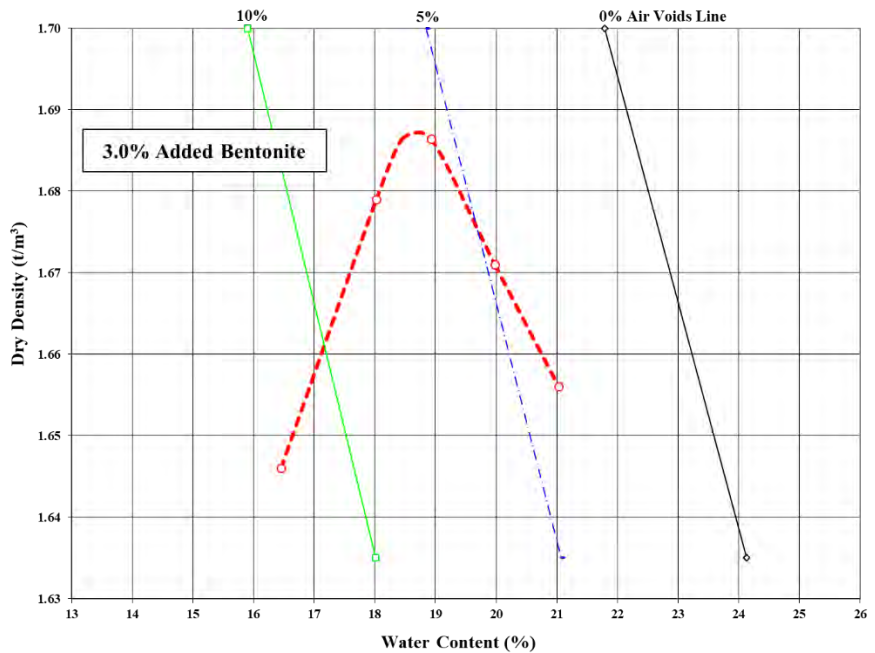
## TEST REPORT – DCC SMOOTH HILL INVESTIGATIONS

Client Details:	GHD Ltd, P.O. Box 13468, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Loess	Order No:	N/A
Sample Source:	BS01 @ 0.5m, BS03 @ 0.7m, BS07 @ 0.5m, BS08 @ 0.6m, BS010 @ 0.7m, BS011 @ 1.3m, BS012 @ 0.4m and BS013 @ 1.2m combined – 3.0% Added Bentonite (by dry mass)		
Date & Time Sampled:	Unknown	Sampled By:	GHD Ltd Staff
Sample Method:	Unknown	Date Received:	6-Dec-19

NZ STANDARD COMPACTION - NZS 4402:1986, Test 4.1.1 SHEAR STRENGTH RESULTS - NZGS 2001			
% Retained: (+19.0mm Test Sieve)		0.0 %	
Maximum Dry Density:		1.69 t/m <sup>3</sup>	
Optimum Water Content:		19.0 %	
Individual Results			
Water Content (%)	Dry Density (t/m <sup>3</sup> )	Shear Strength (kPa)	
		Shear Strength	Residual Strength
16.5	1.646	UTP	UTP
18.0	1.679	> 210	-
18.9	1.686	> 210	-
20.0	1.671	186	87
22.0	1.656	147	81
Notes:			
<ul style="list-style-type: none"><li>• The material was received in a natural state.</li><li>• The air voids lines were calculated from an assumed solid density of 2.70 t/m<sup>3</sup>.</li><li>• UTP = unable to penetrate.</li><li>• The material tested was whole soil.</li></ul>			

The graph plots Dry Density (t/m³) on the y-axis (ranging from 1.63 to 1.70) against Water Content (%) on the x-axis (ranging from 13 to 26). It features three main curves: a green solid line for 10% air voids, a red dashed line for 3.0% added bentonite, and a blue dashed line for 5% air voids. A black solid line represents the 0% air voids line. Data points for shear strength are plotted as open circles on the 3.0% bentonite curve and as open squares on the 10% air voids curve. A text box labeled '3.0% Added Bentonite' is positioned near the peak of the red dashed curve.

Water Content (%)	Dry Density (t/m³)	Shear Strength (kPa)	Residual Strength (kPa)
16.5	1.646	UTP	UTP
18.0	1.679	> 210	-
18.9	1.686	> 210	-
20.0	1.671	186	87
22.0	1.656	147	81



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Tested By: C. Fisher

Date: 24 to 29-Jan-20

Checked By:

Approved Signatory

A.P. Julius  
Laboratory Manager

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Accreditation No: 434



## TEST REPORT – DCC SMOOTH HILL INVESTIGATIONS

Client Details:	GHD Ltd, P.O. Box 13468, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Loess	Order No:	N/A
Sample Source:	BS01 @ 0.5m, BS03 @ 0.7m, BS07 @ 0.5m, BS08 @ 0.6m, BS010 @ 0.7m, BS011 @ 1.3m, BS012 @ 0.4m and BS013 @ 1.2m combined – 2.5% Added Hydrated Lime (by dry mass) – 1 day curing		
Date & Time Sampled:	Unknown	Sampled By:	GHD Ltd Staff
Sample Method:	Unknown	Date Requested:	20-Jan-20

### PINHOLE DISPERSION TEST: ASTM D4647-13e1

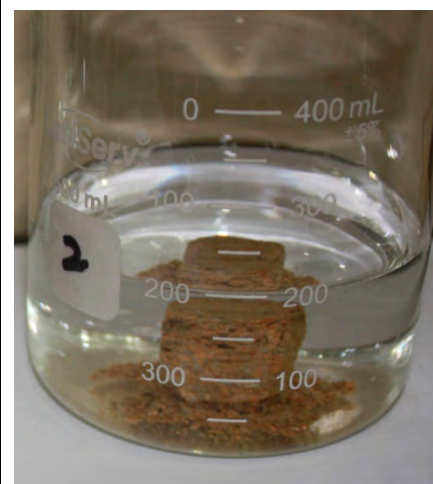
Head (mm)	Elapsed Time (min)	Flow Rate (ml/s)	Colour of Outflow (Cloudiness)
50	5	0.43	Completely Clear
50	10	0.44	Completely Clear
180	15	0.67	Completely Clear
380	20	0.95	Completely Clear
1020	25	1.82	Completely Clear

Diameter of Hole at Start of Test:	1.0 mm
Diameter of Hole at End of Test:	1.0 mm
Water Content Prior to Test:	20.0 %
Dry Density of Sample Tested:	1.56 t/m <sup>3</sup>

Pinhole Dispersion Classification – Method A: (1 Day Curing)	ND1 (Non-Dispersive)
--	----------------------

### CRUMB TEST: ASTM D6572-13e2 (Method B)

Elapsed Time	Estimated Slaking	Observations Recorded
2 min	< 2%	Clear – no colloidal cloud evident
1 hr	≈ 5%	Clear – no colloidal cloud evident
6 hr	≈ 5% - 10%	Clear – no colloidal cloud evident
Crumb Test Classification: (1 Day Curing)		Grade 1 (Non-Dispersive)



#### Note:

- Distilled water was used in the pinhole dispersion and crumb test. Both tests were carried out on remoulded samples.
- The pinhole dispersion sample was compacted to 95% NZ standard compaction.
- Photograph at completion of the crumb test.
- The sample tested was the fraction passing the 2.00mm sieve.

#### General Notes:

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Tested By: L.T. Smith

Date: 25-Jan-20 to 7-Feb-20

Checked By:

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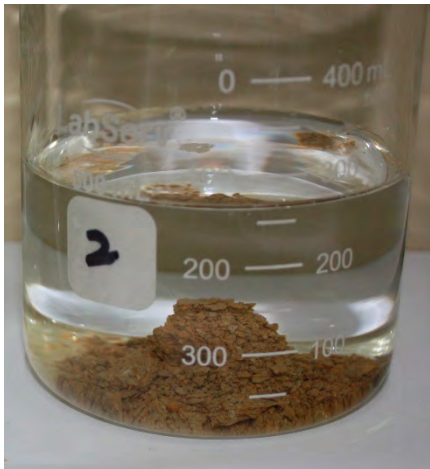
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## TEST REPORT – DCC SMOOTH HILL INVESTIGATIONS

Client Details:	GHD Ltd, P.O. Box 13468, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Loess	Order No:	N/A
Sample Source:	BS01 @ 0.5m, BS03 @ 0.7m, BS07 @ 0.5m, BS08 @ 0.6m, BS010 @ 0.7m, BS011 @ 1.3m, BS012 @ 0.4m and BS013 @ 1.2m combined – 2.5% Added Hydrated Lime (by dry mass) – 7 days curing		
Date & Time Sampled:	Unknown	Sampled By:	GHD Ltd Staff
Sample Method:	Unknown	Date Requested:	20-Jan-20

PINHOLE DISPERSION TEST: ASTM D4647-13e1			
Head (mm)	Elapsed Time (min)	Flow Rate (ml/s)	Colour of Outflow (Cloudiness)
50	5	0.30	Completely Clear
50	10	0.30	Completely Clear
180	15	0.60	Completely Clear
380	20	0.94	Completely Clear
1020	25	1.73	Completely Clear
Diameter of Hole at Start of Test:			1.0 mm
Diameter of Hole at End of Test:			1.0 mm
Water Content Prior to Test:			19.7 %
Dry Density of Sample Tested:			1.56 t/m <sup>3</sup>
Pinhole Dispersion Classification – Method A: (7 Day Curing)			ND1 (Non-Dispersive)
CRUMB TEST: ASTM D6572-13e2 (Method B)			
Elapsed Time	Estimated Slaking	Observations Recorded	
2 min	< 1%	Clear – no colloidal cloud evident	
1 hr	≈ 30%	Clear – no colloidal cloud evident	
6 hr	≈ 60%	Clear – no colloidal cloud evident	
Crumb Test Classification: (7 Day Curing)		Grade 1 (Non-Dispersive)	
<div><div>Note:<ul style="list-style-type: none"><li>Distilled water was used in the pinhole dispersion and crumb test. Both tests were carried out on remoulded samples.</li><li>The pinhole dispersion sample was compacted to 95% NZ standard compaction.</li><li>Photograph at completion of the crumb test.</li><li>The sample tested was the fraction passing the 2.00mm sieve.</li></ul></div><div></div></div>			

### General Notes:

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Tested By: L.T. Smith

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## TEST REPORT – DCC SMOOTH HILL INVESTIGATIONS

Client Details:	GHD Ltd, P.O. Box 13468, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Loess	Order No:	N/A
Sample Source:	BS01 @ 0.5m, BS03 @ 0.7m, BS07 @ 0.5m, BS08 @ 0.6m, BS010 @ 0.7m, BS011 @ 1.3m, BS012 @ 0.4m and BS013 @ 1.2m combined – 3.0% Added Bentonite (by dry mass) – 1 day curing		
Date & Time Sampled:	Unknown	Sampled By:	GHD Ltd Staff
Sample Method:	Unknown	Date Requested:	20-Jan-20

PINHOLE DISPERSION TEST: ASTM D4647-13e1			
Head (mm)	Elapsed Time (min)	Flow Rate (ml/s)	Colour of Outflow (Cloudiness)
50	1	0.50	Dark
50	3	1.23	Very Dark
50	5	2.23	Very Dark
Diameter of Hole at Start of Test:		1.0 mm	
Diameter of Hole at End of Test:		3.0 mm	
Water Content Prior to Test:		17.6 %	
Dry Density of Sample Tested:		1.60 t/m <sup>3</sup>	
Pinhole Dispersion Classification – Method B: (1 Day Curing)		D (Dispersive)	

CRUMB TEST: ASTM D6572-13e2 (Method B)		
Elapsed Time	Estimated Slaking	Observations Recorded
2 min	< 2%	Colloidal cloud evident around cube
1 hr	≈ 55%	Heavy colloidal cloud ≈ 20mm deep covering entire bottom
6 hr	≈ 80%	Heavy colloidal cloud ≈ 20mm deep covering entire bottom
Crumb Test Classification: (1 Day Curing)		Grade 4 (Dispersive)



**Note:**

- Distilled water was used in the pinhole dispersion and crumb test. Both tests were carried out on remoulded samples.
- The pinhole dispersion sample was compacted to 95% NZ standard compaction.
- Photograph at completion of the crumb test.
- The sample tested was the fraction passing the 2.00mm sieve.

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Tested By: L.T. Smith

Date: 25-Jan-20 to 7-Feb-20

Checked By:

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## TEST REPORT – DCC SMOOTH HILL INVESTIGATIONS

Client Details:	GHD Ltd, P.O. Box 13468, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Loess	Order No:	N/A
Sample Source:	BS01 @ 0.5m, BS03 @ 0.7m, BS07 @ 0.5m, BS08 @ 0.6m, BS010 @ 0.7m, BS011 @ 1.3m, BS012 @ 0.4m and BS013 @ 1.2m combined – 3.0% Added Bentonite (by dry mass) – 7 days curing		
Date & Time Sampled:	Unknown	Sampled By:	GHD Ltd Staff
Sample Method:	Unknown	Date Requested:	20-Jan-20

### PINHOLE DISPERSION TEST: ASTM D4647-13e1

Head (mm)	Elapsed Time (min)	Flow Rate (ml/s)	Colour of Outflow (Cloudiness)
50	1	1.43	Very Dark
50	3	2.33	Very Dark
50	5	2.58	Very Dark

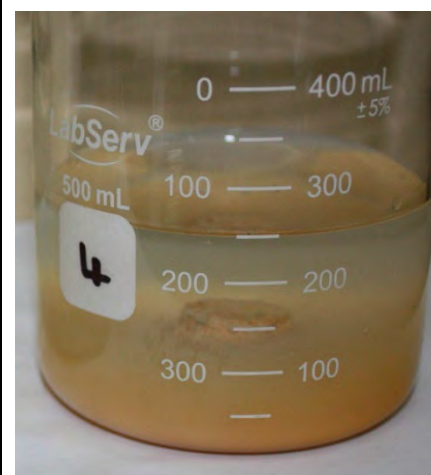
Diameter of Hole at Start of Test:	1.0 mm
Diameter of Hole at End of Test:	4.0 mm
Water Content Prior to Test:	17.9 %
Dry Density of Sample Tested:	1.60 t/m <sup>3</sup>

Pinhole Dispersion Classification – Method B: (7 Day Curing)	D (Dispersive)
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### CRUMB TEST: ASTM D6572-13e2 (Method B)

Elapsed Time	Estimated Slaking	Observations Recorded
2 min	< 1%	Colloidal cloud evident around cube
1 hr	≈ 40%	Heavy colloidal cloud ≈ 20mm deep covering entire bottom
6 hr	≈ 95% - 100%	Heavy colloidal cloud ≈ 20mm deep covering entire bottom

Crumb Test Classification: (7 Day Curing)	Grade 4 (Dispersive)
---	----------------------



#### Note:

- Distilled water was used in the pinhole dispersion and crumb test. Both tests were carried out on remoulded samples.
- The pinhole dispersion sample was compacted to 95% NZ standard compaction.
- Photograph at completion of the crumb test.
- The sample tested was the fraction passing the 2.00mm sieve.

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Date: 25-Jan-20 to 7-Feb-20

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## TEST REPORT – DCC SMOOTH HILL INVESTIGATIONS

Client Details:	GHD Ltd, P.O. Box 13468, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Loess	Order No:	N/A
Sample Source:	BS01 @ 0.5m, BS03 @ 0.7m, BS07 @ 0.5m, BS08 @ 0.6m, BS010 @ 0.7m, BS011 @ 1.3m, BS012 @ 0.4m and BS013 @ 1.2m combined – 2.5% Added Hydrated Lime (by dry mass)		
Date & Time Sampled:	Unknown	Sampled By:	GHD Ltd Staff
Sample Method:	Unknown	Date Requested:	20-Jan-20

PLASTICITY INDEX RESULTS - NZS 4402:1986, Test 2.2, 2.3 & 2.4	
Sample Description:	Loess - 2.5% Added Hydrated Lime (by dry mass)
Time Cured For:	28 days
Liquid Limit: (LL)	53
Plastic Limit: (PL)	30
Plasticity Index: (PI)	23
<i>Note: The sample was received in a natural state. The plasticity index material tested was the fraction passing the 425 µm test sieve.</i>	

## General Notes:

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Tested By: L.T. Smith

Date: 25-Jan-20 to 7-Feb-20

Checked By:

Approved Signatory

A.P. Julius  
Laboratory Manager

All tests reported herein  
have been performed in  
accordance with the  
scope of the  
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## TEST REPORT – DCC SMOOTH HILL INVESTIGATIONS

Client Details:	GHD Ltd, P.O. Box 13468, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Siltstone – Sandy SILT with minor clay	Client Order No:	Not Stated
Sample Source:	BH05 @ 2.7m - 7.2m	Sample Label No:	N/A
Date & Time Sampled:	Unknown	Sampled By:	GHD Ltd Staff
Sample Method:	Borehole *	Date Received:	December 2019

### WATER CONTENT & NZ STANDARD COMPACTION - NZS 4402:1986, Test 2.1 & 4.1.1

% Retained (+19.0mm Fraction)	0.0 %
Water Content: ("All In" As Received)	13.7 %
Maximum Dry Density:	1.76 t/m <sup>3</sup>
Optimum Water Content:	16.0 %
<b>Notes:</b> <ul style="list-style-type: none"> <li>The sample was received in a natural state.</li> <li>The material tested in the NZ Standard Compaction test was whole soil.</li> <li>The air voids lines were calculated from an assumed solid density of 2.70 t/m<sup>3</sup>.</li> </ul>	
<p>The graph plots Dry Density (t/m³) on the y-axis (1.69 to 1.78) against Water Content (%) on the x-axis (8 to 24). It features a red dashed compaction curve peaking at 1.76 t/m³ at 16.0% water content. Three air voids lines are shown: a green line for 10% air voids, a blue dashed line for 5% air voids, and a black solid line for 0% air voids. Data points from the test are plotted as open circles along the compaction curve.</p>	

### PLASTICITY INDEX RESULTS - NZS 4402:1986, Test 2.2, 2.3 & 2.4

Liquid Limit: (LL)	41
Plastic Limit: (PL)	25
Plasticity Index: (PI)	16
<i>Note: The sample was received in a natural state. The plasticity index material tested was the fraction passing the 425 µm test sieve.</i>	

**Note:**

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Tested By: L.T. Smith, N.P. Danischewski, C. Fisher &amp; C. Pearson

Date: 10-Jan-20 to 4-Mar-20

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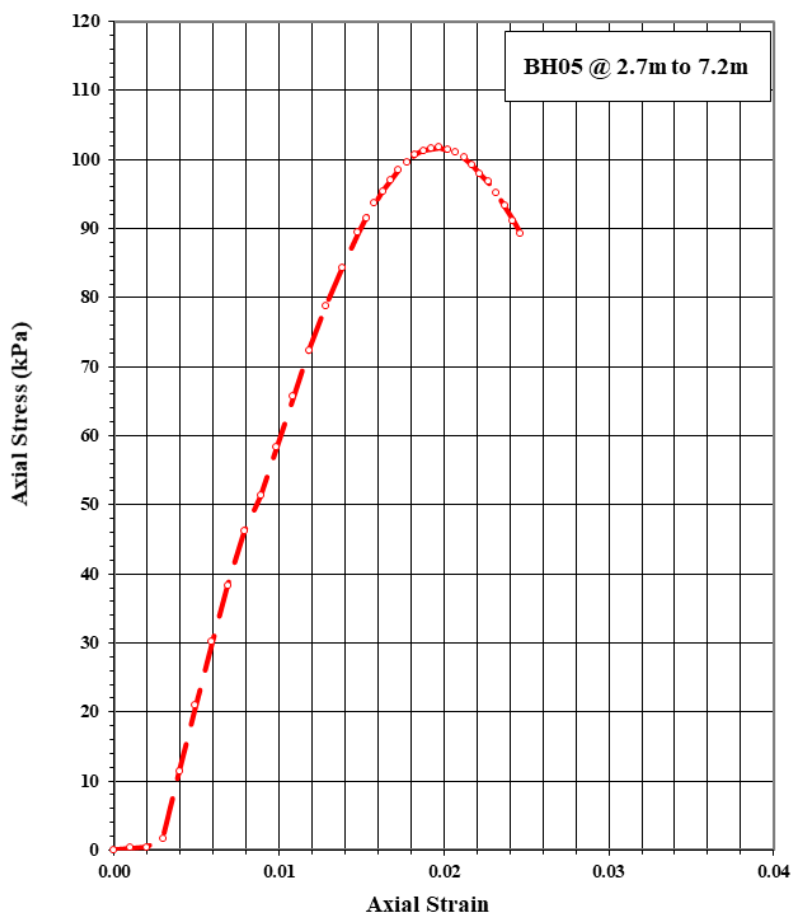


## TEST REPORT – DCC SMOOTH HILL INVESTIGATIONS

Client Details:	GHD Ltd, P.O. Box 13468, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Siltstone – Sandy SILT with minor clay	Client Order No:	Not Stated
Sample Source:	BH05 @ 2.7m - 7.2m	Sample Label No:	N/A
Date & Time Sampled:	Unknown	Sampled By:	GHD Ltd Staff
Sample Method:	Borehole *	Date Received:	December 2019

### UNCONFINED COMPRESSIVE STRENGTH - NZS 4402:1986, Test 6.3.1

Sample Diameter: (mm)	101.50
Sample Length: (mm)	202.68
Length / Diameter Ratio:	2.00
Bulk Density: (t/m <sup>3</sup> )	1.94
Water Content: (%)	15.9
Dry Density (t/m <sup>3</sup> )	1.67
Mode of Failure:	Plastic / Plastic Brittle
Strain @ Failure:	2.0 %
Load @ Failure:	0.840 kN
Unconfined Compressive Strength:	100 kPa
Notes: 1. Dry density rounded to the nearest 0.01 t/m <sup>3</sup> . 2. The rate of axial compression was 0.40 mm/min.	



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## TEST REPORT – DCC SMOOTH HILL INVESTIGATIONS

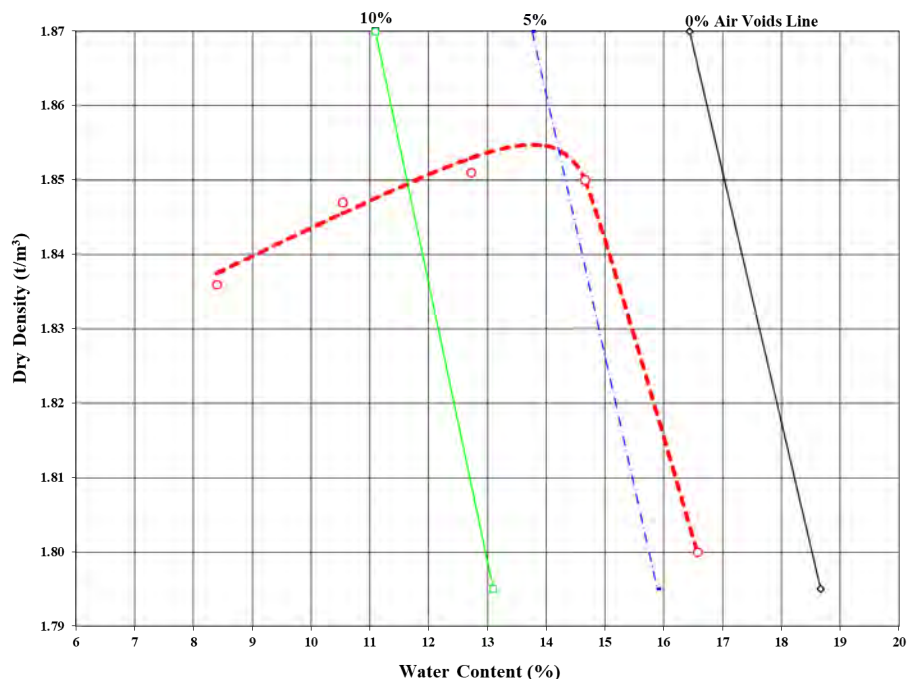
Client Details:	GHD Ltd, P.O. Box 13468, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Sands & Sandstone; SILT & SAND with minor clay	Client Order No:	Not Stated
Sample Source:	BH10 @ 2.4m - 7.0m	Sample Label No:	N/A
Date & Time Sampled:	Unknown	Sampled By:	GHD Ltd Staff
Sample Method:	Borehole *	Date Received:	December 2019

### WATER CONTENT & NZ STANDARD COMPACTION - NZS 4402:1986, Test 2.1 & 4.1.1 DRY DENSITY & ABSORPTION - NZS 3111:1986, Test 12

% Retained (+19.0mm Fraction)	8.0 %
Water Content: ("All In" As Received)	11.9 %
Dry Density: (+19.0mm Fraction)	2.45 t/m <sup>3</sup>
Absorption (+19.0mm Fraction)	3.7 %
Maximum Dry Density: (-19.0mm Fraction)	1.85 t/m <sup>3</sup>
Optimum Water Content: (-19.0mm Fraction)	14.0 %

#### Notes:

- The sample was received in a natural state.
- The material tested in the NZ Standard Compaction test was the fraction passing a 19.0mm test sieve.
- The air voids lines were calculated from an assumed solid density of 2.70 t/m<sup>3</sup>.



### PLASTICITY INDEX RESULTS - NZS 4402:1986, Test 2.2, 2.3 & 2.4

Liquid Limit: (LL)	37
Plastic Limit: (PL)	23
Plasticity Index: (PI)	14
Note: The sample was received in a natural state. The plasticity index material tested was the fraction passing the 425 µm test sieve.	

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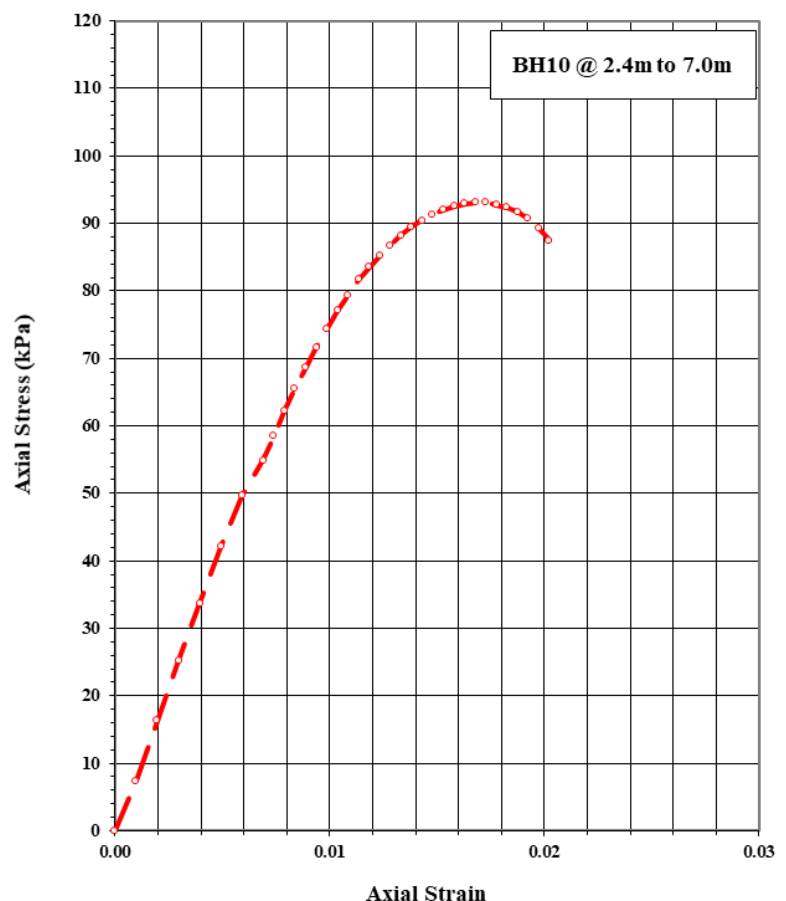


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Client Details:	GHD Ltd, P.O. Box 13468, Christchurch	Attention:	J. Southworth
Job Description:	DCC Smooth Hill Landfill Investigations		
Sample Description:	Sands & Sandstone; SILT & SAND with minor clay	Client Order No:	Not Stated
Sample Source:	BH10 @ 2.4m - 7.0m	Sample Label No:	N/A
Date & Time Sampled:	Unknown	Sampled By:	GHD Ltd Staff
Sample Method:	Borehole *	Date Received:	December 2019

### UNCONFINED COMPRESSIVE STRENGTH - NZS 4402:1986, Test 6.3.1

Sample Diameter: (mm)	101.49
Sample Length: (mm)	202.66
Length / Diameter Ratio:	2.00
Bulk Density: (t/m <sup>3</sup> )	2.00
Water Content: (%)	13.9
Dry Density (t/m <sup>3</sup> )	1.76
Mode of Failure:	Plastic / Plastic Brittle
Strain @ Failure:	1.7 %
Load @ Failure:	0.767 kN
Unconfined Compressive Strength:	93 kPa
<b>Notes:</b> <ol style="list-style-type: none"> <li>Dry density rounded to the nearest 0.01 t/m<sup>3</sup>.</li> <li>The rate of axial compression was 0.40 mm/min.</li> </ol>	



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This report has been prepared by Matt Fitzmaurice, John Southworth and Dhugal McQuistan under the direction of Samantha Webb, a Technical Director and Engineering Geologist with GHD Ltd. Matt has 9 years as an engineering geologist, John has 23 years experience as an engineering geologist and Dhugal has 4 years experience as a geotechnical engineer. Samantha has 30 years in all aspects of engineering geology including a number of landfill projects and has the following qualifications BSc (Hons) Earth Sciences and MSc Engineering Geology.

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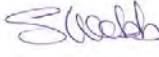

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Document Status

Revision	Author	Reviewer		Approved for Issue		
		Name	Signature	Name	Signature	Date
Rev01	M. Fitzmaurice/ J. Southworth	S.Webb		S.Douglass		17-8-20

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