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Landslide Monitoring Report - Greenacres Street, Macandrew Bay

6 August 2021 CONFIDENTIAL







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Contents

Discl	laimei	rs and Limitations	4
Exec	utive	Summary	5
1	Intro	oduction	6
2	Surv	ey Monitoring	6
	2.1	Monitoring History	
	2.2	Methodology	
	2.3	Accuracy	
	2.4	Future Monitoring	8
3	Mon	itoring Results	8
4	Rain	fall Data	9
5	Aeria	al Photo Inspection	10
6	Cond	clusions and Recommendations	10
List	of Fig	gures	
		easured monthly rainfall compared with average monthly rainfall (CliFlo.niwa.co.nz)	9
List	of Ta	ables	
		mmary of recorded displacements	
Table	e 2 Su	mmary of deformation monitoring results since the previous and base surveys	8

List of Appendices

- Network Diagram Α
- В
- Selected Monitoring Charts Cumulative Monitoring Results Spreadsheet С

Disclaimers and Limitations

This report ('Report') has been prepared by WSP exclusively for Dunedin City Council ('Client') in relation to the landslide monitoring at selected sites in Dunedin (Long-Term Landslide Monitoring SoW DCC Reference 9662). The scope of this report is to present the survey monitoring results and recommendations for future surveys for the site ('Purpose'). The findings in this Report are based on and subject to the assumptions specified in the Report. WSP accepts no liability whatsoever for any reliance on or use of this Report, in whole or in part, for any use or purpose other than the Purpose or any use or reliance on the Report by any third party.

Executive Summary

A recent survey of the Greenacres Street site has been undertaken in June 2021 to assess the extent of movements compared with previous surveys. The main findings, where deformations were found to exceed the accuracy of the survey (±15mm Hz, ±20mm V), are presented in Table 1 below.

Table 1 Summary of recorded displacements.

	Horizontal	Vertical
Displacements from the previous survey	<15mm	<20mm
Displacements from the original survey	310 - 1,610mm	240 - 680mm

The results indicate that horizontal and vertical movement has slowed relative to previous rates and no detectable movement has been measured since the previous survey (May 2020).

We recommend the next survey to be undertaken in mid-2022.

1 Introduction

WSP have been commissioned by Dunedin City Council (DCC) to undertake monitoring of 12 landslide sites around Dunedin. The purpose of monitoring is to identify the trend and magnitude of movements and provide recommendations for future monitoring.

This report presents a summary of the factual survey monitoring results for the Greenacres Street site, as well as monitoring recommendations. A survey network diagram is provided in Appendix A.

2 Survey Monitoring

2.1 Monitoring History

Survey monitoring of the site has been undertaken since 2015 the baseline was recorded on 31 July 2015. The first 3 surveys (including the base survey) were initially completed at 2 - 4 month intervals. The frequency was then extended to approximately 1 year, however the survey in mid-2019 was missed. Approximate survey dates are shown in monitoring charts presented in Appendix B.

2.2 Methodology

This survey monitoring round was undertaken by a WSP Surveyor on 17 June 2021. The specific equipment used were 2 x Trimble R8-3 GNSS receivers.

2.2.1 Field Survey

The survey was undertaken using Static GNSS with 20-minute observations to each monitor mark, logging data at 1s intervals. Monitor marks were occupied by setting the GNSS receiver over the mark with an optical plummet and tripod. Receiver heights were measured by steel tape with slant distance reduced to true vertical.

The CORS network mark DUND, approximately 1.2km from the site, served as the local base station.

IS 5 was not measured as it could not be accessed due to a stockpile of slash from pine forest clearance.

2.2.2 Office Processing

Survey data is processed using Trimble Business Centre post-processing software. Static data from CORS network marks DUND and OUSD were combined with the static data from the rover. A minimally constrained network adjustment was carried out holding control point DUND fixed horizontally in terms of NZGD2000 (Global Reference Epoch 2000.0), North Taieri Circuit projection.

DUND was also held fixed vertically in terms of the GRS80 ellipsoid height. The NZVD2016 geoid was applied in the background during processing for checking purposes. The current ellipsoid height of DUND has been updated since the survey began in 2015, so it is important the original height is maintained to keep measurements in the same terms. The origin of coordinates for the survey is:

DUND	797475.867mN	424425.439mE	386.925mh	(OHz , 1V)
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Vertical check X75 (AFF3, order 2V) failed vertical datum check, being 0.034m higher than the LINZ database level relative to DUND, which agreed with OUSD to 0.003m. This could in part be due to the level of DUND being updated (lowered 0.009m ellipsoid). The observation is relatively consistent with the historical measurements of X75 being 0.013m lower than the 2020 monitor. It was also noted that the seawall adjacent to the mark had been under-cut and ground around the mark had noticeable cracks.

2.2.3 Geodetic Parameters

For continuity of future surveys, it is critical the following geodetic parameters are maintained:

Coordinate System

Summary		
Coordinate system group:	New Zealand/NZGD2000	
Zone:	North Taieri 2000	
Datum transformation:	New Zealand Geodetic 2000 (Molodensky)	
Global reference datum:	NZGD2000	
Global reference epoch:	2000	
Geoid model:	New Zealand Geoid 2016	
RTX datum:	No	

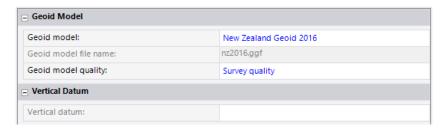
Transformation Parameters, Local Ellipsoid

Datum Transformation		
Method:	Molodensky	
Translation X:	0.0000 m	
Translation Y:	0.0000 m	
Translation Z:	0.0000 m	
Local ellipsoid used:	Geodetic Ref System 1980	
Local ellipsoid semi-major axis:	6378137.0000 m	
Local ellipsoid inverse flattening:	298.257222101	

<u>Projection Settings</u>

Name:	Transverse Mercator	
Origin latitude:	S45°51'41.00000"	
Origin longitude:	E170°16'57.00000"	
False easting:	400000.0000 m	
False northing:	800000.0000 m	
Scale factor:	0.9999600000	
South azimuth system:	No	
Positive coordinate direction:	North / East	

Vertical Datum (Background Use Only)



2.3 **Accuracy**

Considering the equipment, methodology and site observing conditions, this survey is considered to meet the following accuracy:

Horizontal position +/- 15mm (@ 95% CI) Vertical position +/- 20mm (@ 95% CI)

2.4 Future Monitoring

It is recommended that future survey monitors follow the methodology and geodetic parameters detailed above. The lack of redundant control points to check to means the accuracy and repeatability of the survey cannot be confirmed by a simple, practical measurement. Also considering the sole check mark at X75 could be subject to ground disturbance, additional control points should be installed or checked on to during the next monitor. This would add approximately 0.75 hours per control point on all future monitors.

3 Monitoring Results

The cumulative results spreadsheet is presented in Appendix C of this report. The monitoring results are discussed below.

Table 2 Summary of deformation monitoring results since the previous and base surveys.

Deformation since prev		previous survey	Deformation since	base survey
	Horizontal	Vertical	Horizontal	Vertical
Average	<15mm	<20mm	1,070mm	-410mm
Maximum	<15mm	<20mm	1,610mm	-680mm

4 Rainfall Data

A summary of the rainfall data since the previous survey is presented in Figure 1 below. Data was retrieved from the NIWA (National Institute of Water and Atmospheric Research) National Climate Database website (CliFlo.niwa.co.nz) using the Musselburgh Station (Agent ID #15752). Mean monthly rainfall is calculated for the "Dunedin" area between 1981 and 2010 (source: https://niwa.co.nz/education-and-training/schools/resources/climate/meanrain).

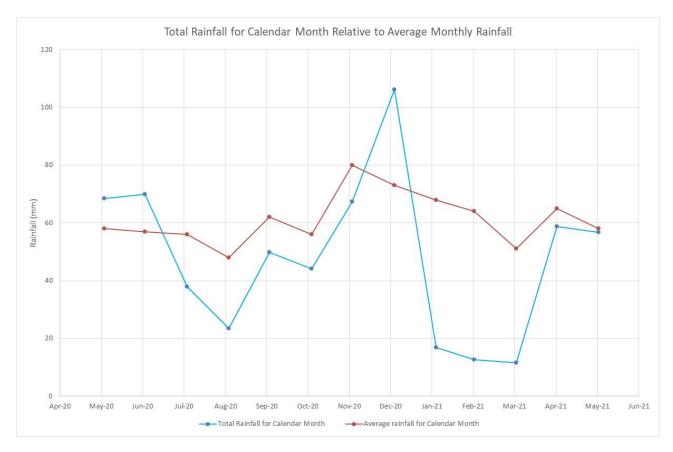


Figure 1 Measured monthly rainfall compared with average monthly rainfall (CliFlo.niwa.co.nz).

5 Aerial Photo Inspection

A brief assessment of aerial photographs (DCC Rates WebMap) and the mapped landslide extent (see Network Diagram in Appendix A) has shown that some of the landslide area was covered by a pine plantation which was harvested at some point between 2013 and 2018/19. In addition, machinery has damaged much of the grassed area overlying the lower reaches of the landslide. It is considered possible that the removal of trees, and resultant damage to the vegetated surface, may have contributed to the recorded movement.

6 Conclusions and Recommendations

In conclusion, the maximum displacements recorded since the July 2015 base survey are as follows:

- 1,610mm in plan,
- 680mm vertically.

Survey results indicate the rate of movement has slowed since large deformations (up to 1.0m horizontally, mark IS4) were recorded between 2017 and 2018. A maximum of 0.4m horizontal movement (up to -0.43m vertically, mark IS2) has been recorded since March 2018. No detectable deformation has been recorded since the previous survey (May 2020); on this basis, we do not see any requirement to change the currently established communication plan with the affected residents.

Previous surveys have shown that large deformations are possible, likely the result of adverse conditions such as heavy rainfall events or a change in land use (e.g. removal of vegetation). Due to this consideration, and the close proximity of residential dwellings (38, 42, and 46 Greenacres Street) to the toe of the landslide, it is recommended a follow-up survey is completed on a 12-monthly basis.

In addition, we recommend consideration be given to a walkover inspection of the site following any significant rainfall event. Without undertaking a close review of deformation relative to daily rainfall, a preliminary threshold has been proposed below as a point for discussion.

- >40mm in 24 hours;
- >60mm in 48 hours; or,
- >100mm in 7 days, as reported by the Musselburgh NIWA weather station, or other equivalent weather monitoring site.

If future surveys or inspections reveal adverse deformation trends, we recommend the DCC discuss possible treatment options with WSP. Given the data presented thus far, some examples of this may include vegetation management (planting grass and shrubs) and improving control of surface water.

Finally, it is recommended that the logging debris obscuring mark IS 5 be cleared to enable access for future surveys.

Appendix A Network Diagram

ACCURACY IS +/- 15mm

HORIZONTAL VECTOR ARROWS ARE AT 1:10 OF THE

LEGEND

LANDSLIDE - OTAGO REGION (CERTAINTY)

LIKELY

DEFINITE

POSSIBLE



NOT ASSESSED NO INFORMATION

INDICATIVE LANDSLIDE EXTENTS ARE BASED ON "REVISED LANDSLIDE DATABASE FOR THE COASTAL SECTOR OF THE DUNEDIN CITY DISTRICT" BY BARRELL, D.J.A., SMITH LYTTLE, B., GLASSEY, P.J. GNS SCIENCE CONSULTANCY REPORT 2017/41, JULY 2017, SOURCED FROM THE OTAGO REGIONAL COUNCIL (ORC) NATURAL HAZARDS PORTAL.

OTHER

- +/- Xmm = CUMULATIVE VERTICAL DISPLACEMENT
- CUMULATIVE HORIZONTAL DISPLACEMENT (1:100 SCALE)



SURVEY MARKER



1:1000 @ A1 1:2000 @ A3 0 10 20 30 40 50 60 70 80 90 100

	DATE	APPROVED	AMENDMENT	REVISION
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DUNEDIN	a-rohe o
CITY COUNCIL	Ötepoti

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Dunedin Office +64 3 471 5500	Private Bag 1913 Dunedin 9016 New Zealand
	CIV/II

SCALES 1:1000		ORIGINAL SIZE A1
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DRAWING VERIFIED	DESIGN VERIFIED	APPROVED DATE

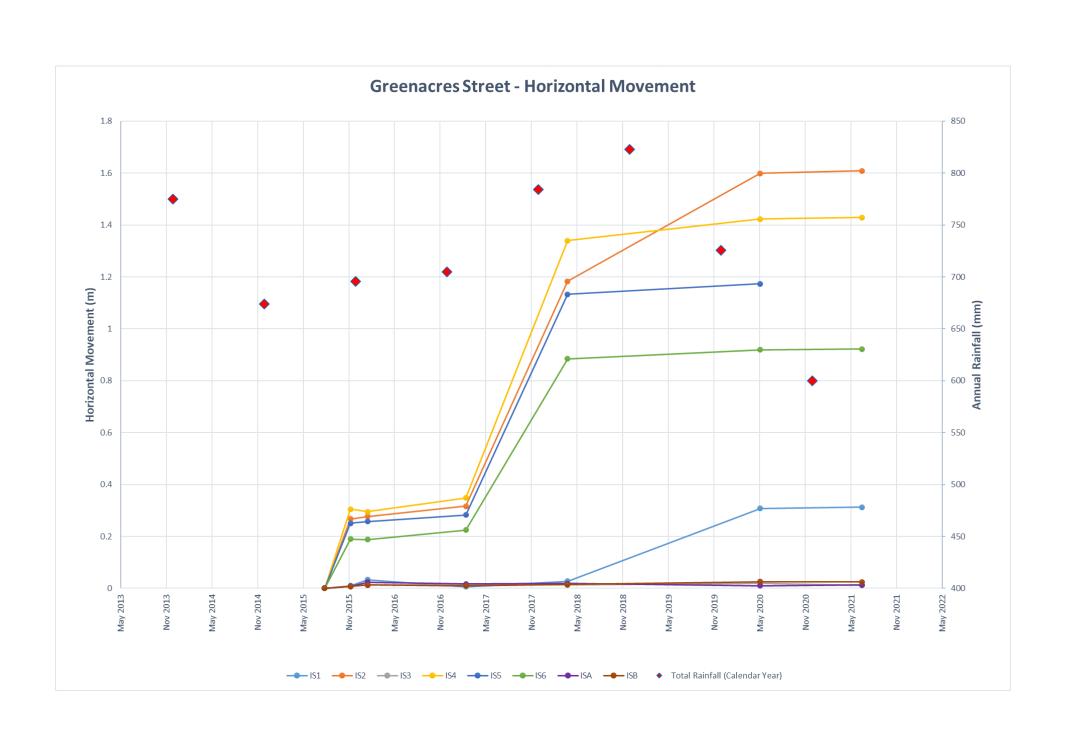
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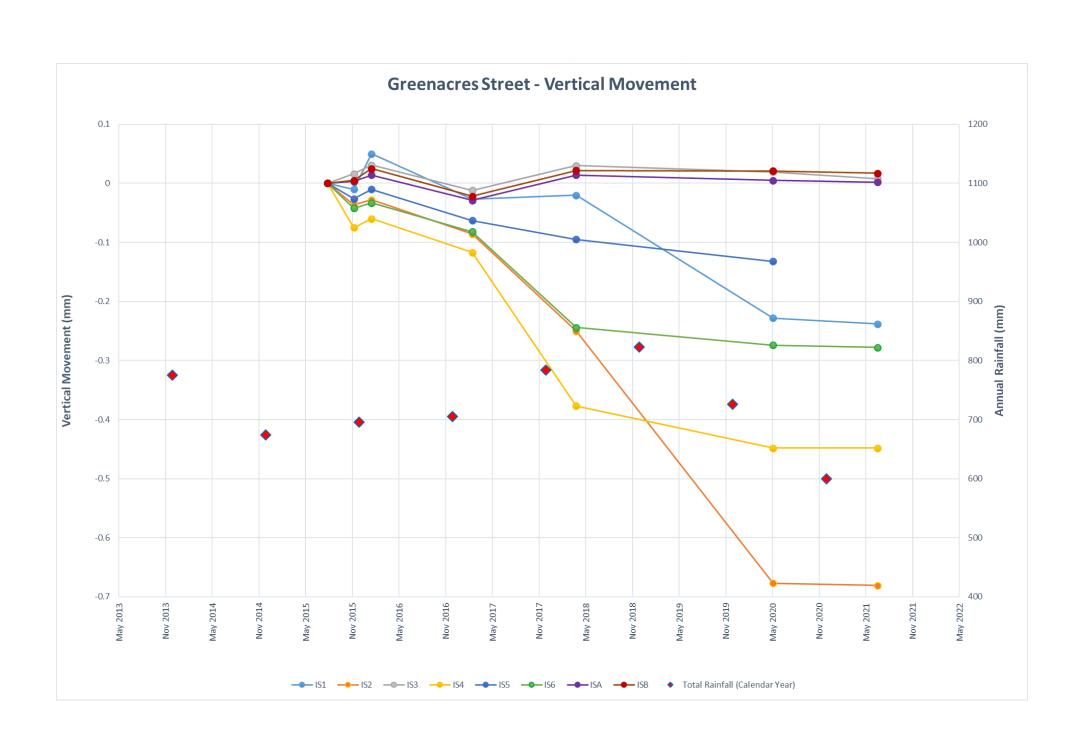
PROJECT DUENDIN CITY GREENACRES LONG TERM L	STREET		
LAND SLIDE M GREENACRES			
WSP PROJECT NO. 6-CD109.55	PROJ-ORIG-VOL-LOC-TYPE 6-CD109.55	SHEET NO.	REVISION A

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Original sheet size A1 (841x594) Plot Date 2021-08-05 at 8:41:28 AM

Appendix B Selected Monitoring Charts





Appendix C Cumulative Monitoring Results Spreadsheet

Greenacres Monitoring Survey

Datum: North Taieri 2000

Heights: Ellipsoidal
Origin: DUND
Field Method: Rapid Static

Processed: TBC

Precision: 15mm Hz & 20mm Vt

Date: 18-Jun-21

Point ID	Easting	Northing	Elevation	Feature Code	Present to Previous Survey				Present to Original					
DUND	424425.439	797475.867	386.925	Base Station	mE	mN	Bearing	Dist	mZ	mE	mN	Bearing	Dist	mZ
IS1	424790.737	798547.500	94.366	IS1	-0.002	0.010	348.690	0.010	-0.010	-0.296	0.100	288.667	0.312	-0.238
IS2	424789.187	798582.929	86.454	IS2	-0.003	0.010	343.301	0.010	-0.004	0.268	1.586	9.591	1.608	-0.681
IS3	424843.107	798601.568	80.951	IS3	0.003	-0.006	153.435	0.007	-0.011	-0.004	0.011	340.017	0.012	0.008
IS4	424785.357	798643.333	72.469	IS4	0.002	0.006	18.435	0.006	0.000	-0.088	1.426	356.469	1.429	-0.448
IS5	Not available		IS5			v.						1).		
IS6	424769.421	798756.668	49.680	IS6	0.004	0.004	45.000	0.006	-0.004	-0.172	0.906	349.251	0.922	-0.278
ISA	424705.909	798523.729	100.976	ISA	-0.001	0.005	348.690	0.005	-0.003	-0.010	0.009	311.987	0.013	0.002
ISB	424818.751	798719.701	59.369	ISB	0.001	-0.002	153.435	0.002	-0.004	0.017	0.018	43.363	0.025	0.017
X75	424540.135	800500.719	7.264	X75	0.005	0.006	39.806	0.008	-0.013	0.000	-0.006	180.000	0.006	-0.006

